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DENMARK

Blokhush Sand

part 1

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Publication date:
1995

Document Version
Publisher's PDF, also known as Version of record

[Link to publication from Aalborg University](#)

Citation for published version (APA):

Ibsen, L. B., Bødker, L., & Jakobsen, F. R. (1995). *Blokhush Sand: part 1*. Soil Mechanics Laboratory, Aalborg University.

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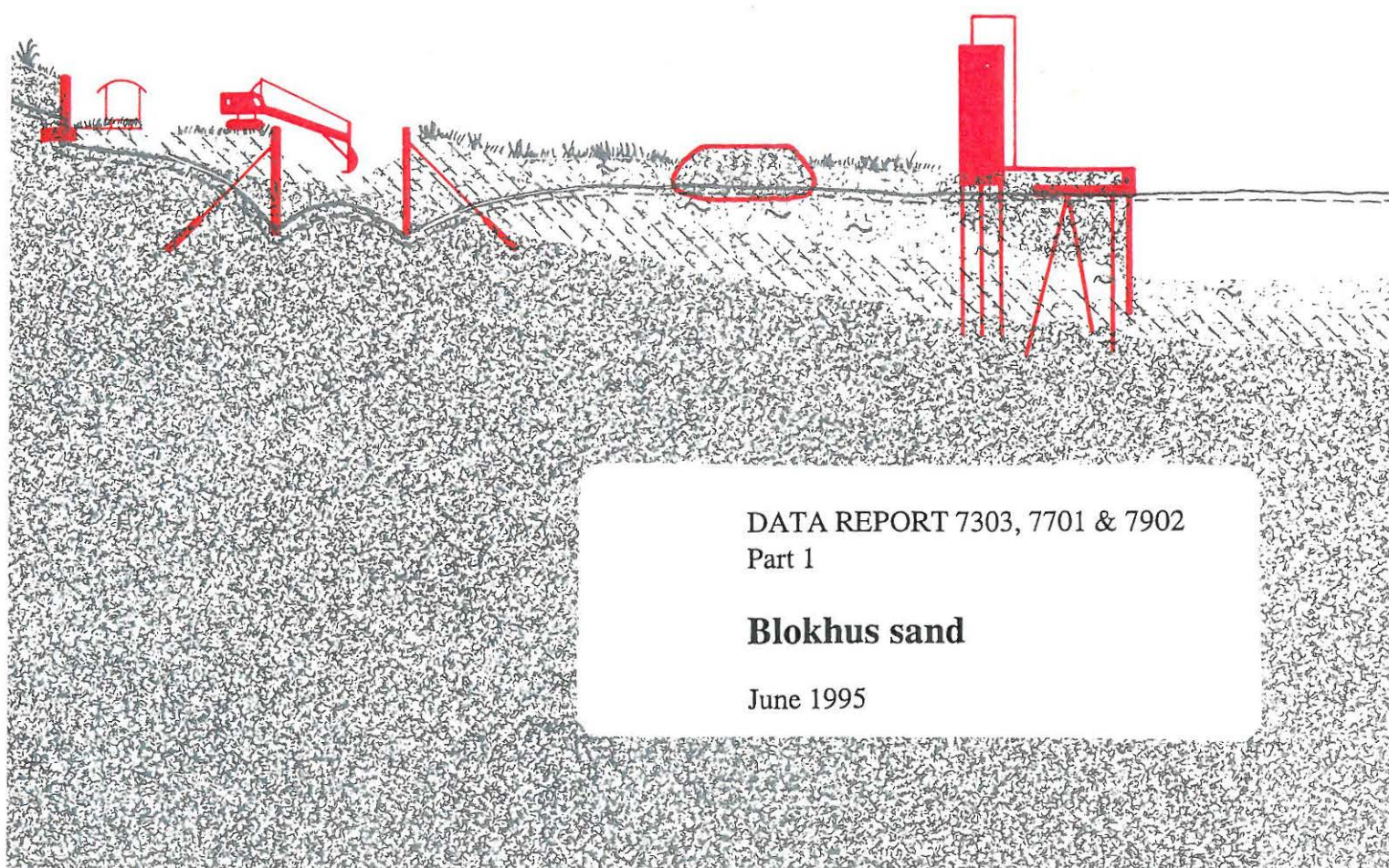
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DATA REPORT 7303, 7701 & 7902
Part 1

Blokhus sand

June 1995

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Blokhush sand

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List of symbols.

Latin letters

d	= diameter of grain
d_{10}	= 10% fractile
d_{50}	= 50% fractile
d_{60}	= 60% fractile
d_s	= grain density
e	= void ratio
e_0	= void ratio before test
e_f	= void ratio at failure
e_{max}	= maximum void ratio
e_{min}	= minimum void ratio
I_D	= density index
p	= mean stress = $1/3(\sigma_1+2\sigma_3)$
q	= deviatoric stress = $\sigma_1-\sigma_3$
S_w	= saturation
$'$	= effective stress

Greek letters

ε	= strain
ε_1	= vertical strain
ε_v	= volumetric strain = $\varepsilon_1+2\varepsilon_3$
ε_r	= shear strain = $2/3(\varepsilon_1-\varepsilon_3)$
σ	= stress
σ_1	= vertical stress
σ_3	= confining pressure
ν	= Poisson's ratio = $\frac{\Delta\varepsilon_1-\Delta\varepsilon_v}{2\Delta\varepsilon_1}$
ψ	= angle of dilatation = $\sin^{-1}\left(\frac{\Delta\varepsilon_v}{\Delta\varepsilon_v-2\Delta\varepsilon_1}\right)$
$'$	= effective stress

Introduction.

During the last 20 years the Soil Mechanics Laboratory at Aalborg University has performed tests with a sand called Blokhús sand. Blokhús sand is a beach sand and therefore the grains are round. The main part of Blokhús sand is quartz, but it also contains a minor part of feldspar, hornblende and muscovite.

For classification of Blokhús sand the following tests have been performed:

- Sieve test.
- Grain density, d_s .
- Maximum, e_{max} , and minimum, e_{min} , void ratio.

To determine the strength parameters of Blokhús sand some drained and undrained triaxial tests have been performed in the Danish Triaxial Cell. The Danish Triaxial Cell prescribes smooth pressure heads and cylindrical specimens with equal height and diameter.

Classification of the sand.

From sieve tests the following parameters have been determined /Blanár J. et al., 1993/ :

- $d_{50} = 0.168$
- $d_{60}/d_{10} = 1.26$

The distribution of the grains is illustrated in figure 1.

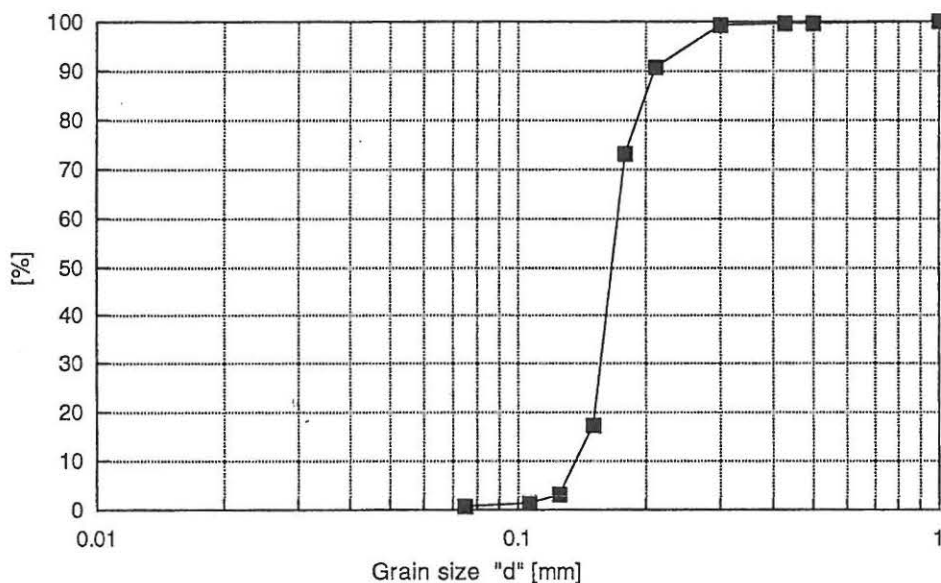


Figure 1 Distribution of the grains for Blokhús sand.

The grain density, maximum and minimum void ratios have been determined to /Blanár J. et al., 1993/ :

- $d_s = 2.65$
- $e_{\max} = 0.834$
- $e_{\min} = 0.564$

All the tests have been performed according to the standard procedures used in the laboratory.

Triaxial tests.

The triaxial tests have been performed in the Soil Mechanics Laboratory. These tests have been performed with a cylindrical specimen with height = diameter = 70 mm and with height = diameter = 250 mm. In order to create homogeneous stress and deformation conditions in the specimens smooth pressure heads are also used.

To investigate the strength parameters of the sand three series of triaxial tests have been performed with 70*70 mm specimens and one series of triaxial tests with 250*250 mm specimens. The three series with 70*70 specimens have been performed with three different void ratios ranging from a loose specimen to a dense specimen. The series with 250*250 mm specimens have only been performed with a dense specimen. The triaxial tests with a dense specimen have been performed with dry sand while the rest of the series have been performed with saturated sand.

Besides the strength parameters at failure the parameters to describe the characteristic state will also be investigated. This state is defined as $\delta\epsilon_v=0$ and is called the characteristic line CL.

Drained triaxial tests

The performed drained triaxial tests with 70*70 mm specimens are listed in table 1 where the test number, void ratio and stress level also are typed.

Stress level, σ_3' [kPa]	Void ratio, $e=0.72$	Void ratio, $e=0.59$	Void ratio, $e=0.55$
2.50	7303.41, 7303.43, 7303.46	7303.21, 7303.22	7701.24, 7701.25
5.00	7303.38, 7303.39	7303.18 - 7303.20	7701.22, 7701.23
10.00	7303.35, 7303.37	7303.14 - 7303.16	7701.19 - 7701.21
20.00	7303.28, 7303.22, 7303.32	7303.12, 7303.14	7701.15 - 7701.18
50.00	7303.44, 7303.45	7303.10, 7303.11	7701.04 - 7701.06
100.00	7303.40, 7303.42	7303.08, 7303.09	7701.01 - 7701.03, 7701.08
200.00	7303.34, 7303.36	7303.06, 7303.07	7701.09, 7701.10
400.00	7303.25-7303.27, 7303.29, 7303.31, 7303.33	7303.02, 7303.03	7701.11, 7701.12
800.00	7303.23, 7303.24	7303.04, 7303.05	7701.13, 7701.14

Table 1 Test numbers for the performed drained tests with 70*70 mm specimens.

The drained triaxial tests performed with 250*250 mm specimens are listed in table 2.

Stress level, σ_3' [kPa]	Void ratio, $e=0.55$
10.00	7902.02 - 7902.06
20.00	7902.07, 7902.08
40.00	7902.09 - 7902.13
80.00	7902.14 - 7902.16

Table 2 Test numbers for the performed drained tests with 250*250 mm specimens.

The main results from the performed tests are listed in the following tables. Two tables are connected to each test series. One for values at failure and one for values at $\delta\epsilon_v = 0$.

Test series CD, $I_d=0.90$				Values at failure					
Test No	e_0	e_r	S_w	σ_3' kPa	p' kPa	q' kPa	ϵ_1 %	ϵ_v %	ϵ_q %
7303.21	0.58	0.68		2.50	9.07	19.70	6.88	-6.15	8.93
7303.22	0.59	0.66	0.97	2.50	8.04	16.61	5.34	-4.93	6.98
7303.18	0.60	0.72	0.98	5.00					
7303.19	0.57	0.67	1.00	5.00	14.75	29.26	7.25	-6.06	9.27
7303.20	0.57	0.69	0.99	5.00	15.42	31.27	8.07	-7.24	10.48
7303.14	0.58	0.65	0.99	10.00	24.56	43.68	5.96	-4.55	7.48
7303.15	0.59	0.66	0.99	10.00	27.07	51.22	5.64	-4.08	7.00
7303.16	0.64	0.69	0.98	10.00	25.66	46.97	5.18	-3.03	6.19
7303.12	0.58	0.64	1.00	20.00	49.62	88.85	5.97	-4.15	7.35
7303.13	0.57	0.65	0.97	20.00	49.06	87.17	6.31	-4.62	7.85
7303.10	0.61	0.66	0.93	50.00	108.68	176.04	6.41	-3.45	7.56
7303.11	0.57	0.64	0.99	50.00	113.05	189.14	6.40	-4.11	7.77
7303.08	0.59	0.64	0.98	100.00	212.81	338.43	6.54	-3.54	7.72
7303.09	0.57	0.62	0.98	100.00	215.20	345.60	5.16	-2.89	6.12
7303.06	0.60	0.65	0.96	199.50	403.16	610.97	6.46	-2.71	7.36
7303.07	0.58	0.63	0.97	200.00	416.67	650.00	6.40	-3.21	7.47
7303.01	0.60	0.60	1.01	400.00					
7303.02	0.59	0.64	1.00	400.00	784.42	1153.25	7.23	-3.01	8.23
7303.03	0.60	0.65	0.94	400.00	782.38	1147.15	7.27	-2.78	8.20
7303.04	0.62	0.66	0.95	800.00	1506.79	2120.36	8.12	-2.04	8.80
7303.05	0.62	0.65	0.95	800.00	1509.96	2129.89	7.44	-1.93	8.08

Table 3.a Values at failure for Blokhuis sand with I_d equal to 0.90, and with 70*70 mm specimens.

Test series CD, $I_d=0.90$				Values at $\delta\varepsilon_v=0$					
Test No	e_0	e_r	S_w	σ'_3 kPa	p' kPa	q' kPa	ε_1 %	ε_v %	ε_q %
7303.21	0.58	0.68		2.50	3.25	2.24	0.00	0.00	0.00
7303.22	0.59	0.66	0.97	2.50	3.25	2.24	0.00	0.00	0.00
7303.18	0.60	0.72	0.98	5.00	8.08	9.23	0.12	0.00	0.12
7303.19	0.57	0.67	1.00	5.00	6.52	4.56	0.04	0.02	0.03
7303.20	0.57	0.69	0.99	5.00	6.52	4.57	0.03	0.02	0.02
7303.14	0.58	0.65	0.99	10.00	14.29	12.87	0.07	0.04	0.06
7303.15	0.59	0.66	0.99	10.00	16.09	18.26	0.16	0.04	0.15
7303.16	0.64	0.69	0.98	10.00	16.00	18.00	0.19	0.04	0.18
7303.12	0.58	0.64	1.00	20.00	31.82	35.47	0.22	0.04	0.21
7303.13	0.57	0.65	0.97	20.00	31.83	35.48	0.19	0.04	0.18
7303.10	0.61	0.66	0.93	50.00	73.98	70.98	0.20	0.07	0.18
7303.11	0.57	0.64	0.99	50.00	73.75	71.24	0.20	0.07	0.18
7303.08	0.59	0.64	0.98	100.00	154.37	163.11	0.48	0.07	0.46
7303.09	0.57	0.62	0.98	100.00	157.82	173.45	0.43	0.07	0.41
7303.06	0.60	0.65	0.96	199.50	305.63	318.40	0.70	0.14	0.65
7303.07	0.58	0.63	0.97	200.00	306.75	320.24	0.59	0.11	0.55
7303.01	0.60	0.60	1.01	400.00	596.38	589.13	0.96	0.17	0.90
7303.02	0.59	0.64	1.00	400.00	596.50	589.50	0.95	0.11	0.92
7303.03	0.60	0.65	0.94	400.00	596.60	589.79	1.13	0.23	1.05
7303.04	0.62	0.66	0.95	800.00	1210.72	1231.72	1.71	0.25	1.63
7303.05	0.62	0.65	0.95	800.00	1210.21	1230.63	1.56	0.23	1.48

Table 3.b Values at $d\varepsilon_v = 0$ for Blokhuis sand with I_D equal to 0.90, and with 70*70 mm specimens.

Test series CD $I_d=0.43$				Values at failure					
Test No	e_0	e_r	S_w	σ'_3 kPa	p' kPa	q' kPa	ε_1 %	ε_v %	ε_q %
7303.41	0.65	0.71	1.02	1.85	5.80	11.85	7.84	-3.90	9.14
7303.46	0.68	0.72	0.94	1.85	7.68	17.48	4.28	-2.35	5.06
7303.43	0.77	0.84	0.94	2.50					
7303.38	0.74	0.79	0.96	4.35	10.44	18.27	8.19	-2.80	9.12
7303.39	0.68	0.74	0.96	4.35	11.75	22.20	6.04	-3.45	7.19
7303.35	0.70	0.75	0.92	10.00	24.37	45.05	5.26	-2.85	6.21
7303.37	0.72	0.78	0.95	10.00	22.28	38.80	9.45	-3.30	10.55
7303.28	0.72	0.77	0.84	20.00	42.49	67.48	6.23	-2.42	7.04
7303.30	0.74	0.77	0.96	20.00	40.98	62.94	6.71	-2.06	7.40
7303.32	0.70	0.76	0.90	20.00	44.33	73.00	7.19	-3.45	8.34
7303.44	0.74	0.78	0.89	50.00	98.29	144.86	6.99	-2.24	7.74
7303.45	0.69	0.72	0.93	50.00	102.74	158.23	5.22	-2.11	5.92
7303.40	0.73	0.77	0.96	100.00	187.75	263.25	10.78	-2.51	11.61
7303.42	0.76	0.79	0.89	100.00	186.33	259.00	7.57	-1.80	8.17
7303.34	0.72	0.75	0.98	200.00					
7303.36	0.73	0.77	0.89	200.00	378.42	535.27	7.88	-1.97	8.54
7303.25	0.68	0.70	0.97	400.00	723.80	971.39	8.35	-1.25	8.77
7303.26	0.70	0.69	1.01	400.00					
7303.27	0.72	0.72	0.96	400.00					
7303.29	0.74	0.74	0.94	400.00					
7303.31	0.76	0.76	0.96	400.00					
7303.33	0.72	0.73	0.95	400.00					
7303.23	0.71	0.73	0.93	800.00	1390.12	1770.35	10.47	-0.88	10.76
7303.24	0.70	0.72	0.97	800.00	1418.56	1855.69	9.96	-0.99	10.29

Table 4.a Values at failure for Blokhuis sand with I_D equal to 0.43 and with 70*70 mm specimens.

Test series CD $I_d=0.43$				Values at $\delta\varepsilon_v=0$					
Test No	e_0	e_r	S_w	σ_3' kPa	p' kPa	q' kPa	ε_1 %	ε_v %	ε_q %
7303.41	0.65	0.71	1.02	1.85	2.61	2.29	0.07	0.00	0.07
7303.46	0.68	0.72	0.94	1.85	3.41	4.68	0.08	0.00	0.08
7303.43	0.77	0.84	0.94	2.50	3.01	3.48	0.27	0.00	0.27
7303.38	0.74	0.79	0.96	4.35	4.73	1.14	0.00	0.00	0.00
7303.39	0.68	0.74	0.96	4.35	5.52	3.50	0.00	0.00	0.00
7303.35	0.70	0.75	0.92	10.00	14.45	15.30	0.13	0.02	0.12
7303.37	0.72	0.78	0.95	10.00	13.64	12.88	0.20	0.04	0.19
7303.28	0.72	0.77	0.84	20.00	30.18	30.54	0.21	0.05	0.19
7303.30	0.74	0.77	0.96	20.00	29.92	29.76	0.26	0.05	0.24
7303.32	0.70	0.76	0.90	20.00	26.18	18.53	0.08	0.04	0.07
7303.44	0.74	0.78	0.89	50.00	78.08	84.23	0.35	0.07	0.33
7303.45	0.69	0.72	0.93	50.00	78.06	84.17	0.44	0.09	0.41
7303.40	0.73	0.77	0.96	100.00	157.79	173.38	0.87	0.14	0.82
7303.42	0.76	0.79	0.89	100.00	153.57	160.70	0.75	0.13	0.71
7303.34	0.72	0.75	0.98	200.00	312.00	336.01	1.00	0.20	0.93
7303.36	0.73	0.77	0.89	200.00	311.95	335.85	1.01	0.16	0.96
7303.25	0.68	0.70	0.97	400.00	606.92	620.77	1.44	0.22	1.37
7303.26	0.70	0.69	1.01	400.00	622.17	666.51	4.93	0.63	4.72
7303.27	0.72	0.72	0.96	400.00	618.08	654.25	1.98	0.29	1.88
7303.29	0.74	0.74	0.94	400.00	618.59	655.76	1.66	0.29	1.56
7303.33	0.72	0.73	0.95	400.00	640.46	721.38	1.68	0.27	1.59
7303.31	0.76	0.76	0.96	400.00	618.59	655.77	1.82	0.36	1.70
7303.23	0.71	0.73	0.93	800.00	1246.36	1339.09	3.97	0.33	3.86
7303.24	0.70	0.72	0.97	800.00	1269.89	1409.67	2.78	0.36	2.66

Table 4.b Values at $d\varepsilon_v = 0$ for Blokhuis sand with I_D equal to 0.43, and with 70*70 mm specimens.

Test series CD $I_d=1.06$				Values at failure					
Test No	e_0	e_r	S_w	σ'_3 kPa	p' kPa	q' kPa	ϵ_1 %	ϵ_v %	ϵ_q %
7701.24	0.55	0.65	0.00	2.50	10.10	22.80	6.87	-6.28	8.96
7701.25	0.54	0.63	0.00	2.50	9.70	21.60	6.96	-6.03	8.97
7701.22	0.55	0.64	0.00	5.00	16.00	32.90	6.47	-5.81	8.41
7701.23	0.54	0.62	0.00	5.00	15.80	32.50	6.24	-5.05	7.92
7701.19	0.53	0.63	0.00	8.00	22.70	44.20	6.51	-4.98	8.17
7701.20	0.56	0.64	0.00	8.00	22.00	41.90	6.51	-4.98	8.17
7701.21	0.55	0.62	0.00	8.00	22.40	43.10	5.82	-4.58	7.35
7701.15	0.55	0.62	0.00	20.00	52.30	97.00	5.06	-4.20	6.46
7701.16	0.56	0.62	0.00	20.00	47.30	82.00	6.56	-3.75	7.81
7701.17	0.53	0.59	0.00	20.00	47.90	83.83	5.91	-3.97	7.23
7701.18	0.57	0.63	0.00	20.00	50.60	91.80	5.04	-3.68	6.27
7701.04	0.53	0.59	0.00	50.00	126.80	230.50	5.00	-4.08	6.36
7701.05	0.52	0.58	0.00	50.00	125.70	227.20	5.08	-3.92	6.39
7701.06	0.54	0.60	0.00	50.00	124.30	222.80	5.25	-4.26	6.67
7701.07	0.55	0.62	0.00	50.00	122.80	218.50	6.10	-4.69	7.66
7701.01	0.59	0.65	0.96	100.00					
7701.02	0.58	0.62	0.00	100.00	234.80	404.50	4.71	-2.94	5.69
7701.03	0.55	0.61	0.00	100.00	234.30	429.90	5.26	-3.65	6.48
7701.08	0.55	0.62	0.00	100.00	235.90	407.70	6.41	-4.57	7.93
7701.09	0.55	0.60	0.00	200.00	464.60	793.90	6.11	-3.46	7.26
7701.10	0.55	0.60	0.00	200.00	456.80	770.30	6.71	-3.68	7.94
7701.11	0.55	0.60	0.00	400.00	888.80	1466.50	7.72	-3.68	8.95
7701.12	0.54	0.60	0.00	400.00	900.30	1500.80	7.34	-3.54	8.52
7701.13	0.55	0.60	0.00	800.00	1729.30	2788.00	7.67	-3.03	8.68

Table 5.a Values at failure for Blokhuis sand with I_d equal to 1.06 and with 70*70 mm specimens.

Test series CD $I_d=1.06$				Values at $\delta\epsilon_v=0$					
Test No	e_0	e_r	S_w	σ_3' kPa	p' kPa	q' kPa	ϵ_1 %	ϵ_v %	ϵ_q %
7701.24	0.55	0.65	0.00	2.50	3.90	4.20	0.07	0.04	0.06
7701.25	0.54	0.63	0.00	2.50	3.90	4.30	0.05	0.00	0.05
7701.22	0.55	0.64	0.00	5.00	6.40	4.30	0.01	0.00	0.01
7701.23	0.54	0.62	0.00	5.00	8.50	10.60	0.29	0.04	0.28
7701.19	0.53	0.63	0.00	8.00	14.30	18.90	0.32	0.00	0.32
7701.20	0.56	0.64	0.00	8.00	14.30	18.90	0.29	0.00	0.29
7701.21	0.55	0.62	0.00	8.00	11.90	11.60	0.12	0.00	0.12
7701.15	0.55	0.62	0.00	20.00	38.10	54.20	0.49	0.04	0.48
7701.16	0.56	0.62	0.00	20.00	33.50	40.50	0.55	0.16	0.50
7701.17	0.53	0.59	0.00	20.00	35.90	47.70	0.47	0.04	0.46
7701.18	0.57	0.63	0.00	20.00	33.50	40.60	0.32	0.04	0.31
7701.04	0.53	0.59	0.00	50.00	90.00	122.60	0.50	0.07	0.48
7701.05	0.52	0.58	0.00	50.00	82.50	97.60	0.40	0.11	0.36
7701.06	0.54	0.60	0.00	50.00	90.00	122.70	0.52	0.04	0.51
7701.07	0.55	0.62	0.00	50.00	91.00	122.90	0.64	0.07	0.62
7701.01	0.59	0.65	0.96	100.00	166.60	199.79	0.64	0.11	0.60
7701.02	0.58	0.62	0.00	100.00	175.00	225.00	0.67	0.07	0.65
7701.03	0.55	0.61	0.00	100.00	175.00	225.10	0.69	0.11	0.65
7701.08	0.55	0.62	0.00	100.00	167.20	201.60	0.69	0.05	0.67
7701.09	0.55	0.60	0.00	200.00	324.90	374.70	0.86	0.14	0.81
7701.10	0.55	0.60	0.00	200.00	324.50	373.40	1.02	0.04	1.01
7701.11	0.55	0.60	0.00	400.00	678.50	835.60	2.14	0.18	2.08
7701.12	0.54	0.60	0.00	400.00	679.10	837.20	2.14	0.14	2.09
7701.13	0.55	0.60	0.00	800.00	1312.90	1538.80	2.64	0.25	2.56

Table 5.b Values at $d\epsilon_v = 0$ for Blokhuis sand with I_D equal to 1.06, and with 70*70 mm specimens.

Test series CD $I_D=1.05$				Values at failure					
Test No	e_0	e_r	S_w	σ_3' kPa	p' kPa	q' kPa	ε_1 %	ε_v %	ε_q %
7902.02	0.63	0.66	0.00	6.40					
7902.03	0.52	0.60	0.00	6.50	17.20	32.10	5.29	-4.85	6.10
7902.04	0.55	0.61	0.00	6.20	15.71	28.23	4.80	-3.54	5.98
7902.05	0.54	0.58	0.00	6.70	16.94	30.73	4.80	-2.97	5.79
7902.06	0.54	0.59	0.00	6.50	16.07	28.70	4.86	-3.83	6.14
7902.07	0.55	0.60	0.00	18.80	42.51	71.14	4.79	-3.26	5.88
7902.08	0.54	0.59	0.00	18.80	41.59	68.38	4.65	-3.14	5.70
7902.09	0.56	0.57	0.00	38.80					
7902.10	0.55	0.55	0.00	43.00	93.86	150.26	3.65	-1.97	4.31
7902.11	0.54	0.56	0.00	38.80					
7902.12	0.56	0.59	0.00	43.00	77.10	116.07	3.75	-1.79	4.34
7902.13	0.54	0.60	0.00	38.80	81.94	129.43	6.19	-3.81	7.46
7902.14	0.56	0.56	0.00	80.00					
7902.15	0.55	0.57	0.00	78.80					
7902.16	0.54	0.56	0.00	78.80	155.01	228.63	3.31	-1.26	3.73

Table 6.a Values at failure for Blokhuis sand with I_D equal to 1.05 and with 250*250 mm specimens

Test series CD $I_D=1.05$				Values at $\delta\varepsilon_v=0$					
Test No	e_0	e_r	S_w	σ_3' kPa	p' kPa	q' kPa	ε_1 %	ε_v %	ε_q %
7902.02	0.63	0.66	0.00	6.40	7.43	3.70	0.00	0.00	0.00
7902.03	0.52	0.60	0.00	6.20	7.53	4.00	0.00	0.00	0.00
7902.04	0.55	0.61	0.00	6.20	11.39	15.57	0.14	0.00	0.14
7902.05	0.54	0.58	0.00	6.50	11.71	15.63	0.13	0.00	0.13
7902.06	0.54	0.59	0.00	6.50	11.63	15.99	0.15	0.00	0.15
7902.07	0.55	0.60	0.00	18.80	31.80	38.99	0.20	0.00	0.20
7902.08	0.54	0.59	0.00	18.80	32.04	39.73	0.22	0.00	0.22
7902.09	0.56	0.57	0.00	38.80	65.35	79.64	0.35	0.04	0.34
7902.10	0.55	0.55	0.00	38.80	65.30	79.49	0.28	0.04	0.27
7902.11	0.54	0.56	0.00	38.80	61.95	69.44	0.30	0.04	0.29
7902.12	0.56	0.59	0.00	38.80	65.25	79.36	0.42	0.01	0.42
7902.13	0.54	0.60	0.00	38.80	65.29	79.46	0.34	0.06	0.32
7902.14	0.56	0.56	0.00	80.00	138.85	176.54	1.01	0.01	1.01
7902.15	0.55	0.57	0.00	78.80	117.95	117.44	0.25	0.07	0.23
7902.16	0.54	0.56	0.00	78.80	126.60	143.40	0.39	0.07	0.37

Table 6.b Values at $\delta\varepsilon_v = 0$ for Blokhuis sand with I_D equal to 1.05, and with 250*250 mm specimens.

The tables 3a, 3b, 4a, 4b, 5a, 5b, 6a and 6.b will form the basis of the interpretation of the parameters to describe the strength of Blokhuis sand.

Undrained triaxial tests, $CU_{u=0}$.

The main results from the performed undrained triaxial tests are listed in the following table. Only three undrained tests have been performed, two with 70*70 mm specimen (Test No 7303.50 and 7303.51) and one with 250*250 mm specimen (Test No 7902.17). The tests are performed as $CU_{u=0}$ tests, and the volume of the specimens are therefor constant.

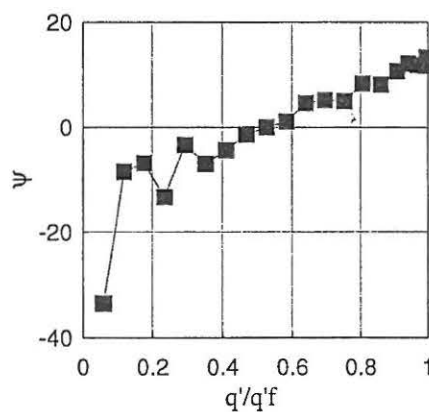
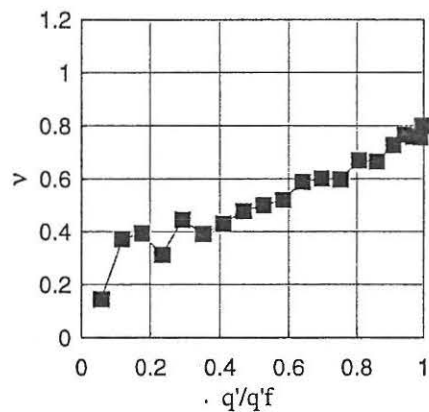
Test series $CU_{u=0}$			Values at start			Values for minimum of σ_3					
Test No	e_0	S_w	σ_3' kPa	p' kPa	q' kPa	σ_3' kPa	p' kPa	q' kPa	ε_1 %	ε_q %	ε_v %
7303.50	0.74	0.91	101.00	101.00	0.00	69.50	100.30	92.40	0.23	0.00	0.23
7303.51	0.71	0.96	50.50	50.50	0.00	35.00	55.04	60.13	0.32	0.00	0.32
7902.17	0.55	0.00	18.30	19.60	3.90	15.30	24.25	26.86	0.08	0.00	0.08

Table 7 Characteristic values to describe the undrained triaxial tests, $CU_{u=0}$.

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	23	
Calibration file	Date	Void ratio	2.65	
		Saturation	0.602	0.636
Loadtransducer	08.05.73	Dimension H mm	1.01	
5001		D mm	72	
			70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	50-400 kPa
		ϵ_1	0.09 %
		ϵ_v	0.43 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'		589.13 kPa
Mean normal stress	p'		596.38 kPa
Confining pressures	σ_3		400.00 kPa
Vertical strain	ϵ_1		0.96 %
Volumetric strain	ϵ_v		0.17 %

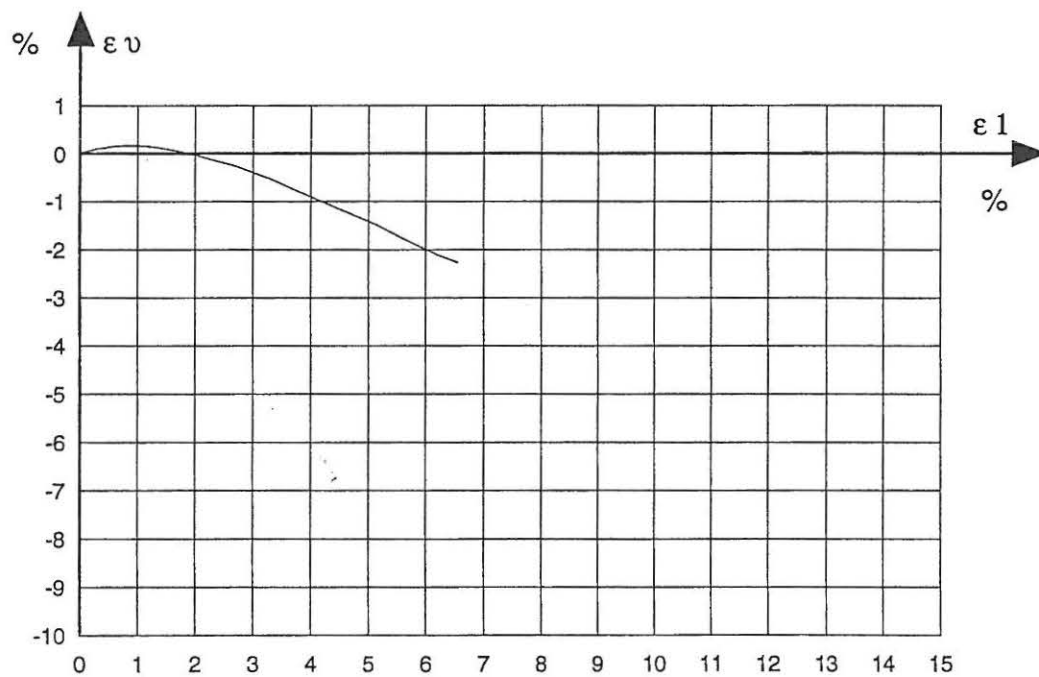
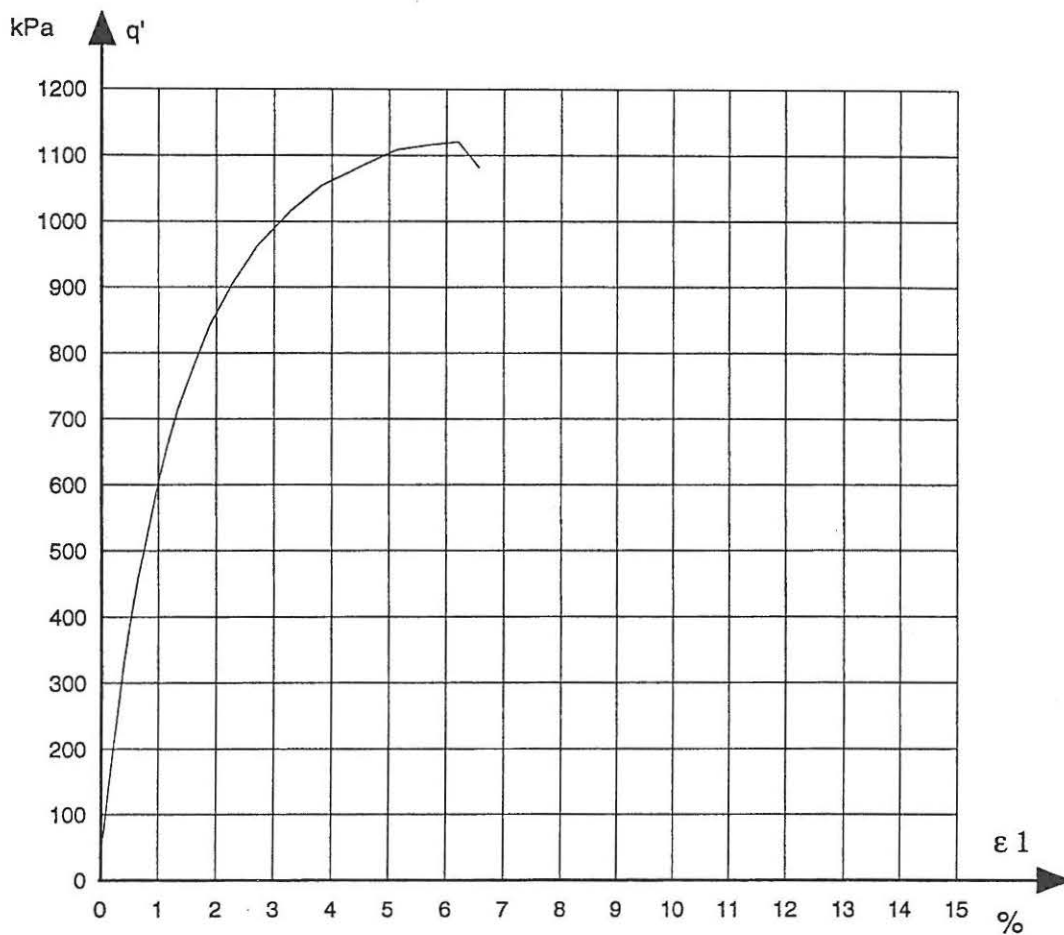


q'	p'	ϵ_1	ϵ_v
64.60	421.53	0.00	0.00
64.56	421.52	0.05	0.04
130.64	443.55	0.12	0.05
196.62	465.54	0.21	0.07
262.53	487.51	0.30	0.11
328.27	509.42	0.40	0.12
393.88	531.29	0.52	0.14
459.28	553.09	0.64	0.16
524.32	574.77	0.80	0.17
589.13	596.38	0.96	0.17
653.53	617.84	1.14	0.16
717.29	639.10	1.34	0.13
780.09	660.03	1.61	0.07
842.36	680.79	1.89	0.02
902.70	700.90	2.26	-0.11
961.59	720.53	2.70	-0.25
1016.77	738.92	3.30	-0.52
1054.26	751.42	3.84	-0.81
1082.83	760.94	4.50	-1.15
1108.58	769.53	5.13	-1.48
1115.54	771.85	5.67	-1.80
1120.30	773.43	6.21	-2.11
1080.38	760.13	6.57	-2.27

Job:	Encl. No
Blokhus sand	1
Exc:	Check:
HSP	LB

Remark:

The specimen slipped out before failure.

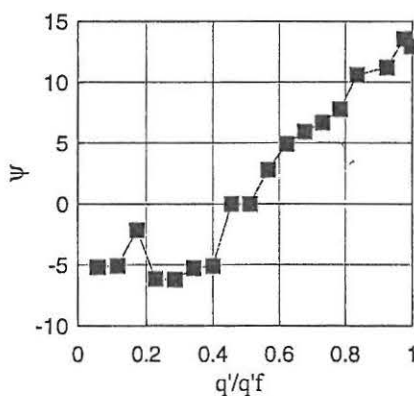
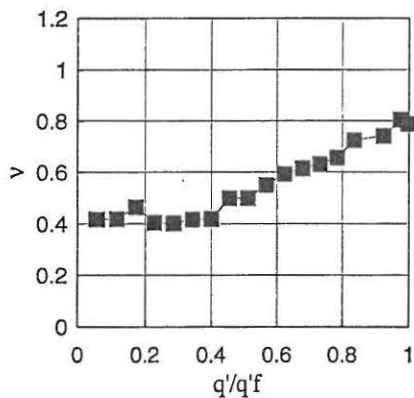


Job:	Encl. No
Blokhus sand	2
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	23	
Calibration file	Date	Void ratio	2.65	
Loadtransducer	09.05.73	Saturation	0.590	0.638
5001		Dimension H mm	1	
		D mm	72	
			70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	50-400 kPa
		ϵ_1	0.11 %
		ϵ_v	0.43 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

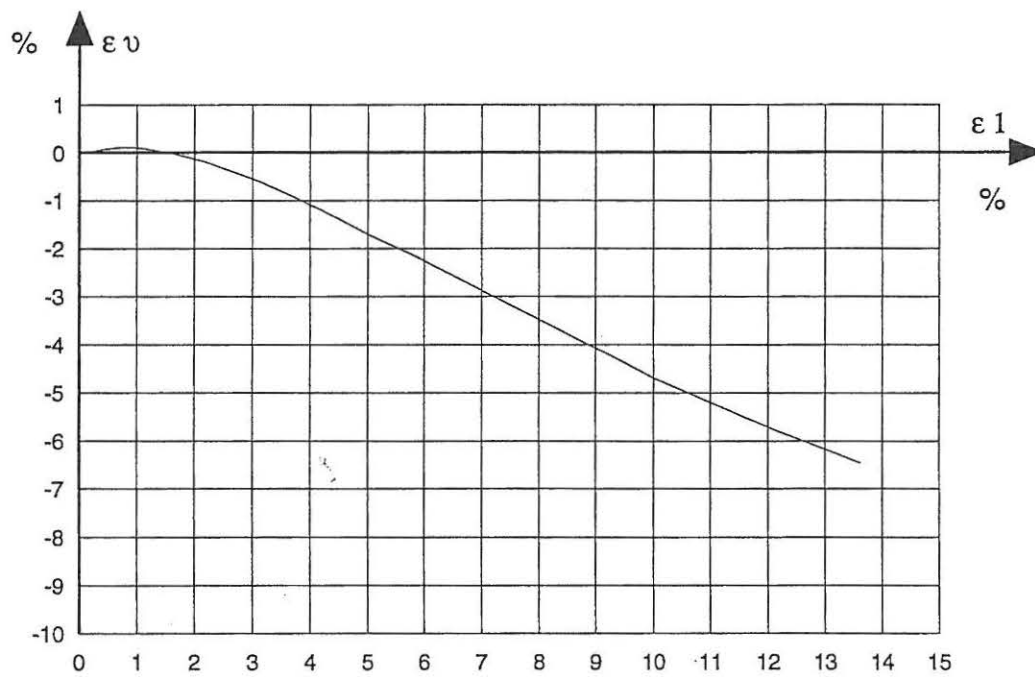
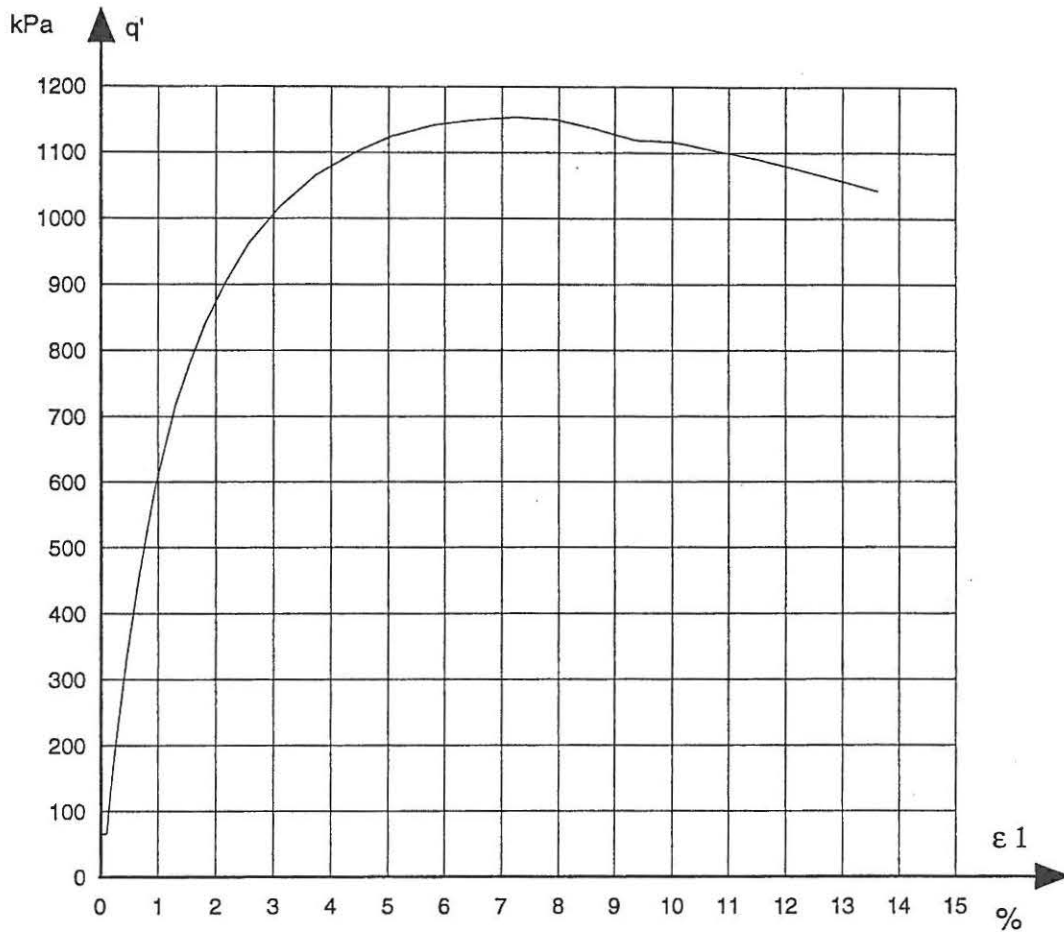
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	1153.25 kPa	589.50 kPa
Mean normal stress	p'	784.42 kPa	596.50 kPa
Confining pressures	σ_3	400.00 kPa	400.00 kPa
Vertical strain	ϵ_1	7.23 %	0.95 %
Volumetric strain	ϵ_v	-3.01 %	0.11 %



q'	p'	ϵ_1	ϵ_v
65.20	421.73	0.00	0.00
65.17	421.72	0.11	0.02
131.17	443.72	0.18	0.03
197.09	465.70	0.27	0.04
262.89	487.63	0.36	0.05
328.60	509.53	0.45	0.07
394.15	531.38	0.56	0.09
459.57	553.19	0.67	0.11
524.66	574.89	0.80	0.11
589.50	596.50	0.95	0.11
653.82	617.94	1.12	0.09
717.63	639.21	1.31	0.05
780.67	660.22	1.55	0.00
842.84	680.95	1.82	-0.07
903.57	701.19	2.17	-0.18
962.48	720.83	2.57	-0.36
1064.80	754.93	3.73	-0.92
1124.48	774.83	5.05	-1.73
1148.84	782.95	6.52	-2.58
1153.25	784.42	7.23	-3.01
1149.99	783.33	7.93	-3.43
1116.20	772.07	10.00	-4.69
1075.17	758.39	12.20	-5.81
1041.64	747.21	13.62	-6.46

Job:	Encl. No
Blokhus sand	3
Exc:	Check:
HSP	LB

Remark:

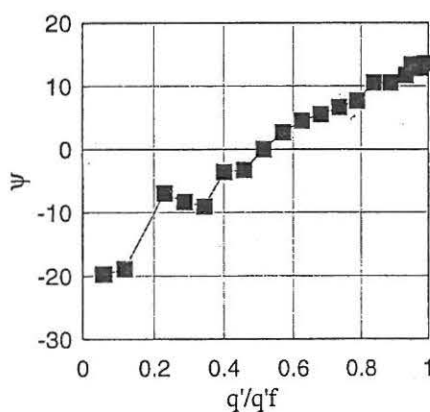
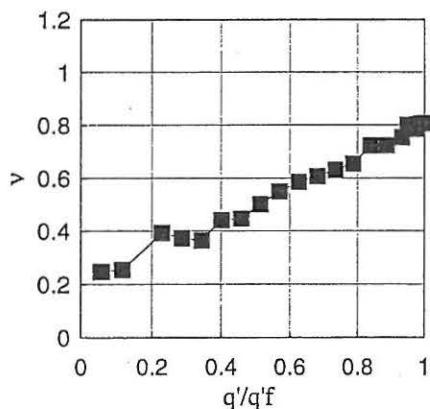


Job:	Encl. No
Blokhus sand	4
Exc:	Check:
HSP	LB

Description of soil			Before test	At failure
Blokhus sand		Water content %	21.3	0.646
		Grain density	2.65	
Calibration file	Date	Void ratio	0.601	
Loadtransducer 5001	10.05.73	Saturation	0.94	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	50-400 kPa
		ϵ_1	0.17 %
		ϵ_v	0.49 %
	2. Drained compression.		
Deformation rate:			9.6 % ph

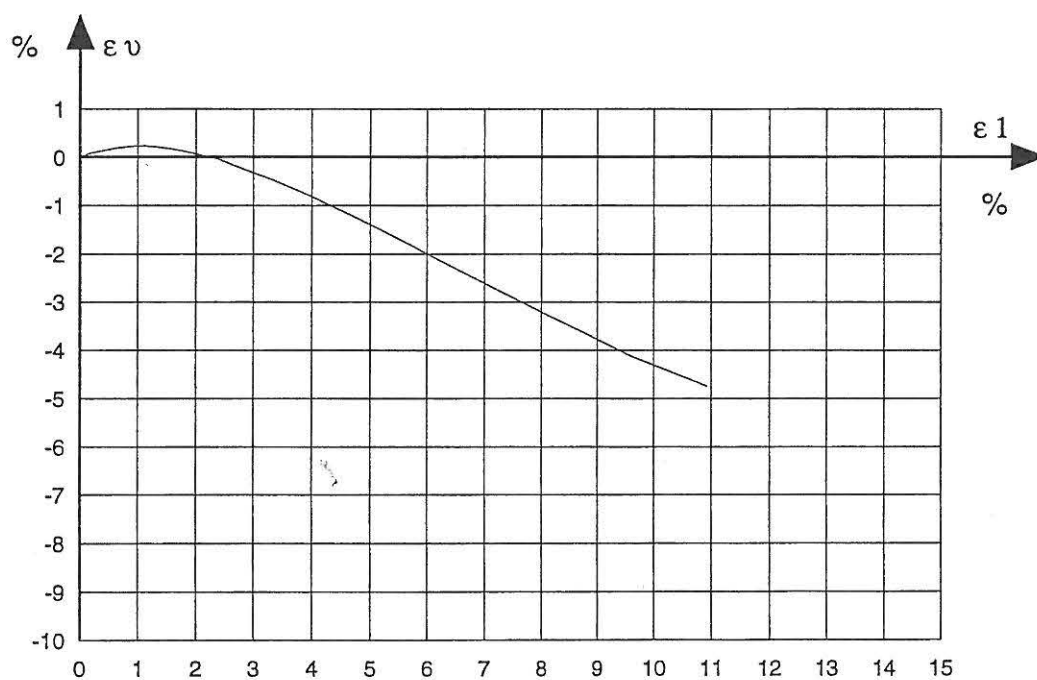
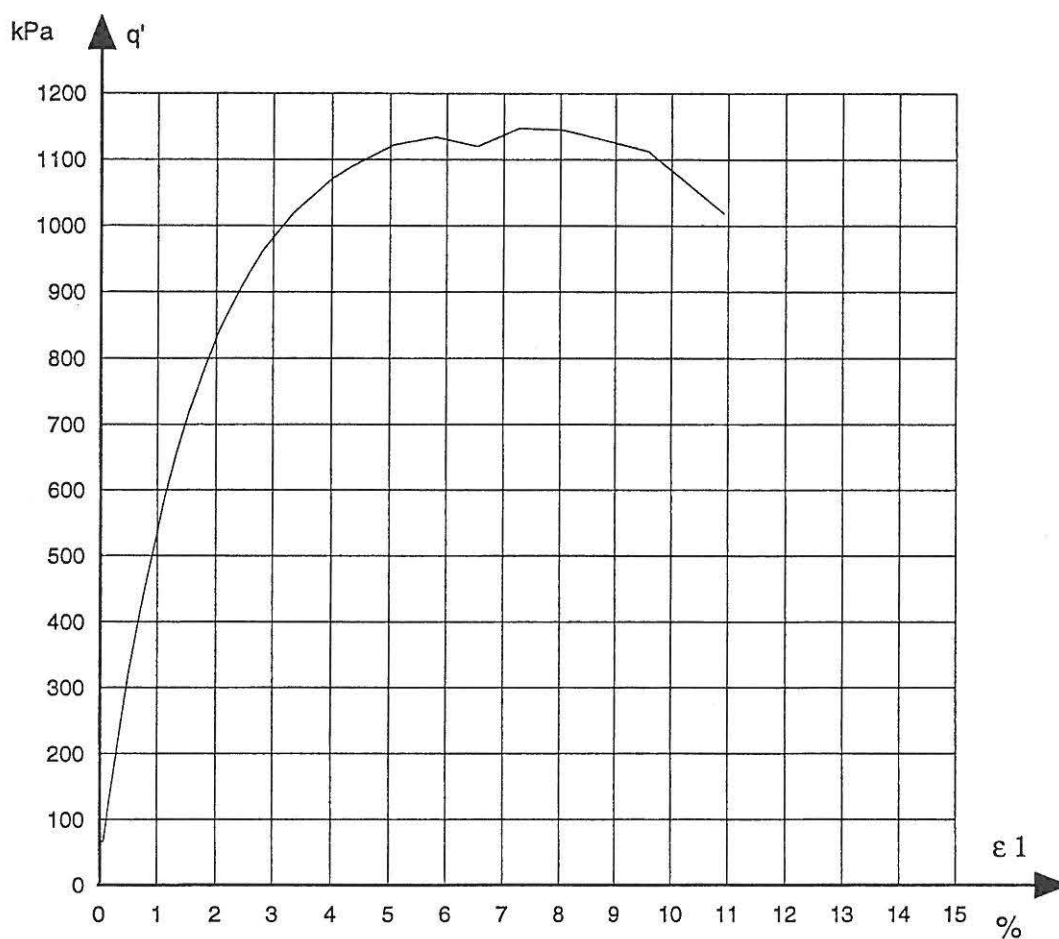
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1147.15	kPa	589.79	kPa
Mean normal stress	p'	782.38	kPa	596.60	kPa
Confining pressures	σ_3	400.00	kPa	400.00	kPa
Vertical strain	ϵ_1	7.27	%	1.13	%
Volumetric strain	ϵ_v	-2.78	%	0.23	%



q'	p'	ϵ_1	ϵ_v
65.90	421.97	0.00	0.00
65.87	421.96	0.07	0.04
131.94	443.98	0.17	0.08
263.65	487.88	0.40	0.13
329.35	509.78	0.52	0.16
394.88	531.63	0.65	0.20
460.11	553.37	0.80	0.22
525.09	575.03	0.97	0.23
589.79	596.60	1.13	0.23
654.00	618.00	1.32	0.22
717.61	639.20	1.53	0.18
780.42	660.14	1.79	0.13
842.51	680.84	2.06	0.05
903.12	701.04	2.41	-0.05
961.92	720.64	2.81	-0.23
1018.16	739.39	3.34	-0.47
1070.63	756.88	4.01	-0.81
1089.42	763.14	4.34	-1.01
1122.06	774.02	5.06	-1.43
1133.89	777.96	5.80	-1.88
1119.73	773.24	6.54	-2.33
1147.15	782.38	7.27	-2.78
1128.83	776.28	8.81	-3.66
1018.00	739.33	10.93	-4.75

Job: Blokhus sand	Encl. No 5
Exc: HSP	Check: LB

Remark:
The specimen slipped out after failure.

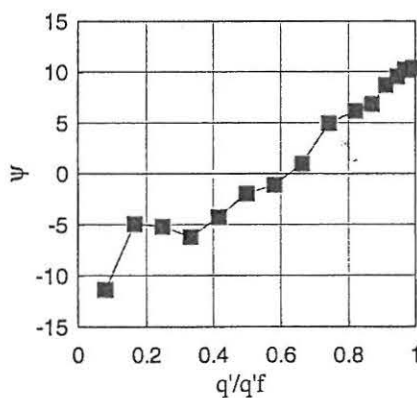
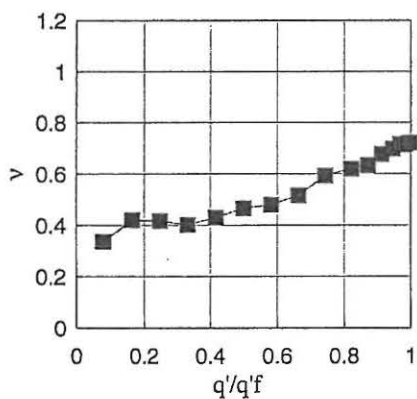


Job:	Encl. No
Blokhuis sand	6
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	22.3	
Calibration file	Date	Void ratio	0.623	0.657
Loadtransducer 15001	20.06.73	Saturation	0.95	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	50-800 kPa
		ϵ_1	0.24 %
		ϵ_v	0.72 %
2. Drained compression.			
Deformation rate:		9.6 % ph	

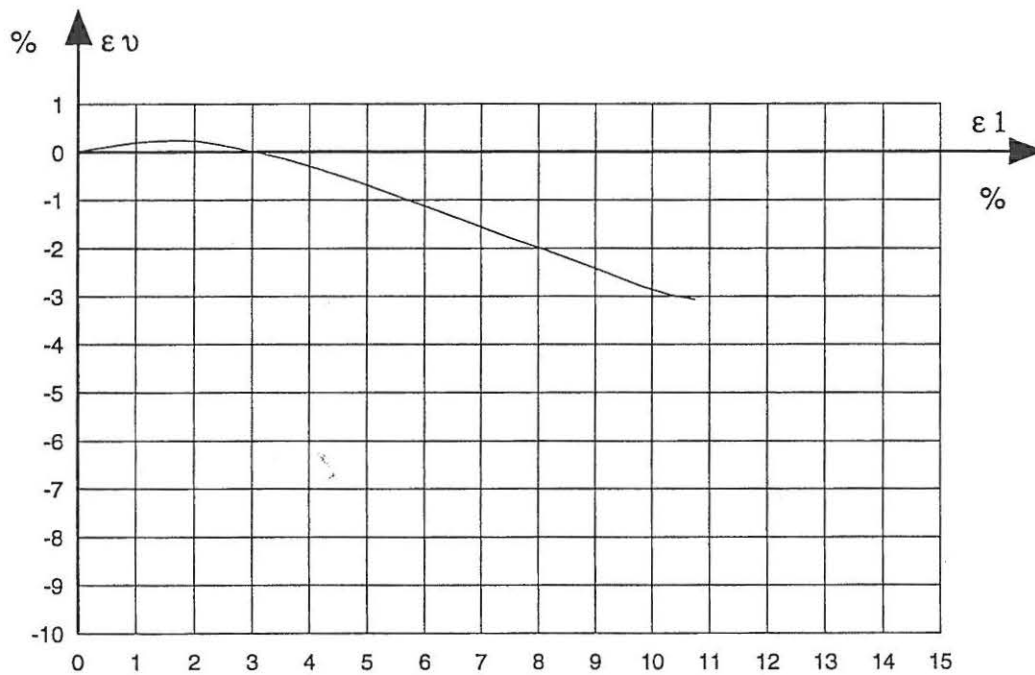
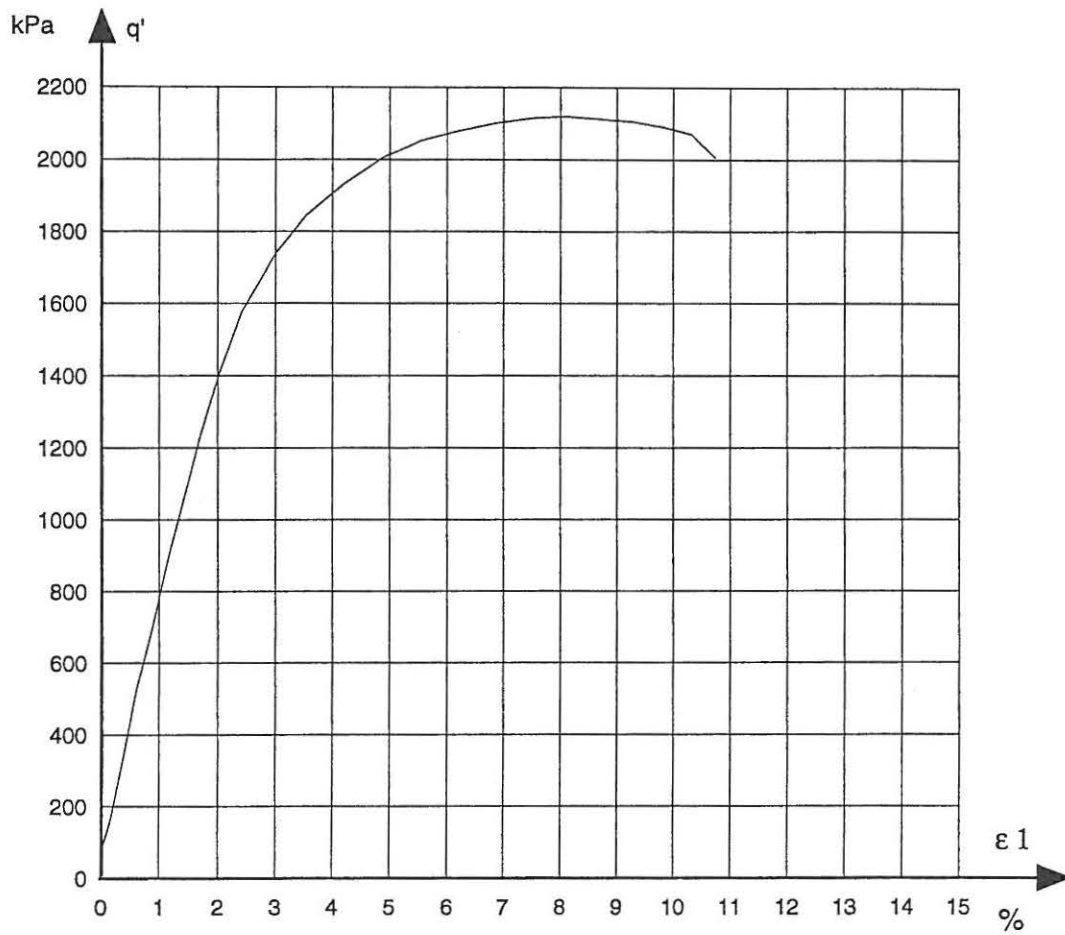
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	2120.36	kPa	1231.72	kPa
Mean normal stress	p'	1506.79	kPa	1210.57	kPa
Confining pressures	σ_3	800.00	kPa	800.00	kPa
Vertical strain	ϵ_1	8.12	%	1.71	%
Volumetric strain	ϵ_v	-2.04	%	0.25	%



q'	p'	ϵ_1	ϵ_v
65.90	821.97	0.00	0.00
166.32	855.44	0.16	0.05
346.05	915.35	0.39	0.09
525.13	975.04	0.61	0.13
703.26	1034.42	0.89	0.18
880.58	1093.53	1.15	0.22
1056.74	1152.25	1.42	0.23
1231.72	1210.57	1.71	0.25
1405.05	1268.35	2.02	0.23
1575.13	1325.04	2.40	0.16
1738.41	1379.47	3.00	0.02
1843.25	1414.42	3.53	-0.13
1930.18	1443.39	4.19	-0.36
2003.87	1467.96	4.87	-0.63
2052.05	1484.02	5.54	-0.92
2080.17	1493.39	6.21	-1.21
2101.51	1500.50	6.87	-1.50
2115.32	1505.11	7.52	-1.79
2120.36	1506.79	8.12	-2.04
2112.79	1504.26	8.70	-2.29
2105.81	1501.94	9.28	-2.54
2091.58	1497.19	9.82	-2.80
2070.88	1490.29	10.32	-2.98
2004.62	1468.21	10.74	-3.07

Job:	Encl. No
Blokhus sand	7
Exc:	Check:
HSP	LB

Remark:

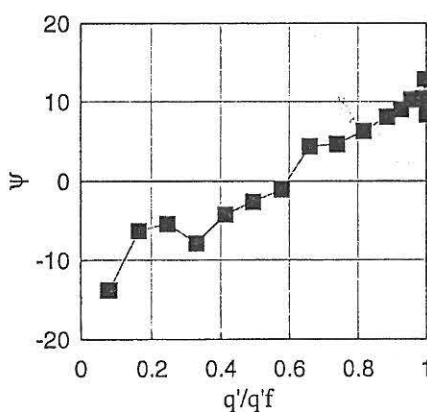
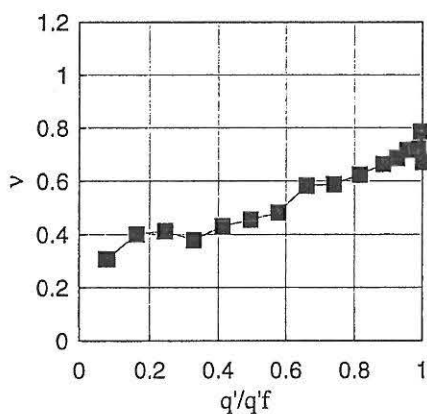


Job:	Encl. No
Blokhus sand	8
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	22.3	
Calibration file	Date	Void ratio	0.623	0.654
Loadtransducer 15001	25.06.73	Saturation	0.95	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	50-800 kPa
		ϵ_1	0.22 %
		ϵ_v	0.49 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	2129.89	kPa	1230.63	kPa
Mean normal stress	p'	1509.96	kPa	1210.21	kPa
Confining pressures	σ_3	800.00	kPa	800.00	kPa
Vertical strain	ϵ_1	7.44	%	1.56	%
Volumetric strain	ϵ_v	-1.93	%	0.23	%

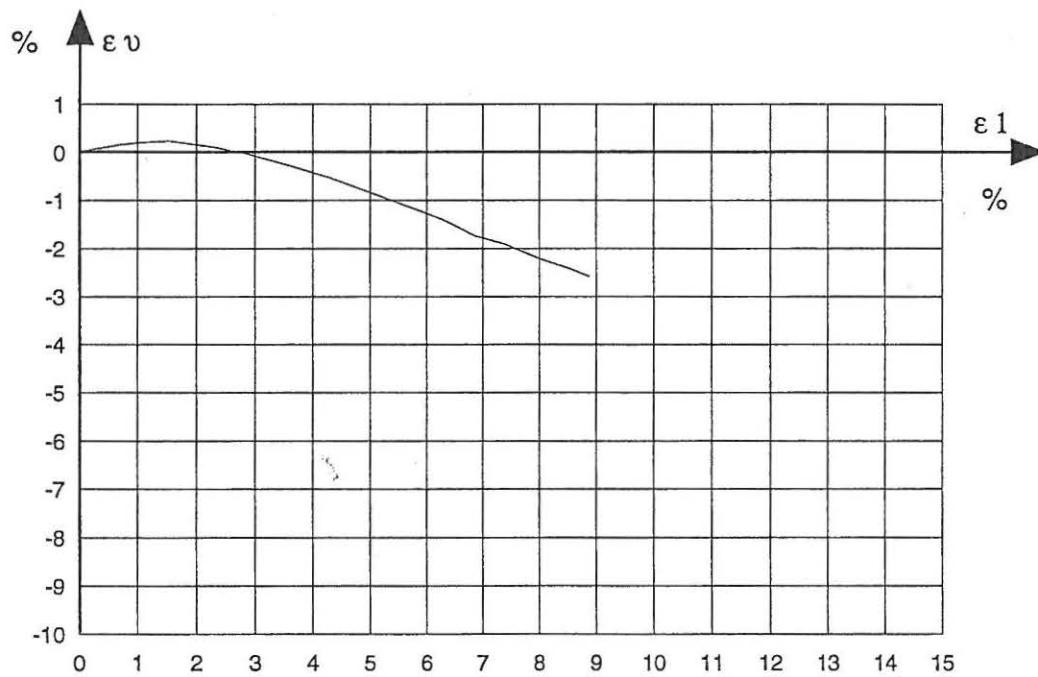
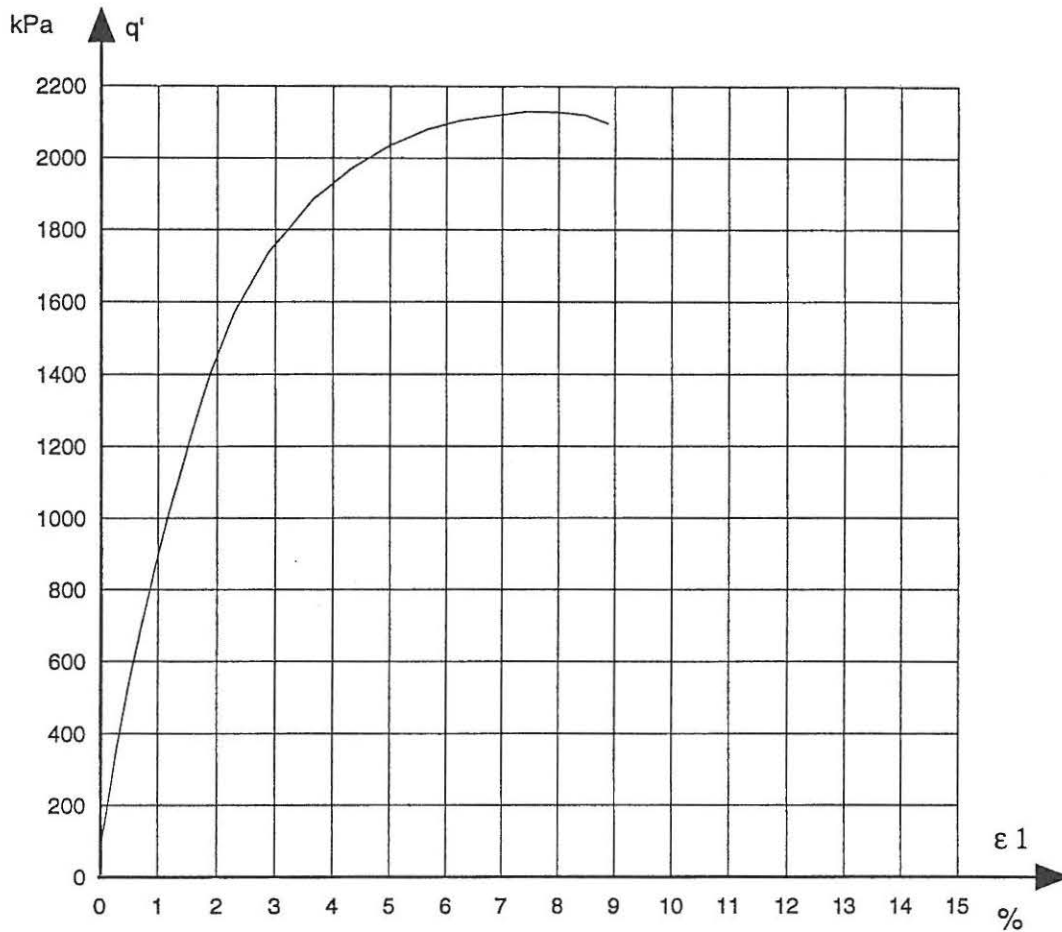


q'	p'	ϵ_1	ϵ_v
65.90	821.97	0.00	0.00
163.52	854.51	0.09	0.04
343.51	914.50	0.28	0.07
522.82	974.27	0.48	0.11
701.53	1033.84	0.71	0.16
879.11	1093.04	0.97	0.20
1055.49	1151.83	1.26	0.22
1230.63	1210.21	1.56	0.23
1403.35	1267.78	1.89	0.18
1573.11	1324.37	2.30	0.11
1736.80	1378.93	2.88	-0.04
1883.85	1427.95	3.65	-0.29
1970.72	1456.91	4.33	-0.54
2034.80	1478.27	4.99	-0.83
2079.26	1493.09	5.65	-1.12
2105.75	1501.92	6.26	-1.39
2118.02	1506.01	6.86	-1.73
2129.89	1509.96	7.44	-1.93
2127.61	1509.20	7.99	-2.20
2120.08	1506.69	8.49	-2.40
2096.61	1498.87	8.88	-2.58

Job:	Encl. No
Blokhus sand	9
Exc:	Check:
HSP	LB

Remark:

The specimen slipped out after failure.

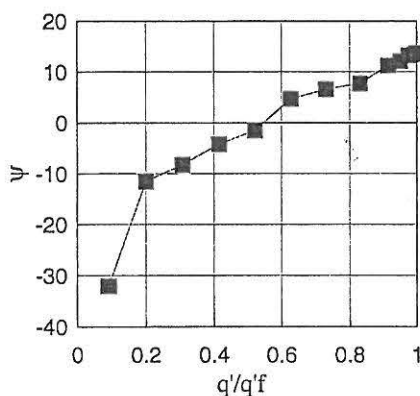
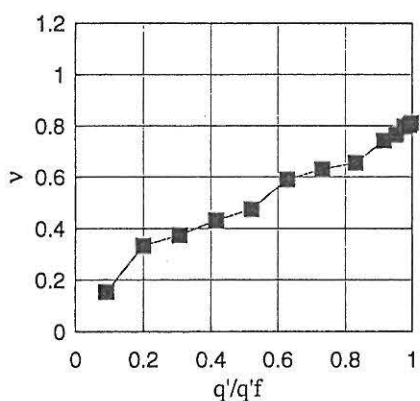


Job:	Encl. No
Blokhuis sand	10
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	21.9	At failure 0.647
		Grain density	2.65	
Calibration file	Date	Void ratio	0.603	
Loadtransducer 5001	27.06.73	Saturation	0.96	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	50-200 kPa
		ϵ_1	0.12 %
		ϵ_v	0.18 %
	2. Drained compression.		
		Deformation rate:	9.6 % ph

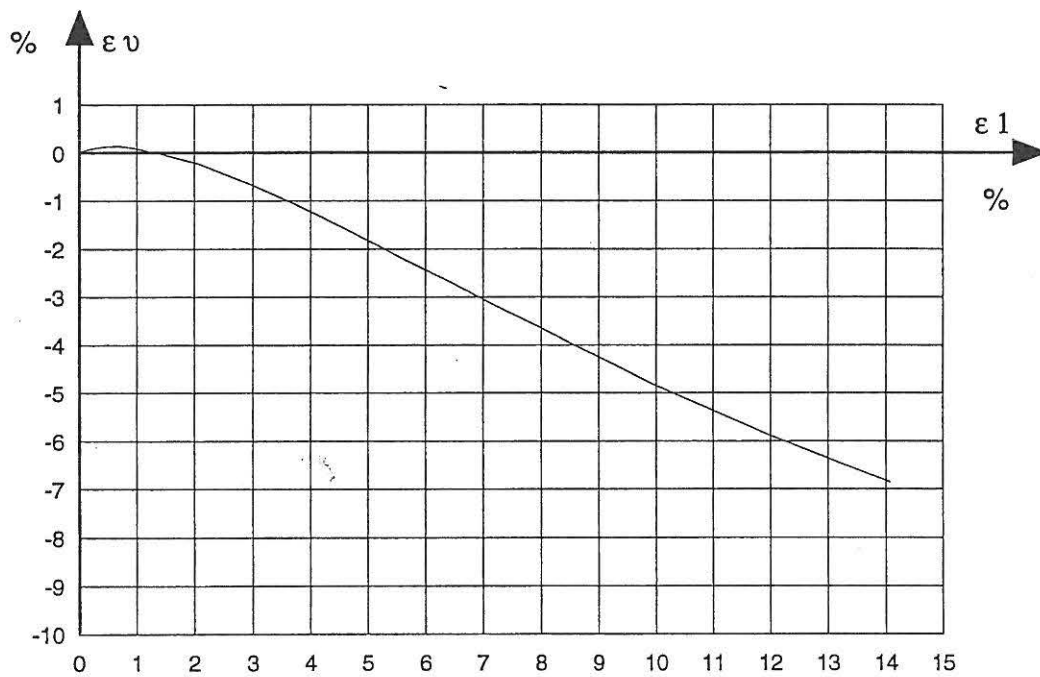
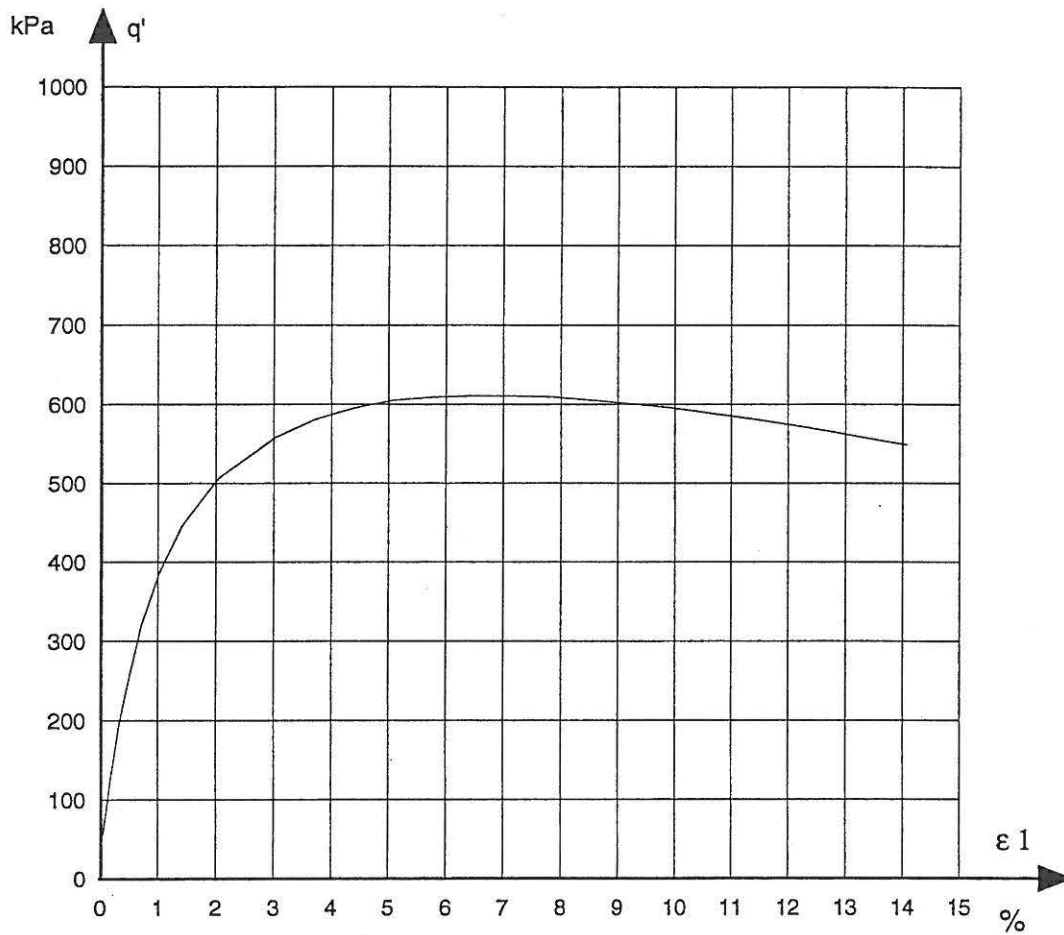
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	610.97 kPa	318.40 kPa
Mean normal stress	p'	403.16 kPa	305.63 kPa
Confining pressures	σ_3	199.50 kPa	199.50 kPa
Vertical strain	ϵ_1	6.46 %	0.70 %
Volumetric strain	ϵ_v	-2.71 %	0.14 %



q'	p'	ϵ_1	ϵ_v
55.60	218.03	0.00	0.00
55.56	218.02	0.05	0.04
121.62	240.04	0.16	0.07
187.51	262.00	0.30	0.11
253.13	283.88	0.49	0.13
318.40	305.63	0.70	0.14
382.82	327.11	1.00	0.09
446.00	348.17	1.42	-0.02
506.88	368.46	2.06	-0.22
558.43	385.64	3.03	-0.69
580.78	393.09	3.70	-1.05
595.66	398.05	4.40	-1.46
605.51	401.34	5.09	-1.88
609.25	402.58	5.78	-2.31
610.97	403.16	6.46	-2.71
610.58	403.03	7.15	-3.14
609.42	402.64	7.83	-3.54
605.58	401.36	8.52	-3.97
600.93	399.81	9.21	-4.37
595.91	398.14	9.91	-4.80
588.09	395.53	10.74	-5.23
582.79	393.76	11.30	-5.52
566.65	388.38	12.68	-6.21
548.99	382.50	14.08	-6.86

Job:	Encl. No
Blokhus sand	11
Exc:	Check:
HSP	LB

Remark:

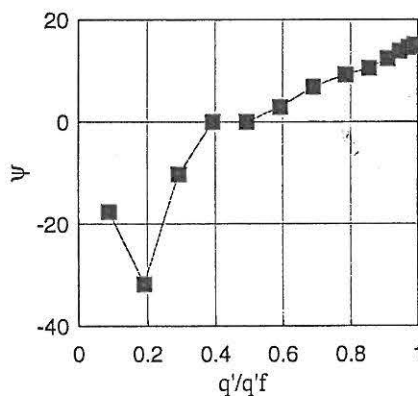
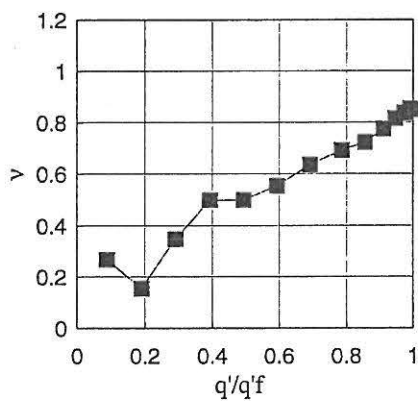


Job:	Encl. No
Blokhus sand	12
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.1	
Calibration file	Date	Void ratio	2.65	
Loadtransducer 5001	29.06.73	Saturation	0.578	
		Dimension H mm	0.97	
		Dimension D mm	72	
			70	0.628

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.	σ_3	50-200 kPa	
		ϵ_1	0.11 %	
		ϵ_v	0.18 %	
	2. Drained compression.			
		Deformation rate:	9.6 % ph	

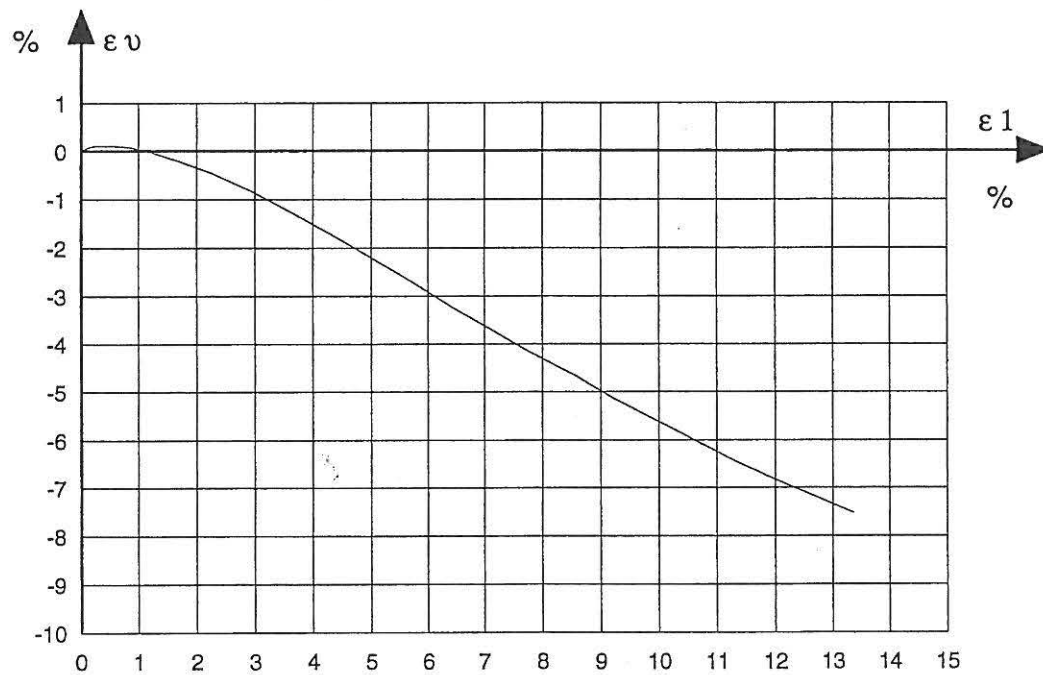
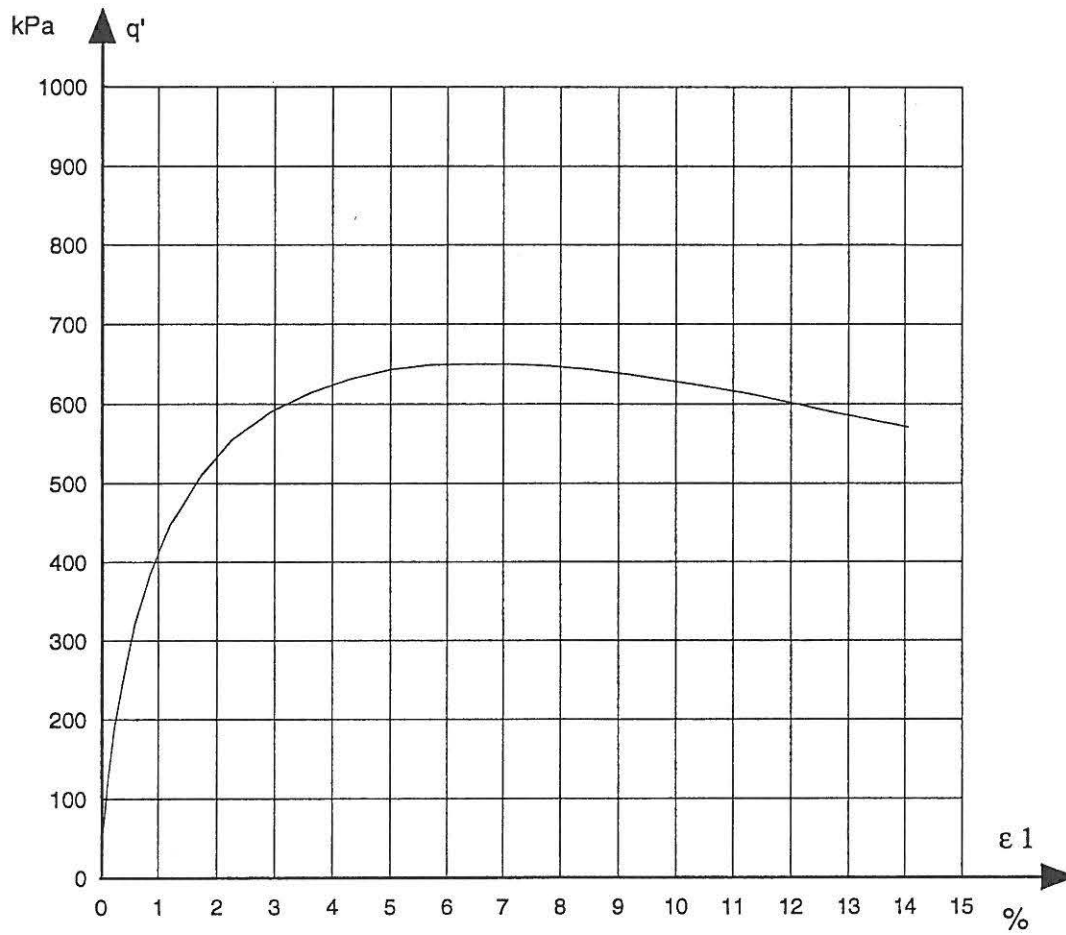
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	650.00	kPa	320.24	kPa
Mean normal stress	p'	416.67	kPa	306.75	kPa
Confining pressures	σ_3	200.00	kPa	200.00	kPa
Vertical strain	ϵ_1	6.40	%	0.59	%
Volumetric strain	ϵ_v	-3.21	%	0.11	%



q'	p'	ϵ_1	ϵ_v
57.10	219.03	0.00	0.00
57.15	219.05	0.04	0.02
123.26	241.09	0.12	0.07
189.23	263.08	0.24	0.11
254.88	284.96	0.40	0.11
320.24	306.75	0.59	0.11
384.93	328.31	0.85	0.08
448.50	349.50	1.21	-0.02
510.13	370.04	1.73	-0.22
555.14	385.05	2.26	-0.45
591.18	397.06	2.95	-0.83
614.71	404.90	3.63	-1.26
632.08	410.69	4.33	-1.73
643.23	414.41	5.02	-2.22
648.97	416.32	5.72	-2.71
650.00	416.67	6.40	-3.21
649.98	416.66	7.09	-3.68
647.65	415.88	7.76	-4.15
642.72	414.24	8.58	-4.67
636.78	412.26	9.20	-5.12
629.32	409.77	9.89	-5.56
621.48	407.16	10.59	-5.99
602.02	400.67	11.98	-6.82
581.20	393.73	13.37	-7.52

Job:	Encl. No
Blokhus sand	13
Exc:	Check:
HSP	LB

Remark:

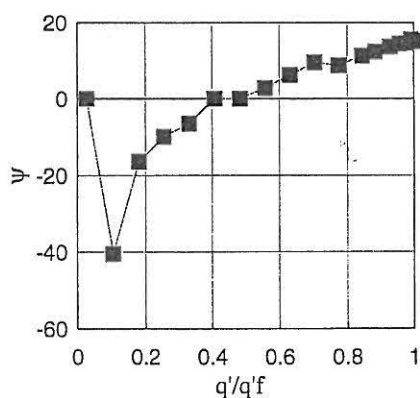
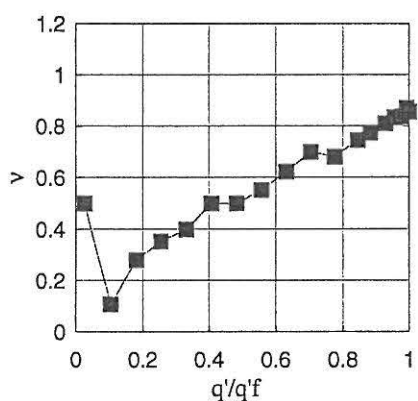


Job:	Blokhuis sand	Encl. No	14
Exc:	HSP	Check:	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.6	
Calibration file	Date	Void ratio	0.586	0.642
Loadtransducer 2003	05.07.73	Saturation	0.98	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	50-100 kPa
		ϵ_1	0.03 %
		ϵ_v	0.09 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

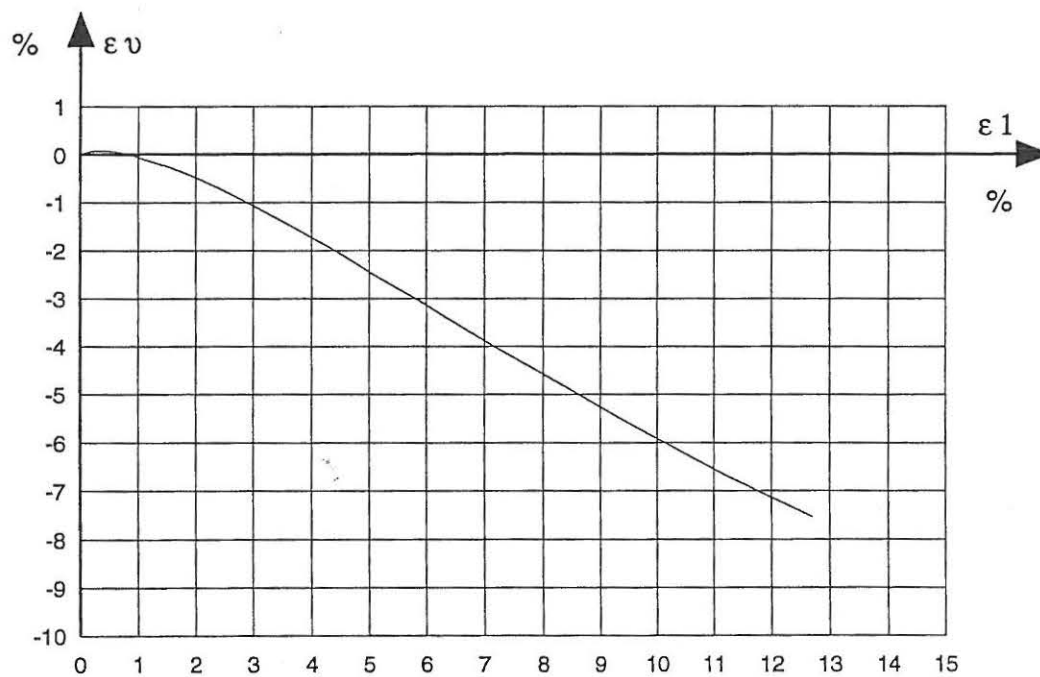
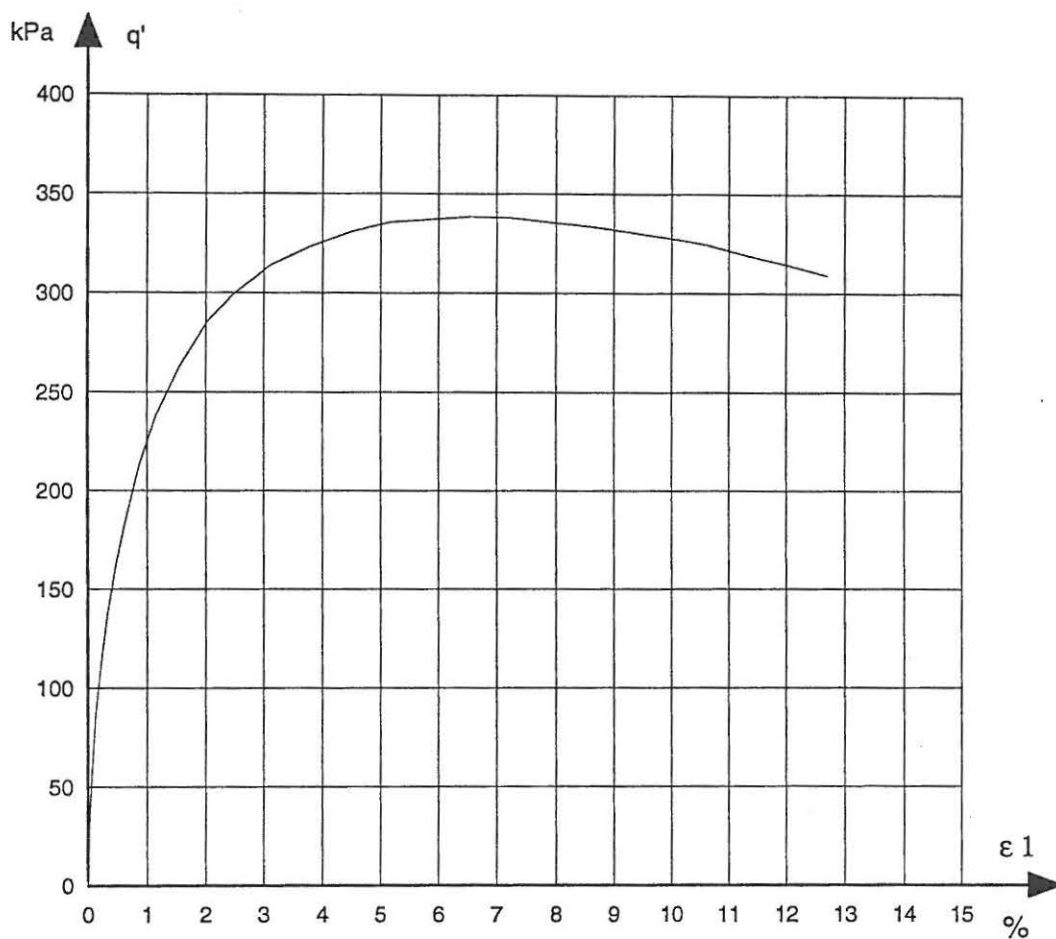
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	338.43	kPa	163.11	kPa
Mean normal stress	p'	212.81	kPa	154.37	kPa
Confining pressures	σ_3	100.00	kPa	100.00	kPa
Vertical strain	ϵ_1	6.54	%	0.48	%
Volumetric strain	ϵ_v	-3.54	%	0.07	%



q'	p'	ϵ_1	ϵ_v
9.30	103.10	0.00	0.00
9.27	103.09	0.01	0.00
35.02	111.67	0.03	0.02
60.75	120.25	0.07	0.04
86.46	128.82	0.13	0.05
112.11	137.37	0.22	0.07
137.66	145.89	0.33	0.07
163.11	154.37	0.48	0.07
188.40	162.80	0.65	0.05
213.40	171.13	0.87	0.00
238.02	179.34	1.14	-0.11
262.01	187.34	1.54	-0.25
285.23	195.08	2.02	-0.49
298.44	199.48	2.45	-0.72
313.78	204.59	3.12	-1.14
323.37	207.79	3.79	-1.59
330.75	210.25	4.48	-2.06
335.57	211.86	5.17	-2.56
338.43	212.81	6.54	-3.54
337.94	212.65	7.23	-4.04
333.60	211.20	8.59	-4.98
327.82	209.27	9.94	-5.88
319.01	206.34	11.33	-6.75
308.84	202.95	12.71	-7.54

Job:	Encl. No
Blokhus sand	15
Exc:	Check:
HSP	LB

Remark:

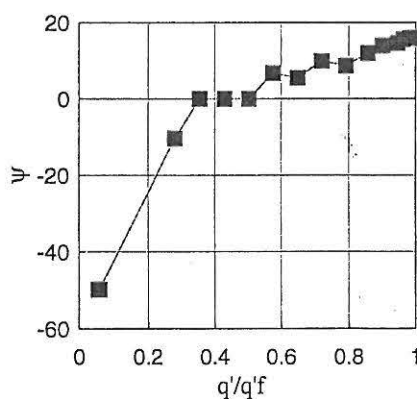
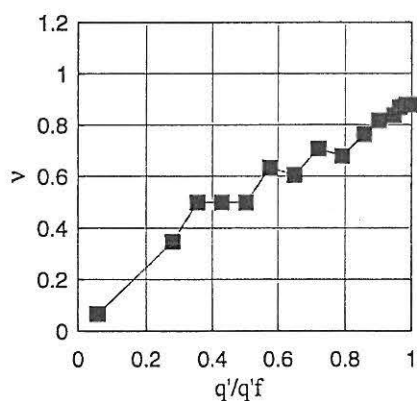


Job:	Encl. No
Blokhus sand	16
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.2	
Calibration file	Date	Void ratio	0.571	0.616
Loadtransducer 2003	23.07.73	Saturation	0.98	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	50-100 kPa
		ϵ_1	0.03 %
		ϵ_v	0.11 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

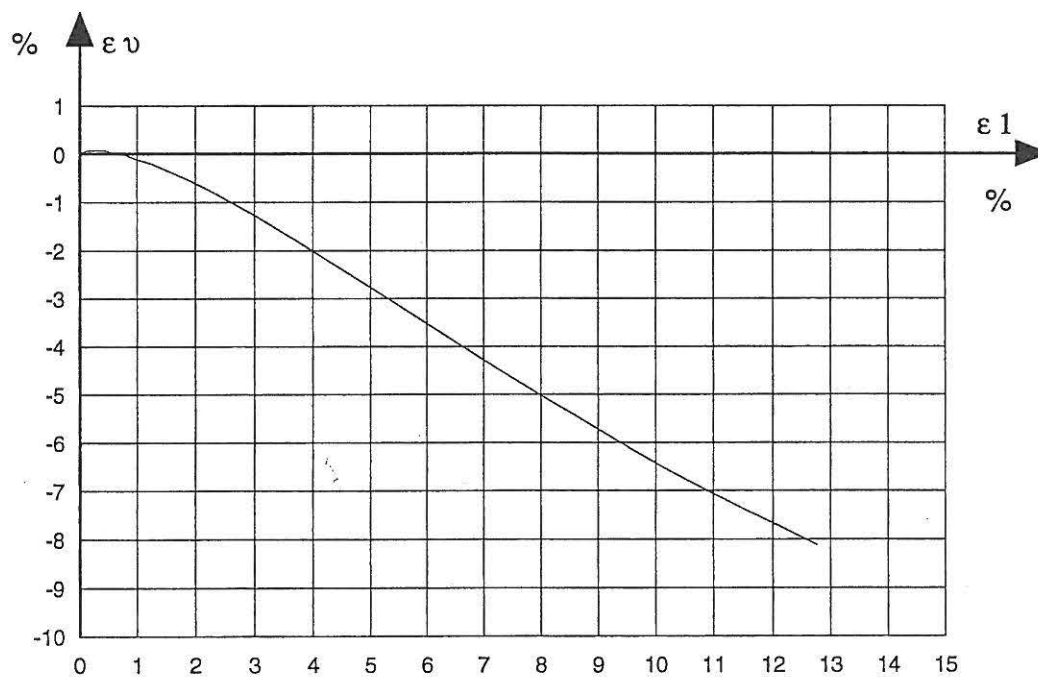
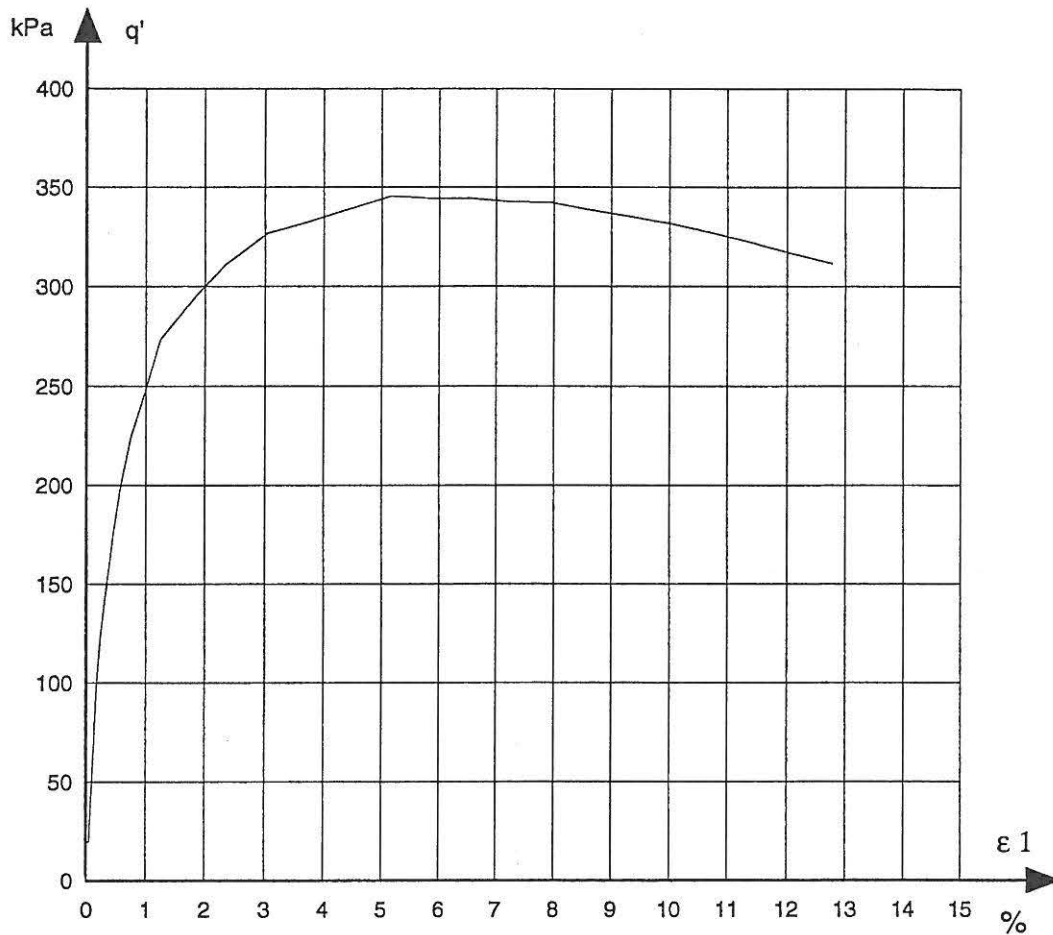
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	345.60	kPa	173.45	kPa
Mean normal stress	p'	215.20	kPa	157.82	kPa
Confining pressures	σ_3	100.00	kPa	100.00	kPa
Vertical strain	ϵ_1	5.16	%	0.43	%
Volumetric strain	ϵ_v	-2.89	%	0.07	%



q'	p'	ϵ_1	ϵ_v
19.60	106.53	0.00	0.00
19.57	106.52	0.04	0.04
96.74	132.25	0.16	0.07
122.38	140.79	0.23	0.07
147.96	149.32	0.32	0.07
173.45	157.82	0.43	0.07
198.76	166.25	0.57	0.04
223.91	174.64	0.74	0.00
248.54	182.85	1.00	-0.11
273.07	191.02	1.25	-0.20
295.63	198.54	1.87	-0.52
311.23	203.74	2.35	-0.83
326.91	208.97	3.05	-1.30
332.73	210.91	3.75	-1.82
339.30	213.10	4.46	-2.35
345.60	215.20	5.16	-2.89
344.53	214.84	5.85	-3.41
344.53	214.84	6.55	-3.93
342.53	214.18	7.93	-4.96
338.76	212.92	8.62	-5.45
331.89	210.63	10.00	-6.42
322.38	207.46	11.39	-7.31
316.71	205.57	12.08	-7.72
311.28	203.76	12.77	-8.12

Job:	Encl. No
Blokhus sand	17
Exc:	Check:
HSP	LB

Remark:

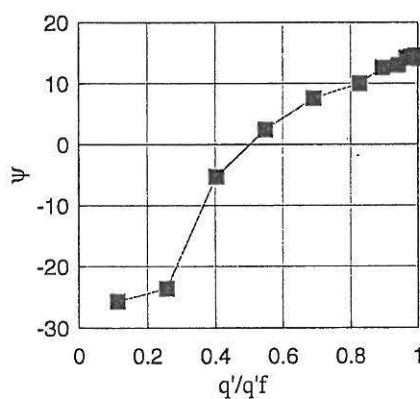
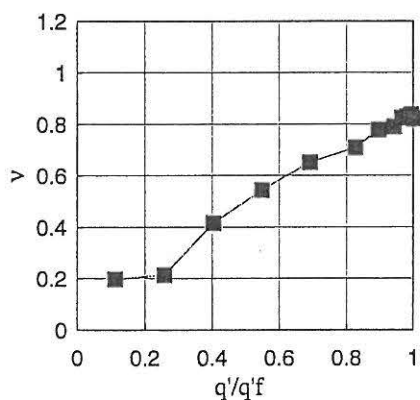


Job:	Encl. No
Blokhuis sand	18
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	22.6	At failure 0.663
		Grain density	2.65	
Calibration file	Date	Void ratio	0.608	
Loadtransducer 2003	24.07.73	Saturation	0.93	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	20-50 kPa
		ϵ_1	0.05 %
		ϵ_v	0.04 %
	2. Drained compression.		
Deformation rate:			9.6 % ph

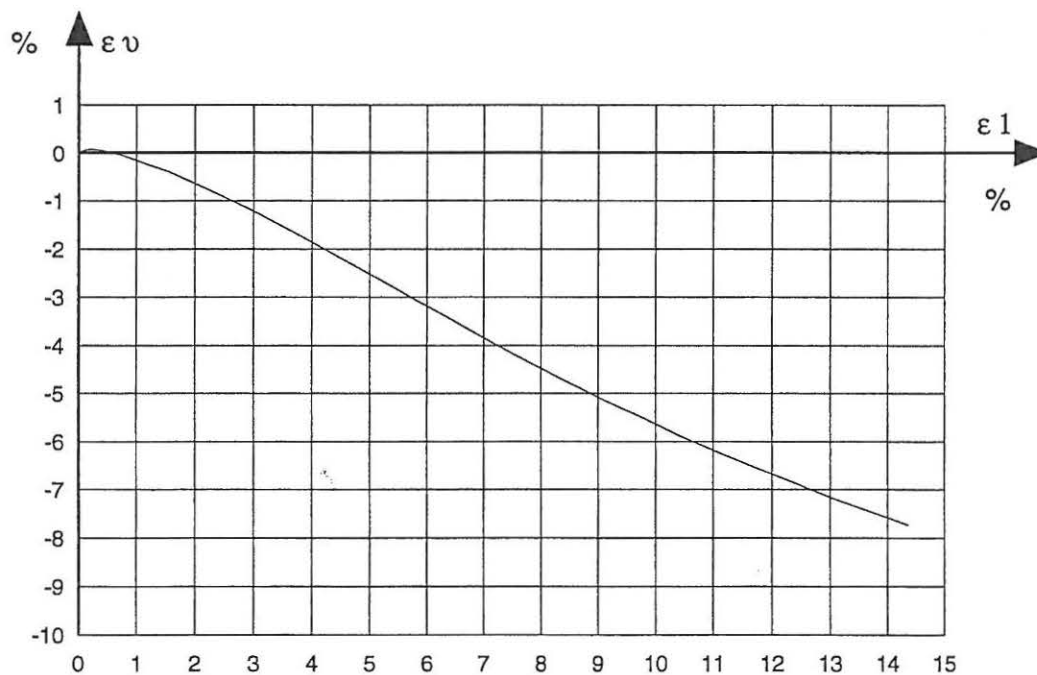
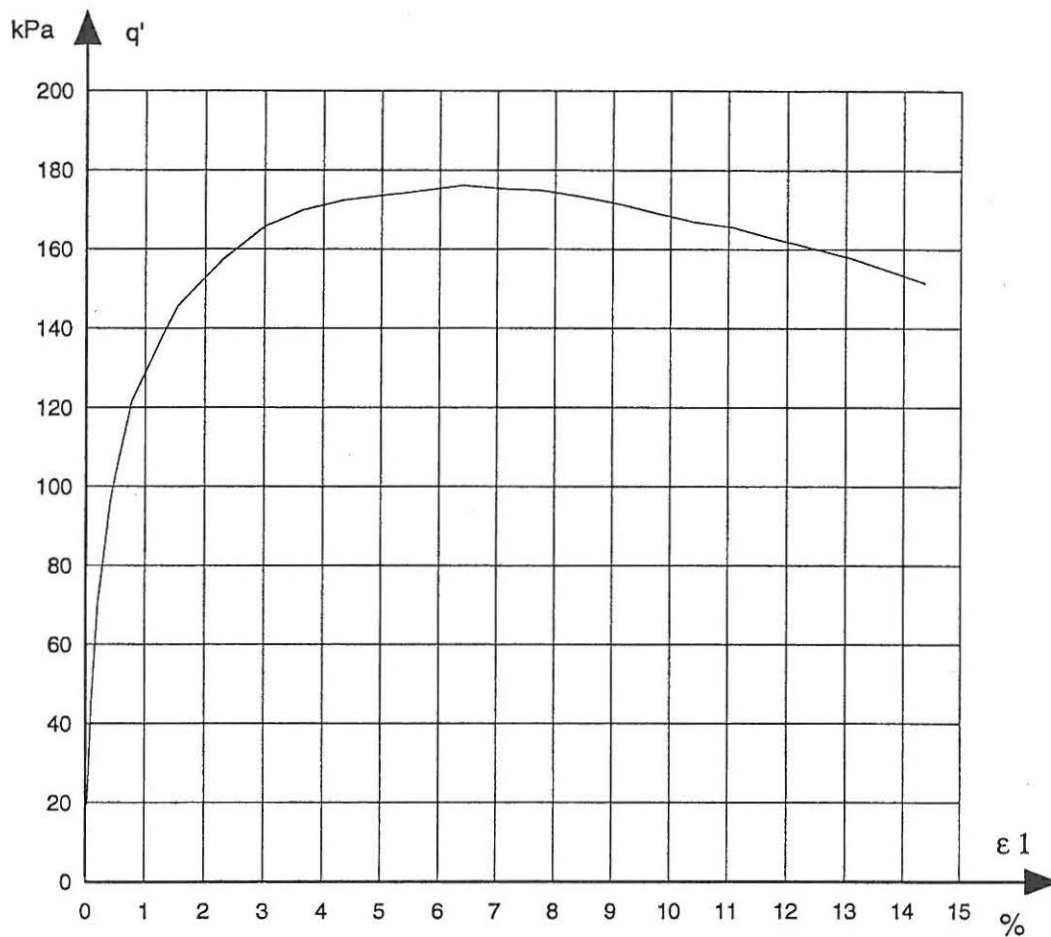
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	176.04	kPa	70.98	kPa
Mean normal stress	p'	108.68	kPa	73.66	kPa
Confining pressures	σ_3	50.00	kPa	50.00	kPa
Vertical strain	ϵ_1	6.41	%	0.20	%
Volumetric strain	ϵ_v	-3.45	%	0.07	%



q'	p'	ϵ_1	ϵ_v
19.60	56.53	0.00	0.00
19.57	56.52	0.03	0.02
45.30	65.10	0.09	0.05
70.98	73.66	0.20	0.07
96.48	82.16	0.40	0.05
121.58	90.53	0.76	-0.05
145.49	98.50	1.54	-0.38
157.55	102.52	2.31	-0.81
165.75	105.25	3.00	-1.21
169.91	106.64	3.67	-1.64
172.45	107.48	4.35	-2.09
173.61	107.87	5.04	-2.54
174.85	108.28	5.73	-3.01
176.04	108.68	6.41	-3.45
175.28	108.43	7.09	-3.90
174.84	108.28	7.76	-4.33
173.33	107.78	8.43	-4.75
171.53	107.18	9.09	-5.14
169.15	106.38	9.76	-5.50
166.88	105.63	10.42	-5.88
165.53	105.18	11.09	-6.23
162.89	104.30	11.74	-6.55
157.83	102.61	13.07	-7.20
151.43	100.48	14.38	-7.74

Job:	Encl. No
Blokhus sand	19
Exc:	Check:
HSP	LB

Remark:

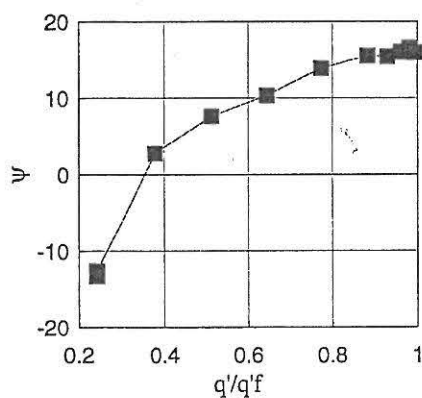
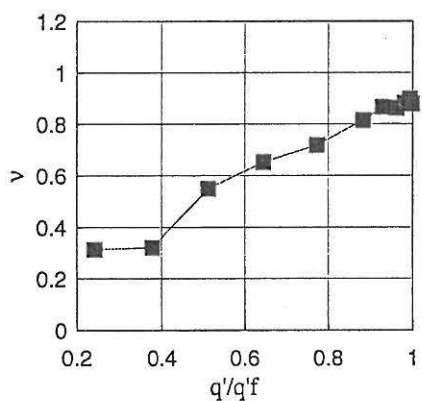


Job:	Encl. No
Blokhus sand	20
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.5	
Calibration file	Date	Void ratio	0.574	0.639
Loadtransducer	25.07.73	Saturation	0.99	
2003		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	20-50 kPa
		ϵ_1	0.04 %
		ϵ_v	0.05 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

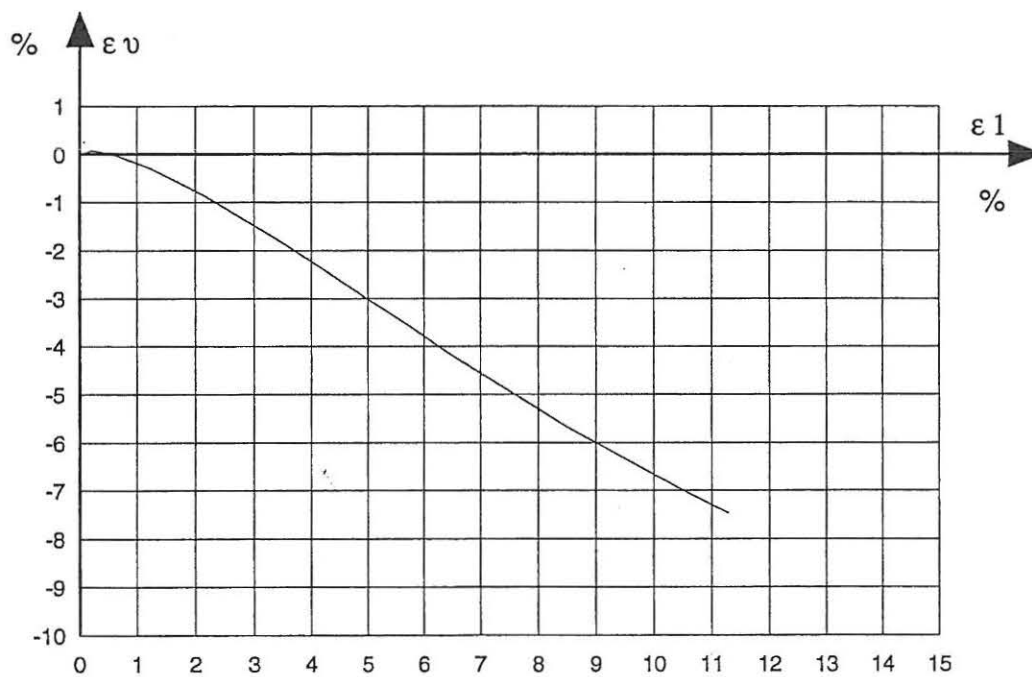
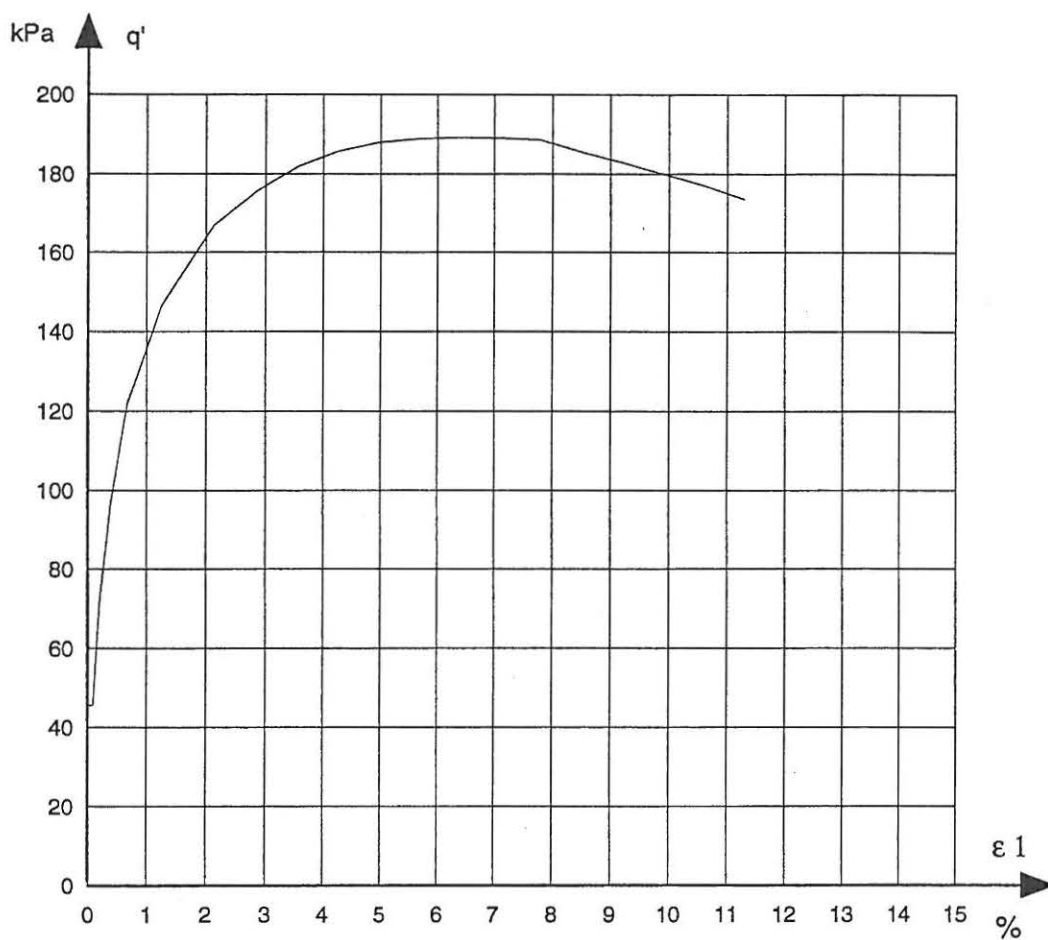
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	189.14	kPa	71.24	kPa
Mean normal stress	p'	113.05	kPa	73.75	kPa
Confining pressures	σ_3	50.00	kPa	50.00	kPa
Vertical strain	ϵ_1	6.40	%	0.20	%
Volumetric strain	ϵ_v	-4.11	%	0.07	%



q'	p'	ϵ_1	ϵ_v
45.60	65.20	0.00	0.00
45.55	65.18	0.10	0.04
71.24	73.75	0.20	0.07
96.77	82.26	0.37	0.05
121.97	90.66	0.67	-0.04
146.31	98.77	1.25	-0.29
166.88	105.63	2.13	-0.85
175.61	108.54	2.85	-1.37
181.73	110.58	3.54	-1.88
185.61	111.87	4.25	-2.42
187.92	112.64	4.98	-3.00
188.82	112.94	5.69	-3.54
189.14	113.05	6.40	-4.11
188.98	112.99	7.10	-4.64
188.58	112.86	7.79	-5.14
185.54	111.85	8.49	-5.67
182.85	110.95	9.20	-6.14
179.94	109.98	9.90	-6.60
176.97	108.99	10.60	-7.06
173.56	107.85	11.29	-7.47

Job:	Encl. No
Blokhus sand	21
Exc:	Check:
HSP	LB

Remark:

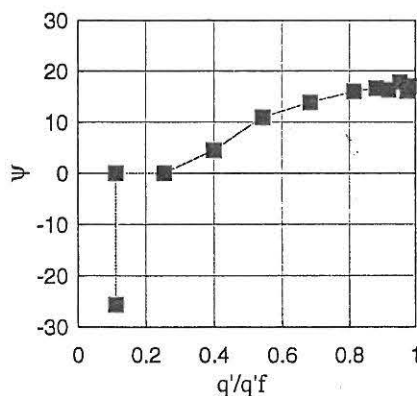
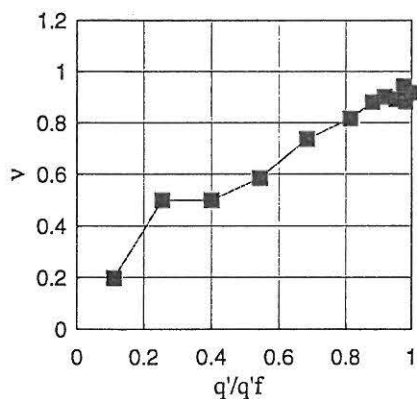


Job:	Encl. No
Blokhus sand	22
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.8	
Calibration file	Date	Void ratio	0.579	0.644
Loadtransducer 2003	26.07.73	Saturation	1	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-20 kPa
		ϵ_1	0.02 %
		ϵ_v	0.04 %
	2.Drained compression.		
	Deformation rate:		9.6 % ph

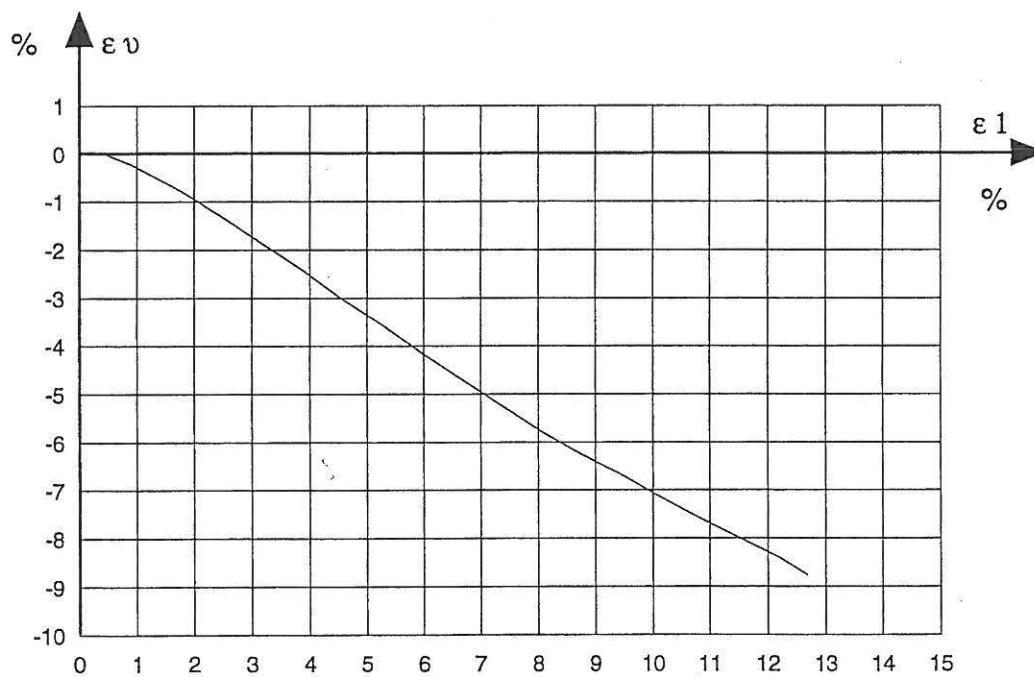
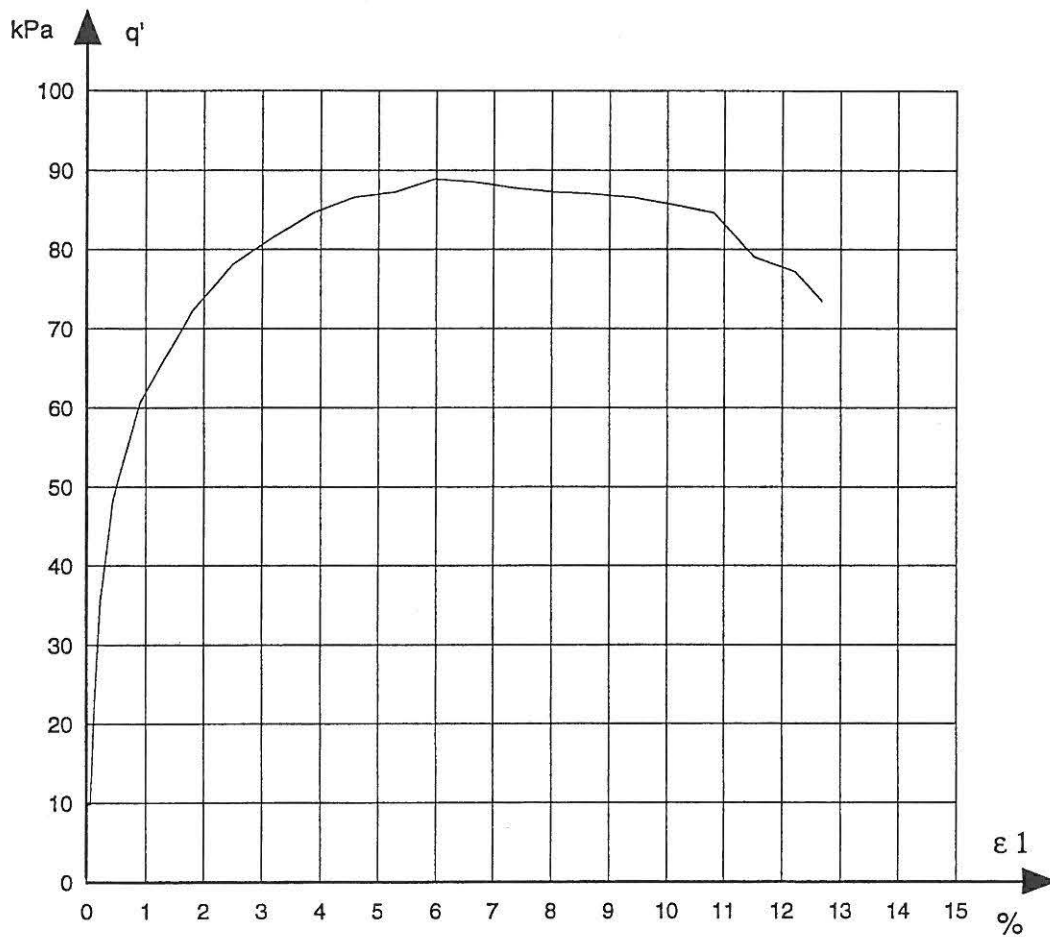
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	88.85	kPa	35.47	kPa
Mean normal stress	p'	49.62	kPa	31.82	kPa
Confining pressures	σ_3	20.00	kPa	20.00	kPa
Vertical strain	ϵ_1	5.97	%	0.22	%
Volumetric strain	ϵ_v	-4.15	%	0.04	%



q'	p'	ϵ_1	ϵ_v
9.80	23.27	0.00	0.00
9.78	23.26	0.06	0.04
22.64	27.55	0.13	0.04
35.47	31.82	0.22	0.04
48.20	36.07	0.43	0.00
60.61	40.20	0.89	-0.22
72.25	44.08	1.80	-0.79
78.18	46.06	2.51	-1.34
81.54	47.18	3.19	-1.88
84.63	48.21	3.88	-2.42
86.58	48.86	4.57	-3.03
87.23	49.08	5.28	-3.57
88.85	49.62	5.97	-4.15
88.48	49.49	6.67	-4.69
87.73	49.24	7.36	-5.23
87.21	49.07	8.05	-5.77
86.97	48.99	8.74	-6.24
86.53	48.84	9.43	-6.68
85.58	48.53	10.12	-7.15
84.62	48.21	10.81	-7.58
79.08	46.36	11.51	-7.99
77.24	45.75	12.21	-8.41
73.38	44.46	12.69	-8.77

Job:	Encl. No
Blokhus sand	23
Exc:	Check:
HSP	LB

Remark:

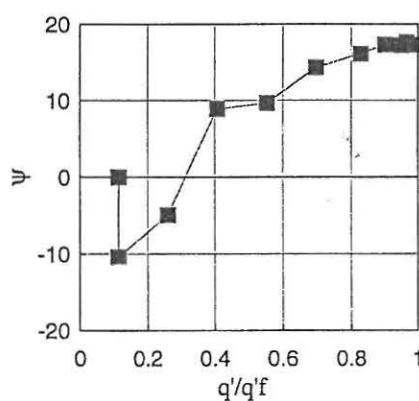
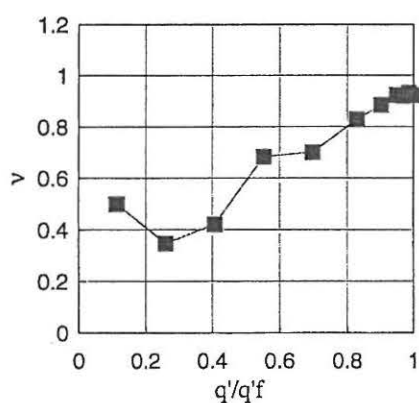


Job:	Encl. No
Blokhus sand	24
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21	
Calibration file	Date	Void ratio	2.65	
		Saturation	0.572	0.645
Loadtransducer	27.07.73	Dimension H mm	0.97	
2003		D mm	72	
			70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-20 kPa
		ϵ_1	0.02 %
		ϵ_v	0.07 %
	2. Drained compression.		
Deformation rate:		9.6 % ph	

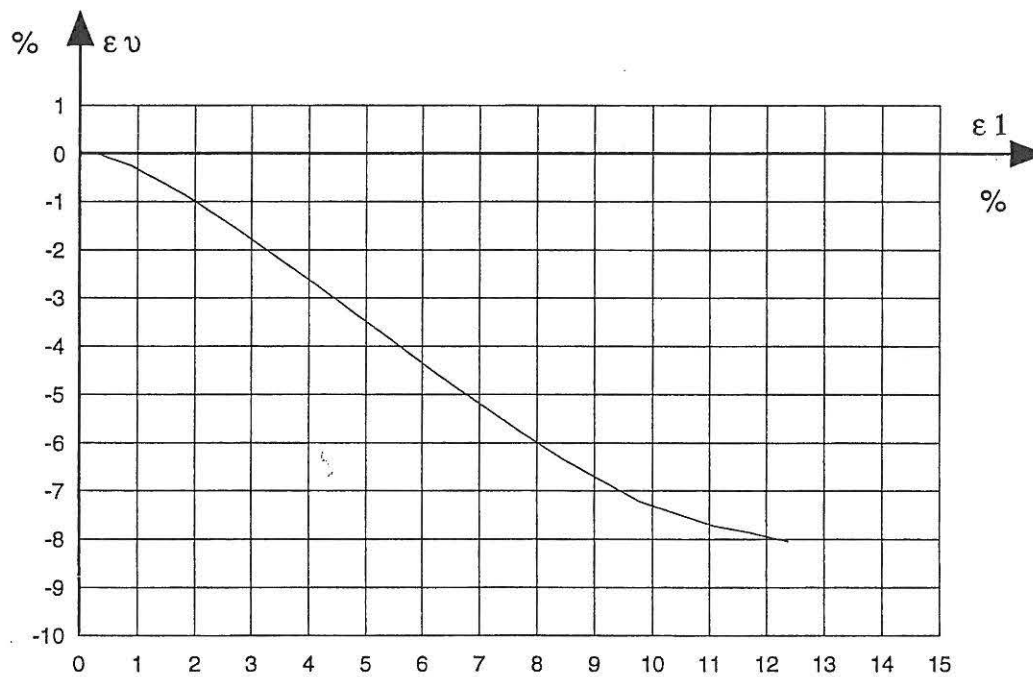
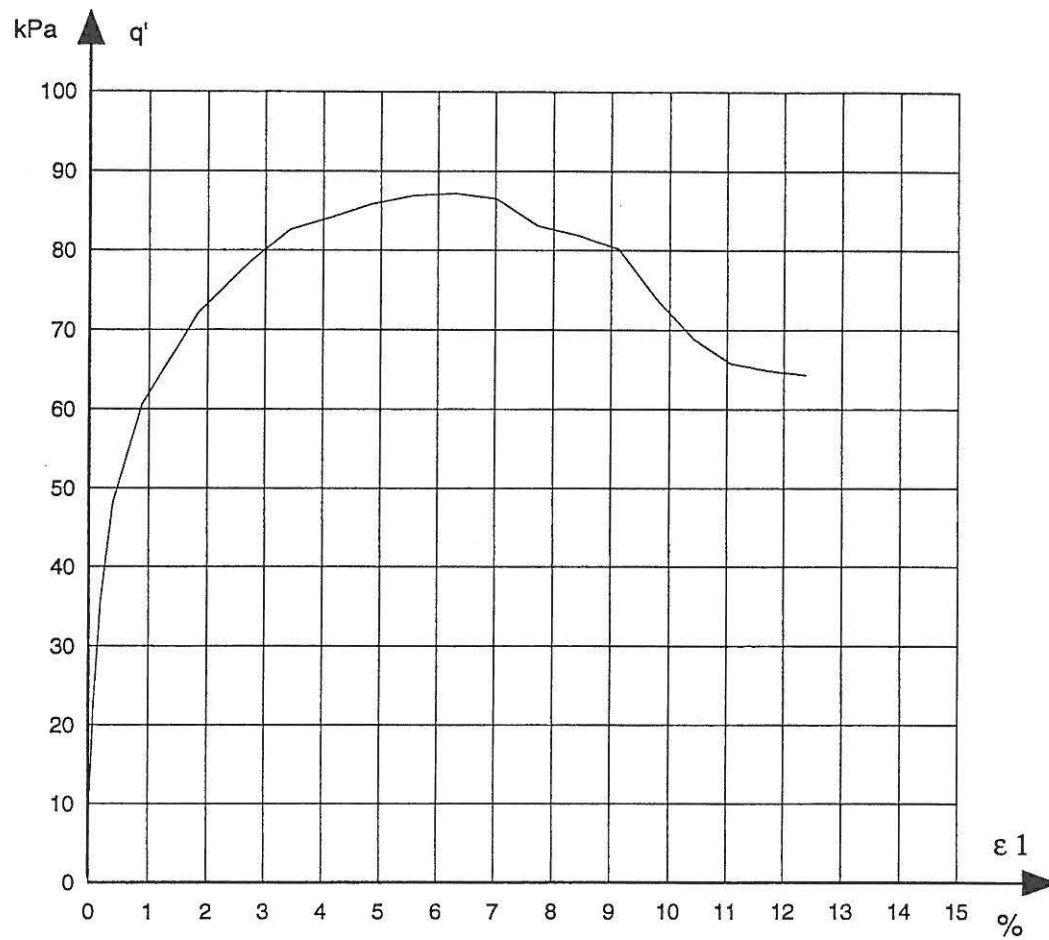
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	87.17	kPa	35.48	kPa
Mean normal stress	p'	49.06	kPa	31.83	kPa
Confining pressures	σ_3	20.00	kPa	20.00	kPa
Vertical strain	ϵ_1	6.31	%	0.19	%
Volumetric strain	ϵ_v	-4.62	%	0.04	%



q'	p'	ϵ_1	ϵ_v
9.80	23.27	0.00	0.00
9.78	23.26	0.02	0.00
22.65	27.55	0.08	0.02
35.48	31.83	0.19	0.04
48.21	36.07	0.39	-0.04
60.61	40.20	0.88	-0.23
72.17	44.06	1.84	-0.87
78.53	46.18	2.73	-1.55
82.62	47.54	3.43	-2.15
84.19	48.06	4.14	-2.74
85.82	48.61	4.85	-3.36
86.91	48.97	5.57	-3.97
87.17	49.06	6.31	-4.62
86.49	48.83	7.02	-5.20
83.12	47.71	7.72	-5.77
81.89	47.30	8.42	-6.32
80.22	46.74	9.11	-6.78
73.89	44.63	9.78	-7.22
68.90	42.97	10.42	-7.47
65.82	41.94	11.07	-7.72
64.91	41.64	11.72	-7.87
64.32	41.44	12.38	-8.05

Job:	Encl. No
Blokhus sand	25
Exc:	Check:
HSP	LB

Remark:

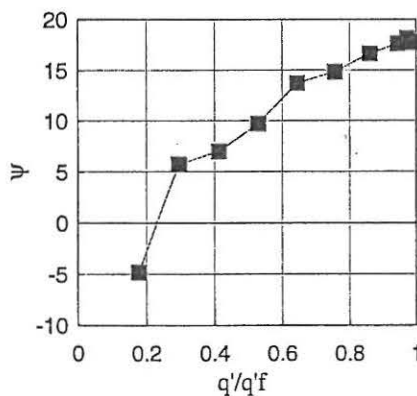
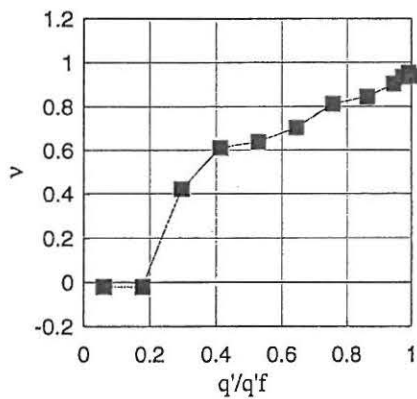


Job:	Encl. No
Blokhuis sand	26
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.7	
Calibration file	Date	Void ratio	0.581	0.652
Loadtransducer	30.07.73	Saturation	0.99	
2003		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	5-10 kPa
		ϵ_1	0.00 %
		ϵ_v	0.05 %
	2. Drained compression.		
	Deformation rate:		9.6 % ph

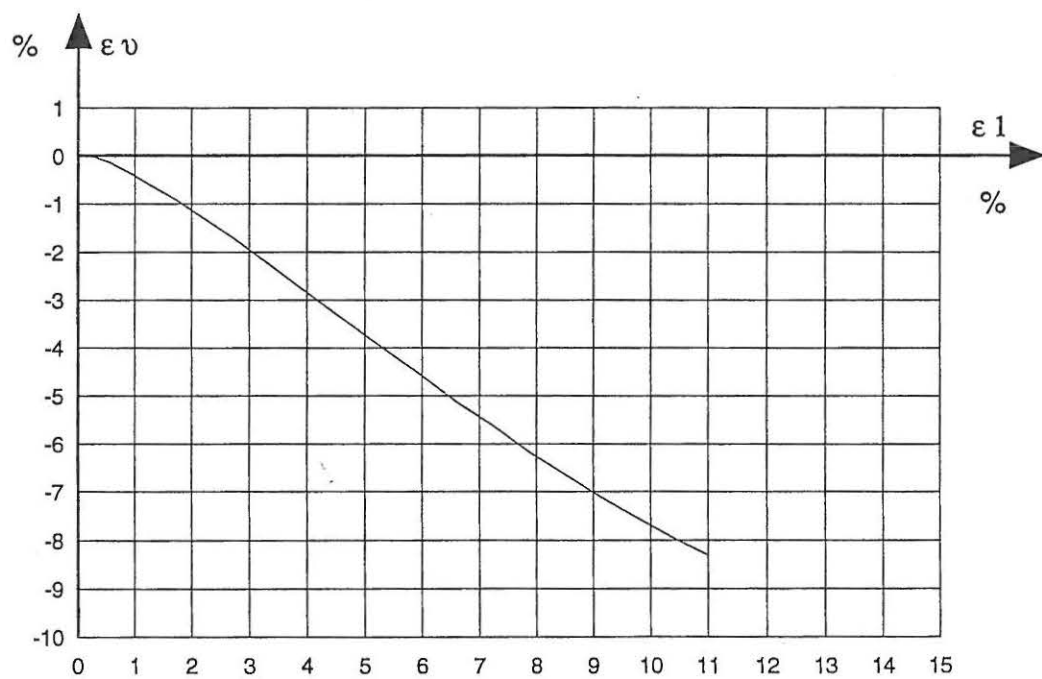
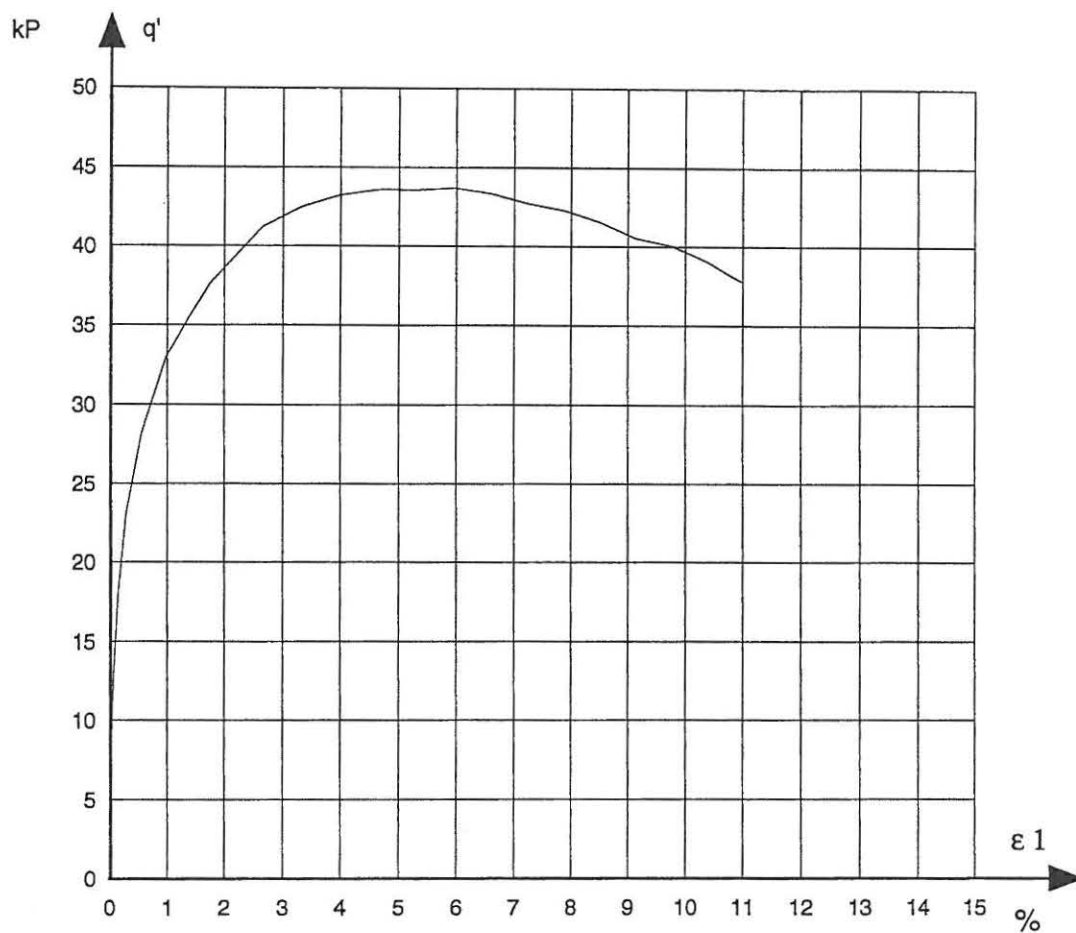
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	43.68	kPa	12.87	kPa
Mean normal stress	p'	24.56	kPa	14.29	kPa
Confining pressures	σ_3	10.00	kPa	10.00	kPa
Vertical strain	ϵ_1	5.96	%	0.07	%
Volumetric strain	ϵ_v	-4.55	%	0.04	%



q'	p'	ϵ_1	ϵ_v
2.60	10.87	0.00	0.00
2.58	10.86	0.01	0.01
7.73	12.58	0.03	0.03
12.87	14.29	0.07	0.04
18.00	16.00	0.16	0.02
23.11	17.70	0.29	-0.02
28.13	19.38	0.55	-0.13
33.01	21.00	0.99	-0.40
37.61	22.54	1.75	-0.92
41.22	23.74	2.66	-1.66
42.46	24.15	3.33	-2.24
43.21	24.40	4.00	-2.85
43.58	24.53	4.66	-3.43
43.56	24.52	5.32	-4.01
43.68	24.56	5.96	-4.55
43.33	24.44	6.60	-5.12
42.69	24.23	7.25	-5.63
42.27	24.09	7.88	-6.17
41.53	23.84	8.51	-6.64
40.50	23.50	9.14	-7.11
40.06	23.35	9.76	-7.54
39.09	23.03	10.38	-7.94
37.80	22.60	10.98	-8.30

Job:	Encl. No
Blokhus sand	27
Exc:	Check:
HSP	LB

Remark:

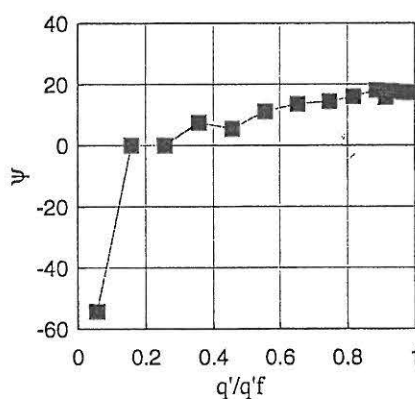
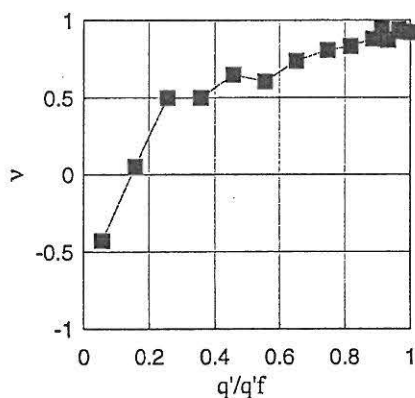


Job:	Encl. No
Blokhus sand	28
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	22.1	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.590		
Loadtransducer 2003	31.07.73	Saturation	0.99		
		Dimension H mm	72		
		D mm	70		

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	5-10 kPa
		ϵ_1	0.01 %
		ϵ_v	0.1 %
	2. Drained compression.		
Deformation rate:			9.6 % ph

		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	51.22	kPa	18.26	kPa
Mean normal stress	p'	27.07	kPa	16.09	kPa
Confining pressures	σ_3	10.00	kPa	10.00	kPa
Vertical strain	ϵ_1	5.64	%	0.16	%
Volumetric strain	ϵ_v	-4.08	%	0.04	%

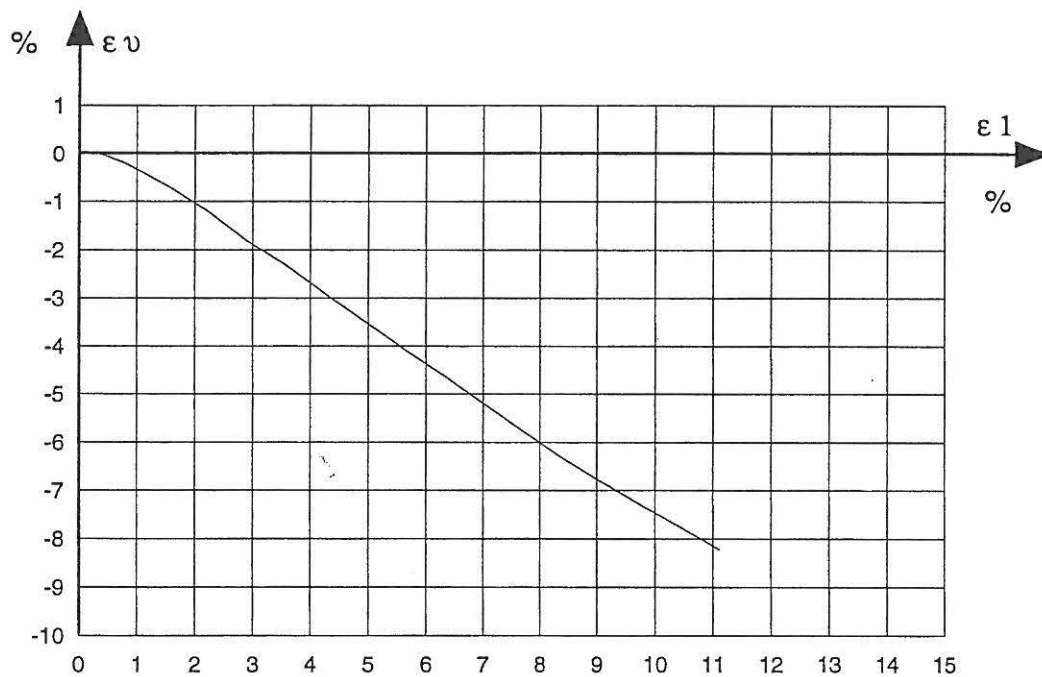
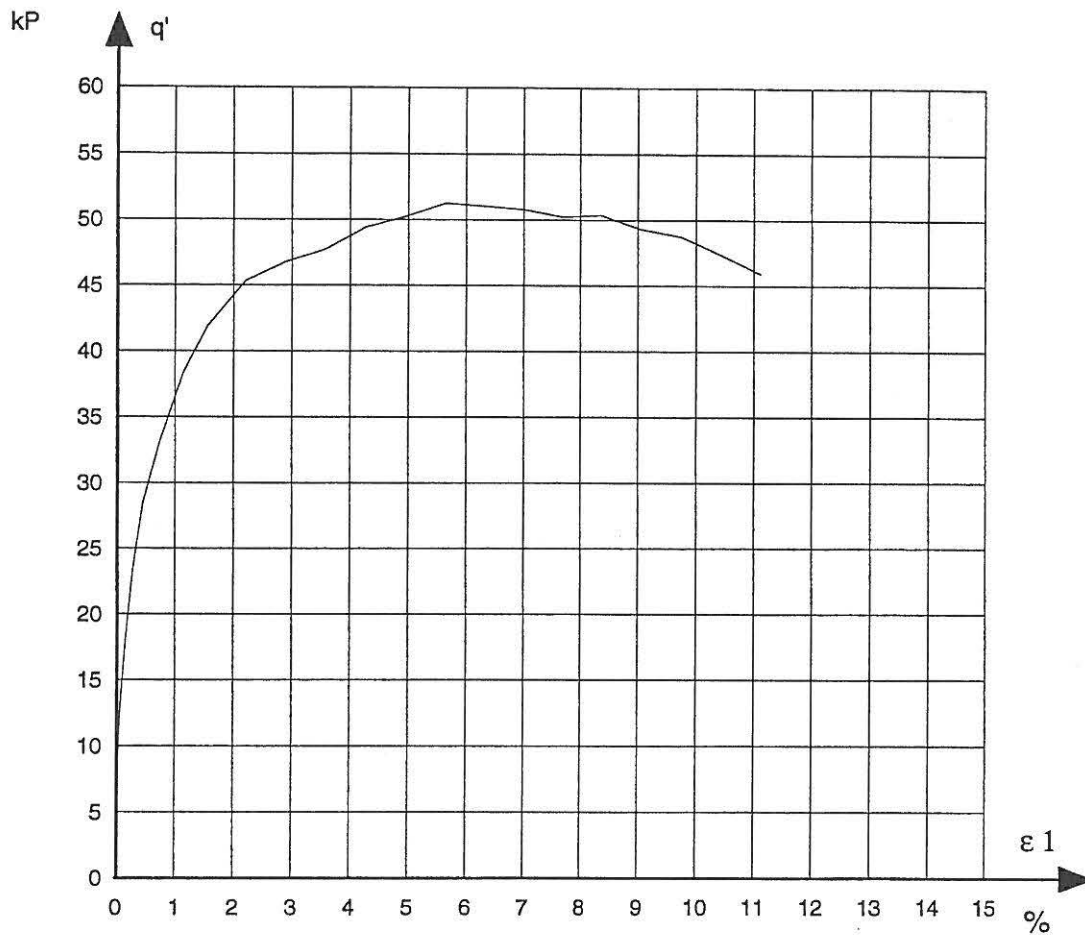


q'	p'	ϵ_1	ϵ_v
2.80	10.93	0.00	0.00
2.83	10.94	0.01	0.02
7.98	12.66	0.03	0.04
13.13	14.38	0.08	0.04
18.26	16.09	0.16	0.04
23.37	17.79	0.28	0.00
28.44	19.48	0.45	-0.04
33.42	21.14	0.75	-0.18
38.28	22.76	1.14	-0.42
41.85	23.95	1.55	-0.69
45.34	25.11	2.21	-1.19
46.77	25.59	2.89	-1.80
47.71	25.90	3.58	-2.31
49.38	26.46	4.27	-2.92
50.23	26.74	4.95	-3.50
51.22	27.07	5.64	-4.08
51.01	27.00	6.32	-4.62
50.77	26.92	7.00	-5.20
50.22	26.74	7.68	-5.76
50.35	26.78	8.37	-6.30
49.29	26.43	9.05	-6.80
48.74	26.25	9.74	-7.29
47.34	25.78	10.43	-7.76
45.89	25.30	11.12	-8.23

Job:	Encl. No
Blokhus sand	29
Exc:	Check:
HSP	LB

Remark:

Bad saturation - membrane seems dry

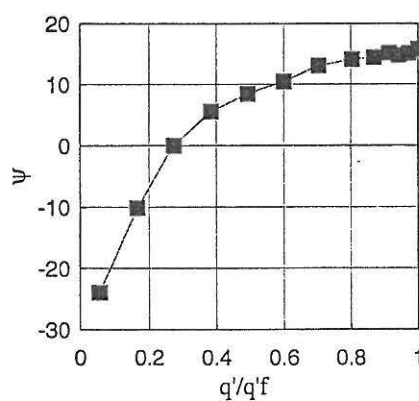
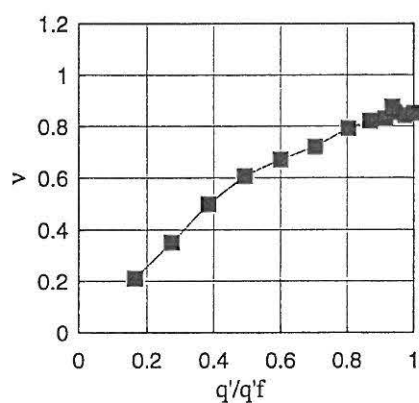


Job:	Encl. No
Blokhuss sand	30
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	23.6	At failure 0.685
		Grain density	2.65	
Calibration file	Date	Void ratio	0.636	
Loadtransducer 2003	31.07.73	Saturation	0.98	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	5-10 kPa
		ϵ_1	0.01 %
		ϵ_v	0.05 %
	2. Drained compression.	Deformation rate: 9.6 % ph	

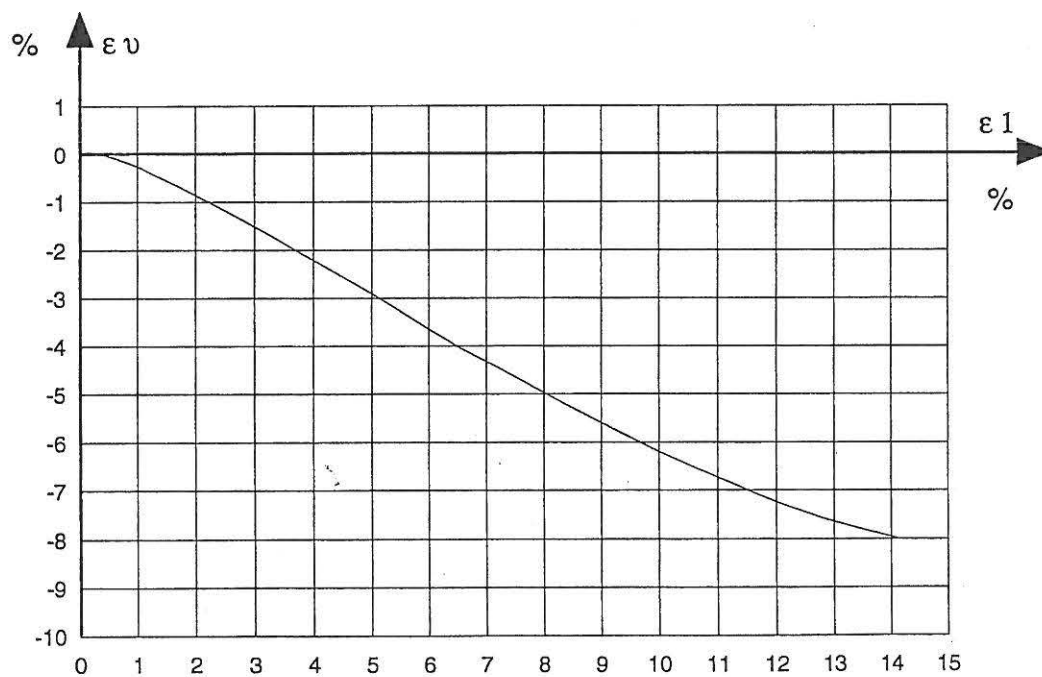
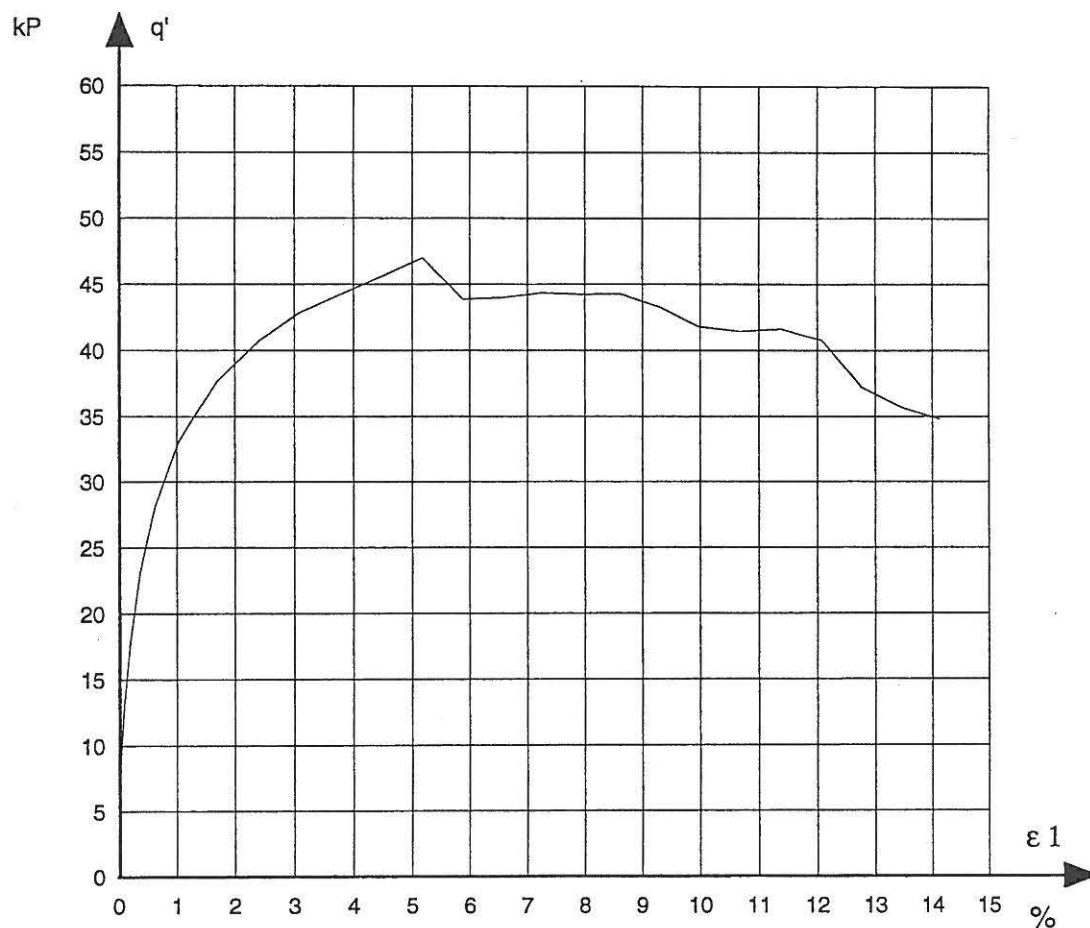
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	46.97	kPa	18.00	kPa
Mean normal stress	p'	25.66	kPa	16.00	kPa
Confining pressures	σ_3	10.00	kPa	10.00	kPa
Vertical strain	ϵ_1	5.18	%	0.19	%
Volumetric strain	ϵ_v	-3.03	%	0.04	%



q'	p'	ϵ_1	ϵ_v
2.60	10.87	0.00	0.00
7.72	12.57	0.03	0.02
12.87	14.29	0.09	0.04
18.00	16.00	0.19	0.04
23.09	17.70	0.36	0.00
28.12	19.37	0.62	-0.09
33.04	21.01	1.03	-0.27
37.71	22.57	1.71	-0.67
40.76	23.59	2.41	-1.12
42.85	24.28	3.09	-1.57
44.24	24.75	3.78	-2.06
45.62	25.21	4.49	-2.54
46.97	25.66	5.18	-3.03
43.86	24.62	5.87	-3.55
43.99	24.66	6.56	-4.04
44.35	24.78	7.24	-4.48
44.24	24.75	7.93	-4.93
44.27	24.76	8.61	-5.36
43.28	24.43	9.29	-5.77
41.78	23.93	9.99	-6.19
41.44	23.81	10.68	-6.57
41.61	23.87	11.39	-6.93
37.22	22.41	12.76	-7.56
34.80	21.60	14.14	-7.99

Job: Blokhus sand	Encl. No 31
Exc: HSP	Check: LB

Remark:

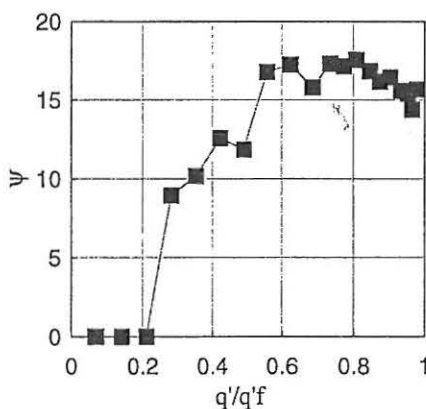
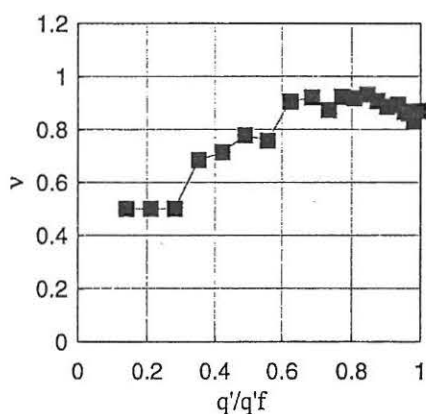


Job:	Encl. No
Blokhus sand	32
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	22.1	
Calibration file	Date	Void ratio	0.603	0.723
Loadtransducer 181	07.08.73	Saturation	0.98	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	5-5 kPa
		ϵ_1	0.00 %
		ϵ_v	0.00 %
2. Drained compression.			
Deformation rate:		9.6 % ph	

		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'		9.23 kPa
Mean normal stress	p'		8.08 kPa
Confining pressures	σ_3		5.00 kPa
Vertical strain	ϵ_1		0.12 %
Volumetric strain	ϵ_v		0.00 %

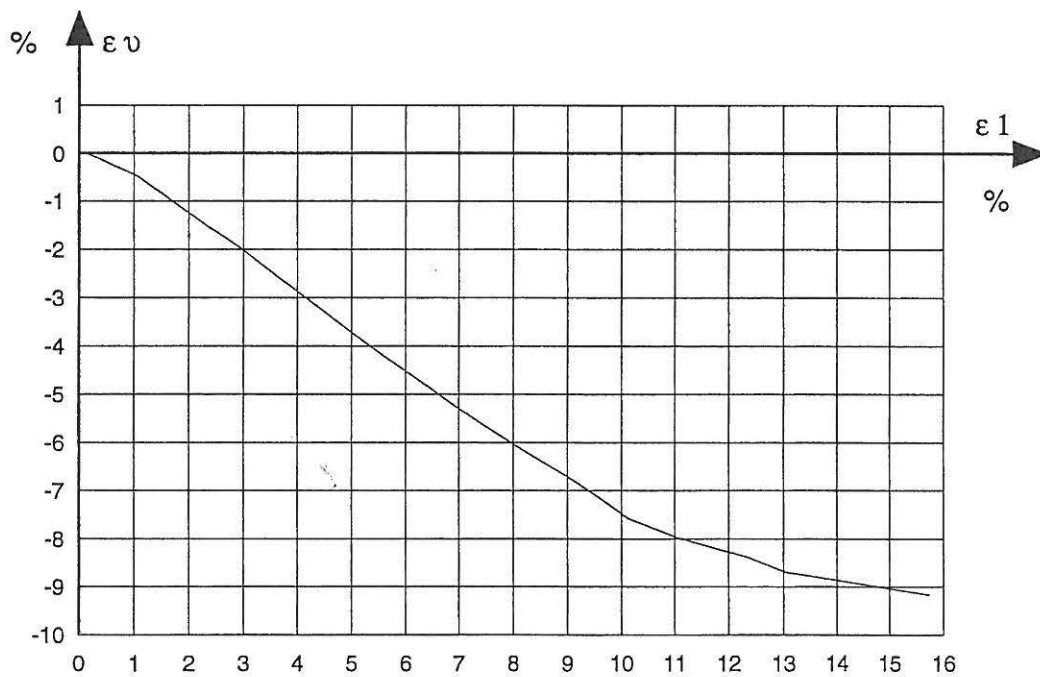
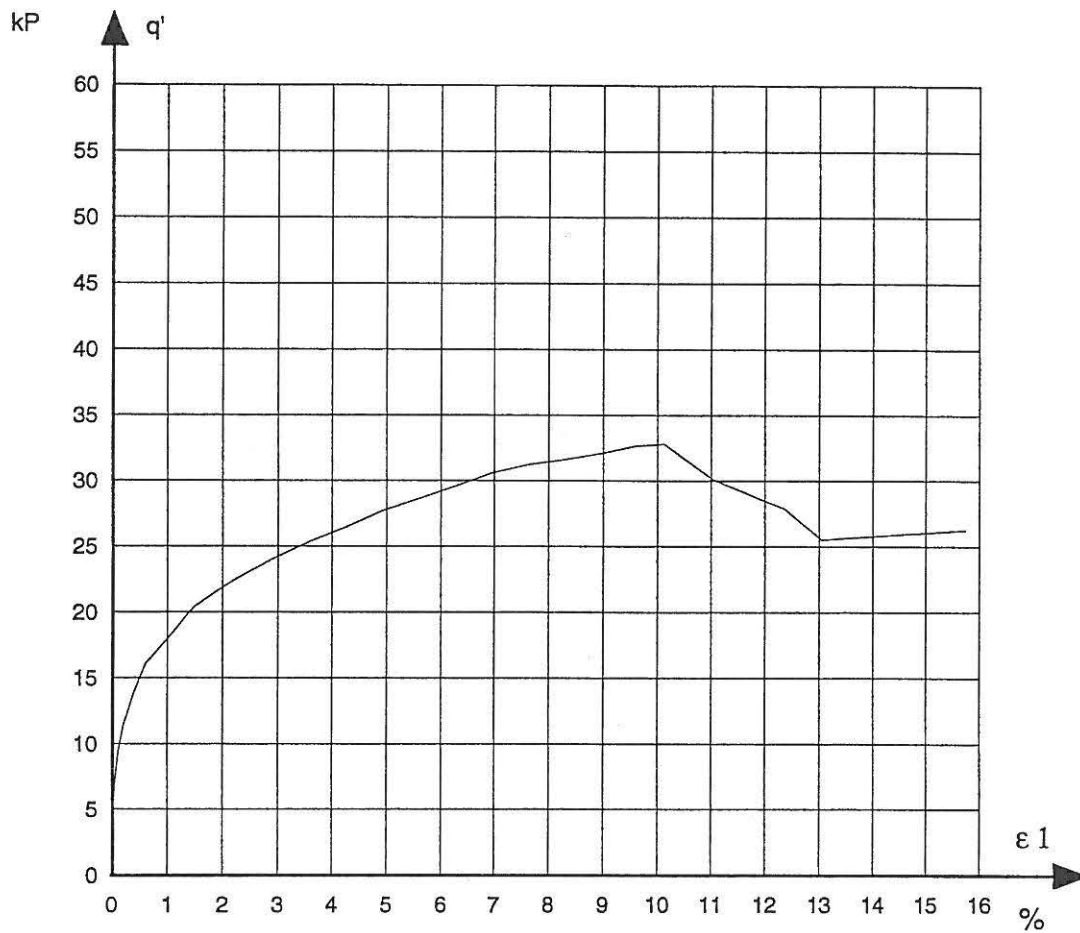


q'	p'	ϵ_1	ϵ_v
2.24	5.75	0.00	0.00
4.57	6.52	0.02	0.00
6.90	7.30	0.06	0.00
9.23	8.08	0.12	0.00
11.54	8.85	0.21	-0.04
13.84	9.61	0.38	-0.11
16.10	10.37	0.61	-0.23
18.29	11.10	1.06	-0.47
20.43	11.81	1.49	-0.81
22.43	12.48	2.19	-1.41
24.05	13.02	2.91	-1.95
25.32	13.44	3.57	-2.51
26.45	13.82	4.24	-3.07
27.70	14.23	4.91	-3.65
28.62	14.54	5.59	-4.20
29.59	14.86	6.27	-4.73
30.60	15.20	6.96	-5.27
31.25	15.42	7.64	-5.77
31.62	15.54	8.32	-6.26
32.12	15.71	9.00	-6.71
32.65	15.88	9.58	-7.15
32.79	15.93	10.13	-7.58
27.88	14.29	12.36	-8.39
26.27	13.76	15.75	-9.17

Job:	Encl. No
Blokhus sand	33
Exc:	Check:
HSP	LB

Remark:

No isotropic consolidation.
The specimen slipped out before failure.

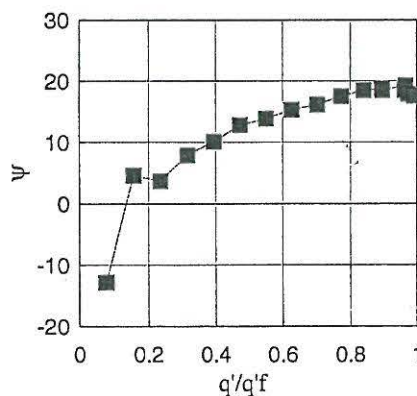
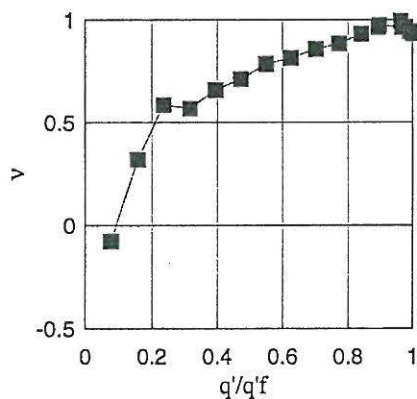


Job:	Encl. No
Blokhus sand	34
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.5	
Calibration file	Date	Void ratio	0.573	0.668
Loadtransducer 181	08.08.73	Saturation	1	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	5-5 kPa
		ϵ_1	0.00 %
		ϵ_v	0.00 %
	2. Drained compression.	Deformation rate: 10.1 % ph	

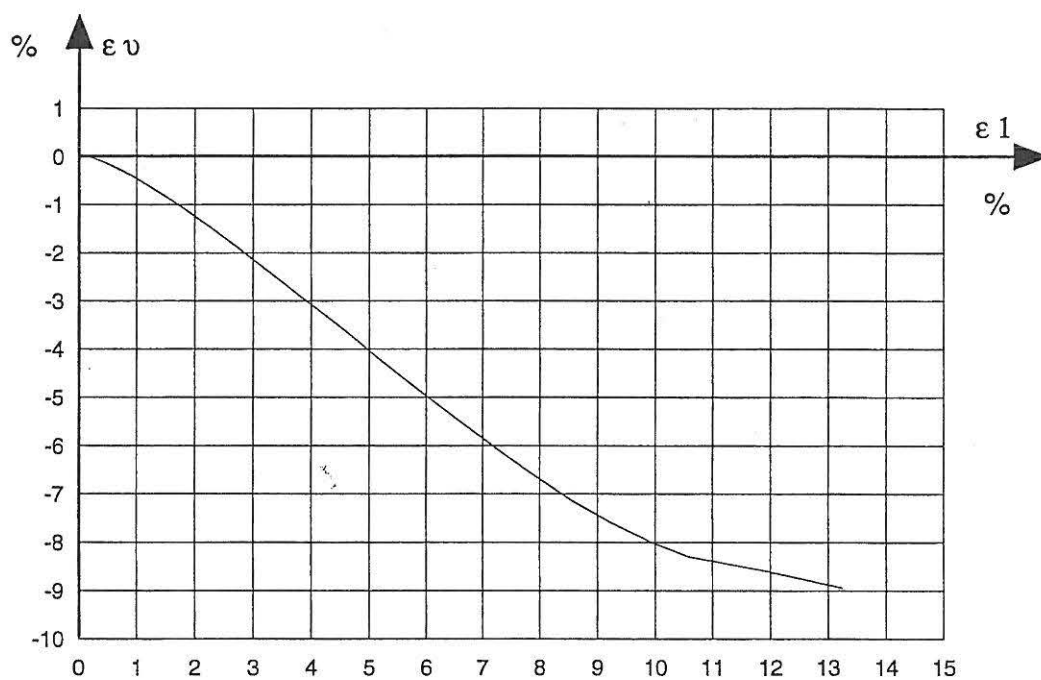
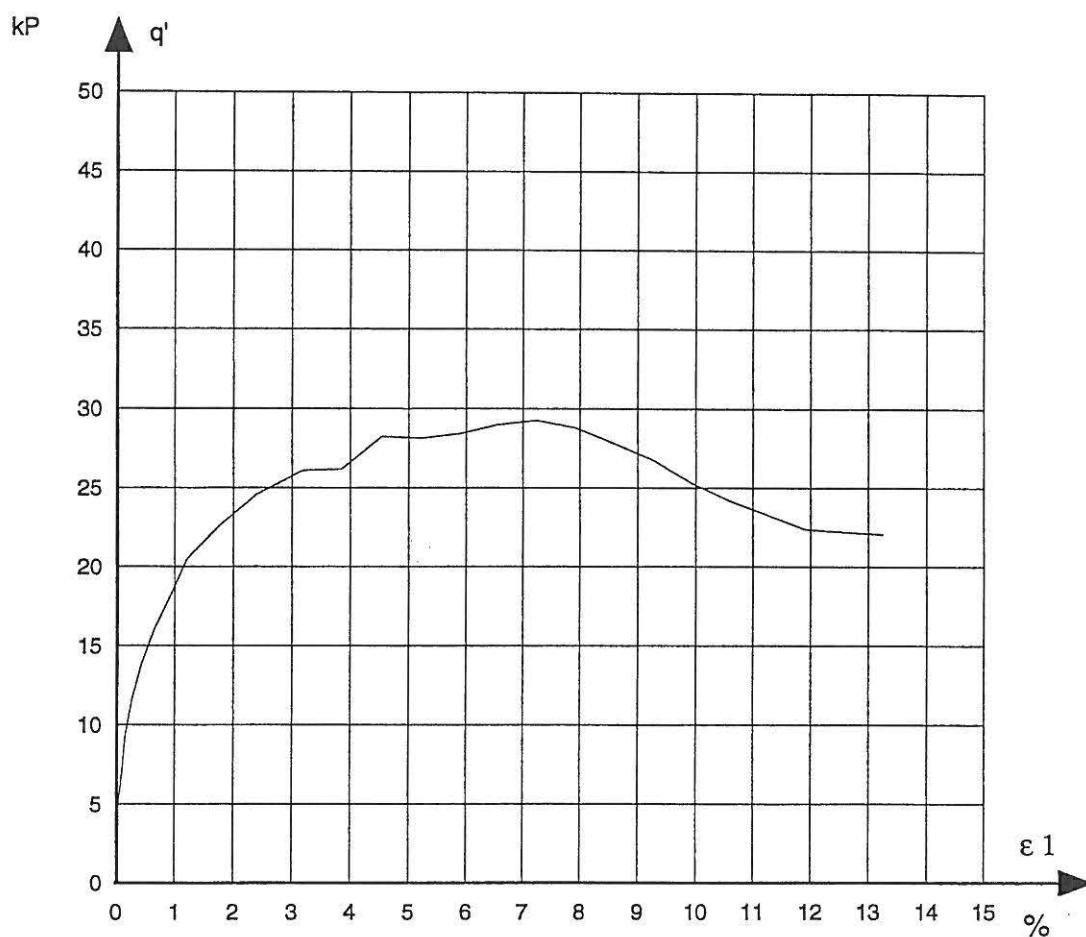
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	29.26	kPa	4.56	kPa
Mean normal stress	p'	14.75	kPa	6.52	kPa
Confining pressures	σ_3	5.00	kPa	5.00	kPa
Vertical strain	ϵ_1	7.25	%	0.04	%
Volumetric strain	ϵ_v	-6.06	%	0.02	%



q'	p'	ϵ_1	ϵ_v
2.23	5.74	0.00	0.00
2.23	5.74	0.01	0.01
4.56	6.52	0.04	0.02
6.89	7.30	0.10	0.01
9.22	8.07	0.15	0.00
11.53	8.85	0.26	-0.04
13.82	9.63	0.43	-0.11
16.09	10.38	0.66	-0.23
18.31	11.13	0.94	-0.42
20.52	11.89	1.22	-0.61
22.59	12.61	1.76	-1.03
24.57	13.24	2.39	-1.57
26.11	13.79	3.19	-2.31
26.19	13.80	3.85	-2.94
28.25	14.44	4.54	-3.57
28.15	14.38	5.21	-4.24
28.45	14.48	5.89	-4.87
29.00	14.67	6.56	-5.47
29.26	14.75	7.25	-6.06
28.83	14.61	7.92	-6.62
27.82	14.27	8.59	-7.16
26.79	13.93	9.26	-7.61
24.21	13.44	9.92	-7.99
22.05	13.07	10.59	-8.30

Job:	Encl. No
Blokhus sand	35
Exc:	Check:
HSP	LB

Remark:
No isotropic consolidation.

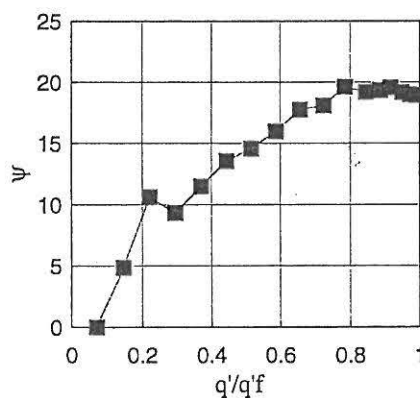
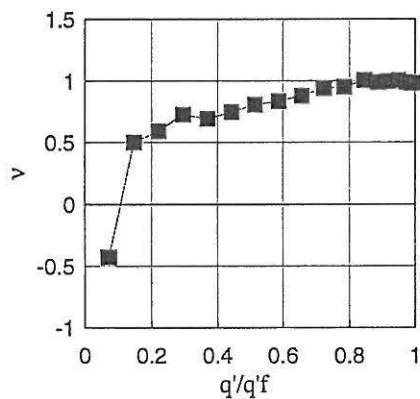


Job:	Encl. No
Blokhus sand	36
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	21.3	
Calibration file	Date	Void ratio	0.571	0.685
Loadtransducer	09.08.73	Saturation	0.99	
181		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	5-5 kPa
		ϵ_1	0.00 %
		ϵ_v	0.00 %
	2. Drained compression.		
	Deformation rate:		10.1 % ph

		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	31.27	kPa	4.57	kPa
Mean normal stress	p'	15.42	kPa	6.52	kPa
Confining pressures	σ_3	5.00	kPa	5.00	kPa
Vertical strain	ϵ_1	8.07	%	0.03	%
Volumetric strain	ϵ_v	-7.24	%	0.02	%

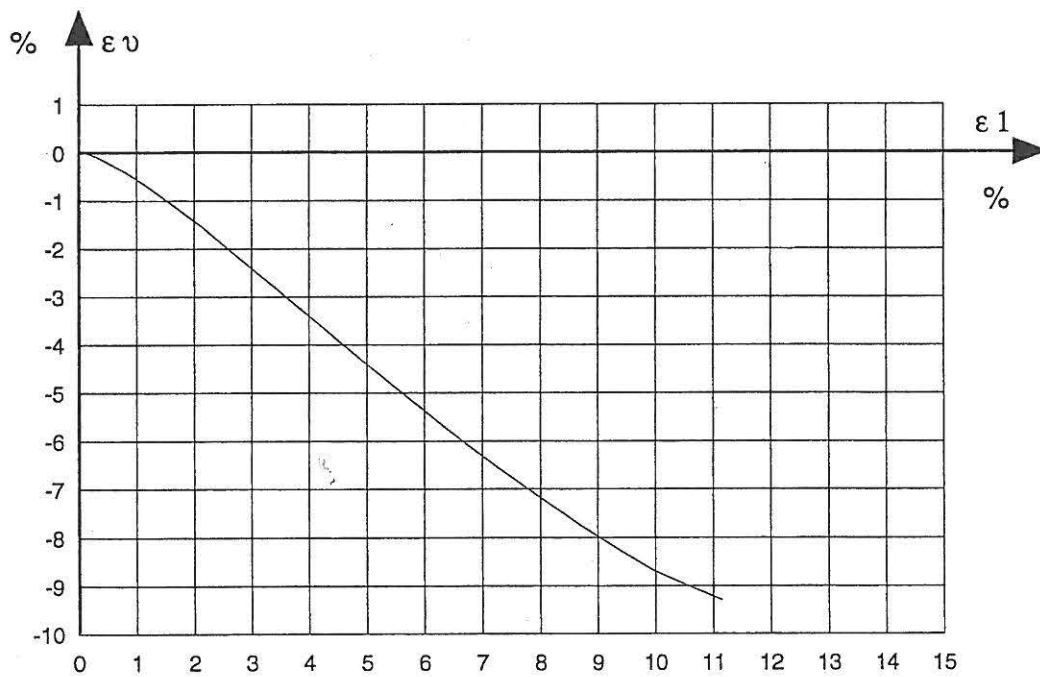
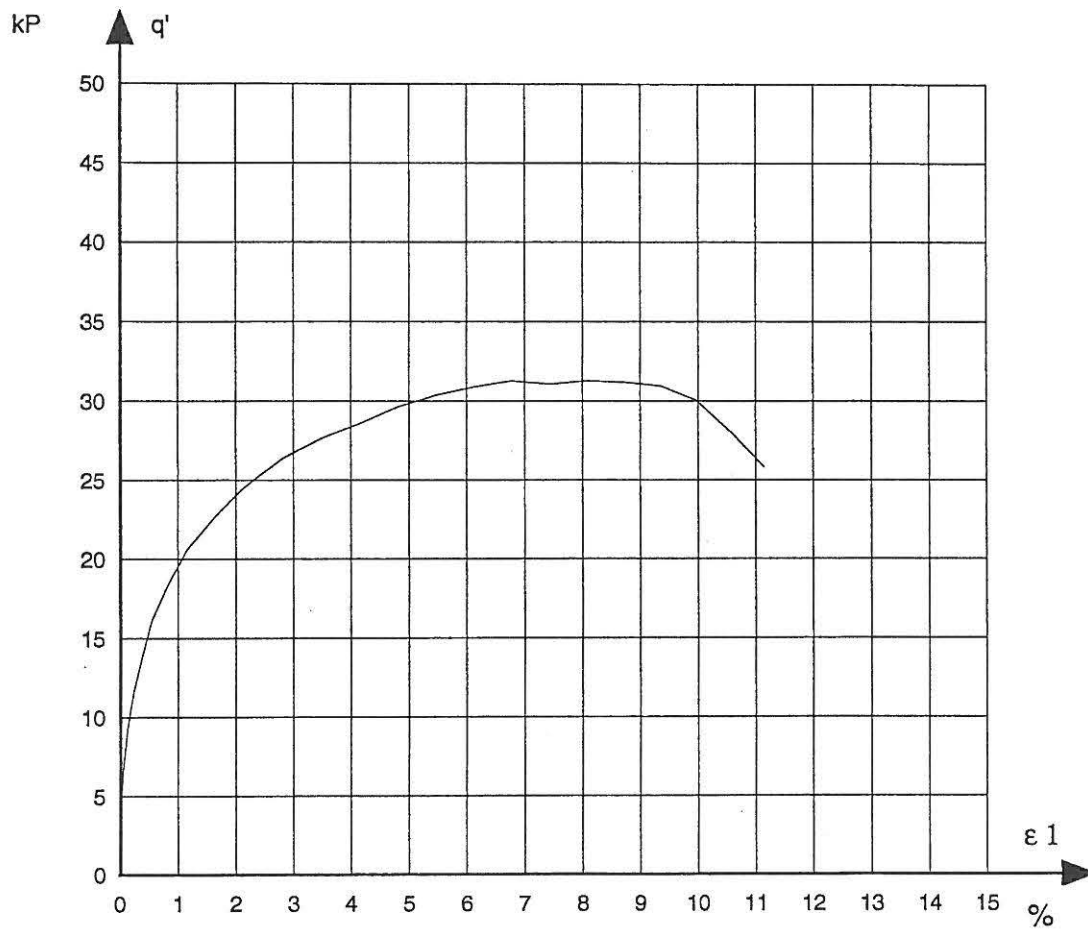


q'	p'	ϵ_1	ϵ_v
2.24	5.75	0.00	0.00
2.24	5.75	0.01	0.02
4.57	6.52	0.03	0.02
6.90	7.30	0.07	0.01
9.23	8.08	0.14	-0.02
11.54	8.85	0.23	-0.05
13.84	9.61	0.38	-0.13
16.11	10.37	0.55	-0.23
18.35	11.12	0.82	-0.42
20.53	11.84	1.15	-0.67
22.62	12.54	1.63	-1.08
24.50	13.17	2.13	-1.53
26.37	13.79	2.80	-2.22
27.63	14.21	3.47	-2.87
28.58	14.53	4.14	-3.54
29.64	14.88	4.80	-4.20
30.36	15.12	5.47	-4.85
30.89	15.30	6.12	-5.49
31.27	15.42	6.78	-6.12
31.07	15.36	7.43	-6.69
31.27	15.42	8.07	-7.24
31.20	15.40	8.72	-7.76
30.06	15.32	9.35	-8.25
25.78	15.02	9.97	-8.68

Job:	Encl. No
Blokhus sand	37
Exc:	Check:
HSP	LB

Remark:

No isotropic consolidation.

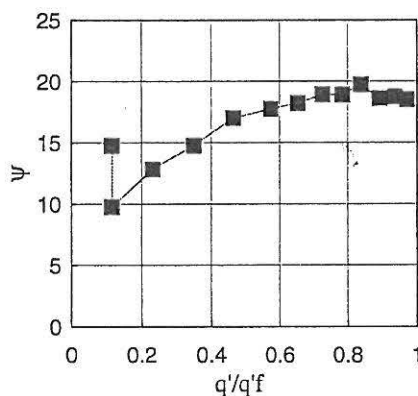
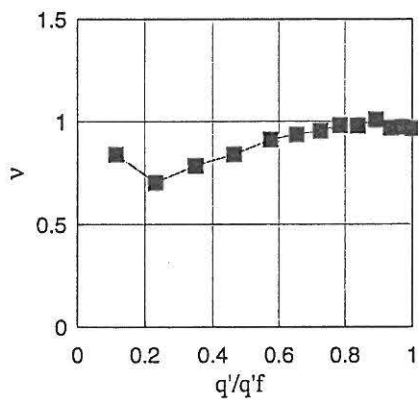


Job:	Blokhus sand	Encl. No	38
Exc:	HSP	Check:	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
Calibration file		Grain density	2.65	
Date		Void ratio	0.579	0.676
Loadtransducer 181		Saturation		
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	5-2.5 kPa
		ϵ_1	0.00 %
		ϵ_v	0.05 %
	2.Drained compression.		
	Deformation rate:		9.9 % ph

		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	19.70	kPa	2.24	kPa
Mean normal stress	p'	9.07	kPa	3.25	kPa
Confining pressures	σ_3	2.50	kPa	2.50	kPa
Vertical strain	ϵ_1	6.88	%	0.00	%
Volumetric strain	ϵ_v	-6.15	%	0.00	%

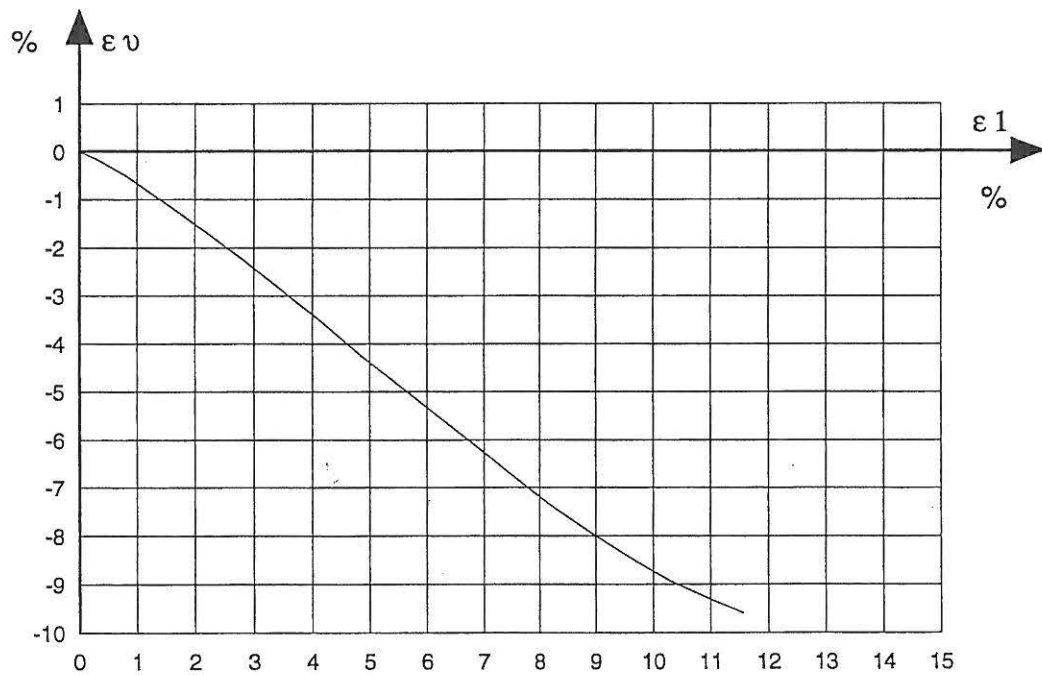
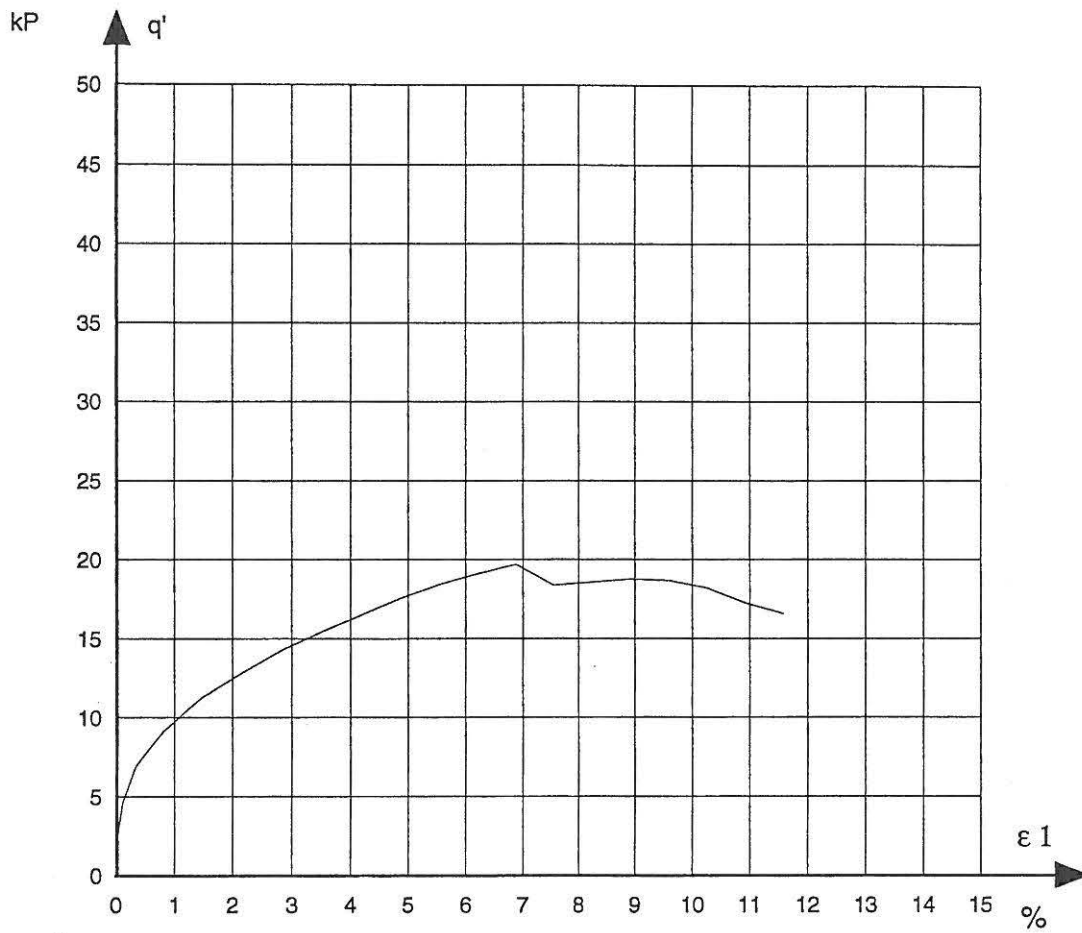


q'	p'	ϵ_1	ϵ_v
2.24	3.25	0.00	0.00
2.24	3.25	0.03	-0.02
4.57	4.02	0.11	-0.05
6.87	4.79	0.34	-0.18
9.12	5.54	0.81	-0.51
11.28	6.26	1.49	-1.06
12.85	6.78	2.17	-1.66
14.29	7.26	2.84	-2.27
15.44	7.65	3.52	-2.92
16.49	8.00	4.19	-3.57
17.54	8.35	4.86	-4.26
18.41	8.64	5.54	-4.89
19.10	8.87	6.20	-5.52
19.70	9.07	6.88	-6.15
18.38	8.63	7.55	-6.78
18.58	8.69	8.24	-7.40
18.76	8.75	8.92	-7.94
18.68	8.73	9.58	-8.44
18.19	8.56	10.26	-8.91
17.23	8.24	10.93	-9.28
16.56	8.02	11.59	-9.60

Job:	Encl. No
Blokhus sand	39
Exc:	Check:
HSP	LB

Remark:

No measurement during saturation.

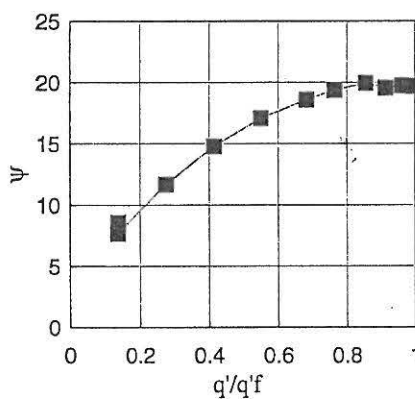
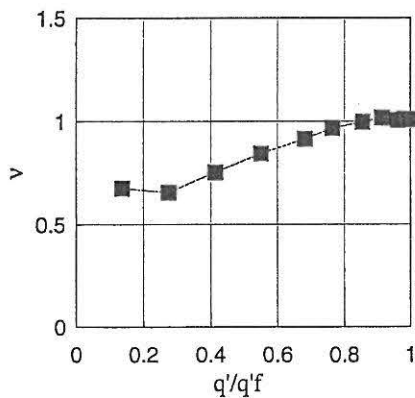


Job:	Encl. No
Blokhus sand	40
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	21.5	At failure 0.664
		Grain density	2.65	
Calibration file	Date	Void ratio	0.586	
Loadtransducer 181	10.08.73	Saturation	0.97	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	5-2.5 kPa
		ϵ_1	0.00 %
		ϵ_v	0.04 %
	2. Drained compression.	Deformation rate: 9.9 % ph	

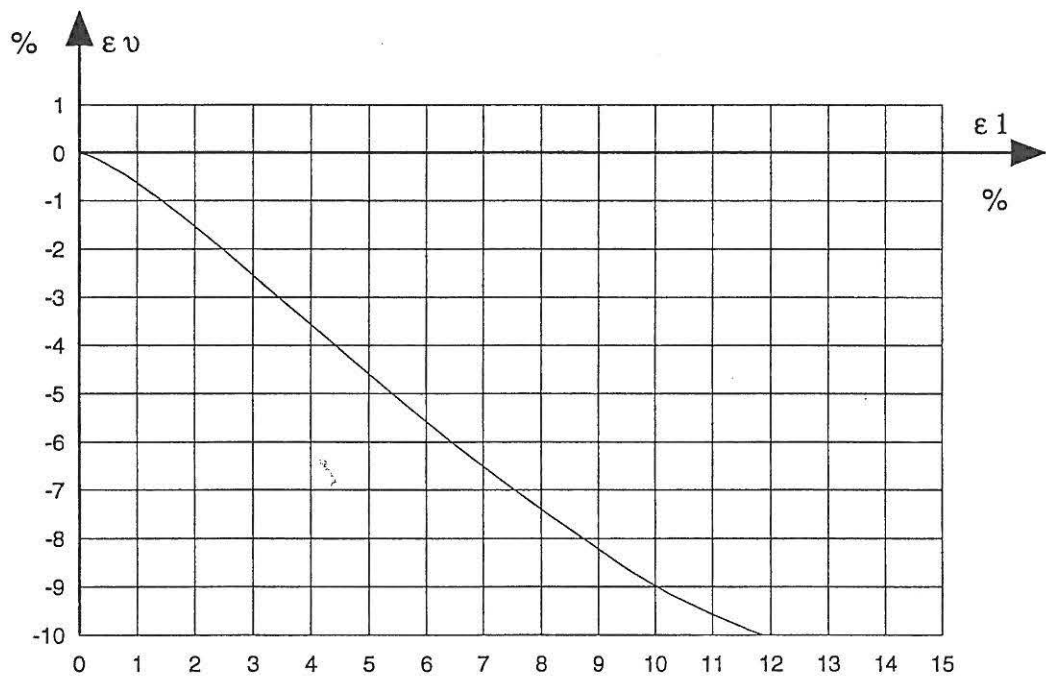
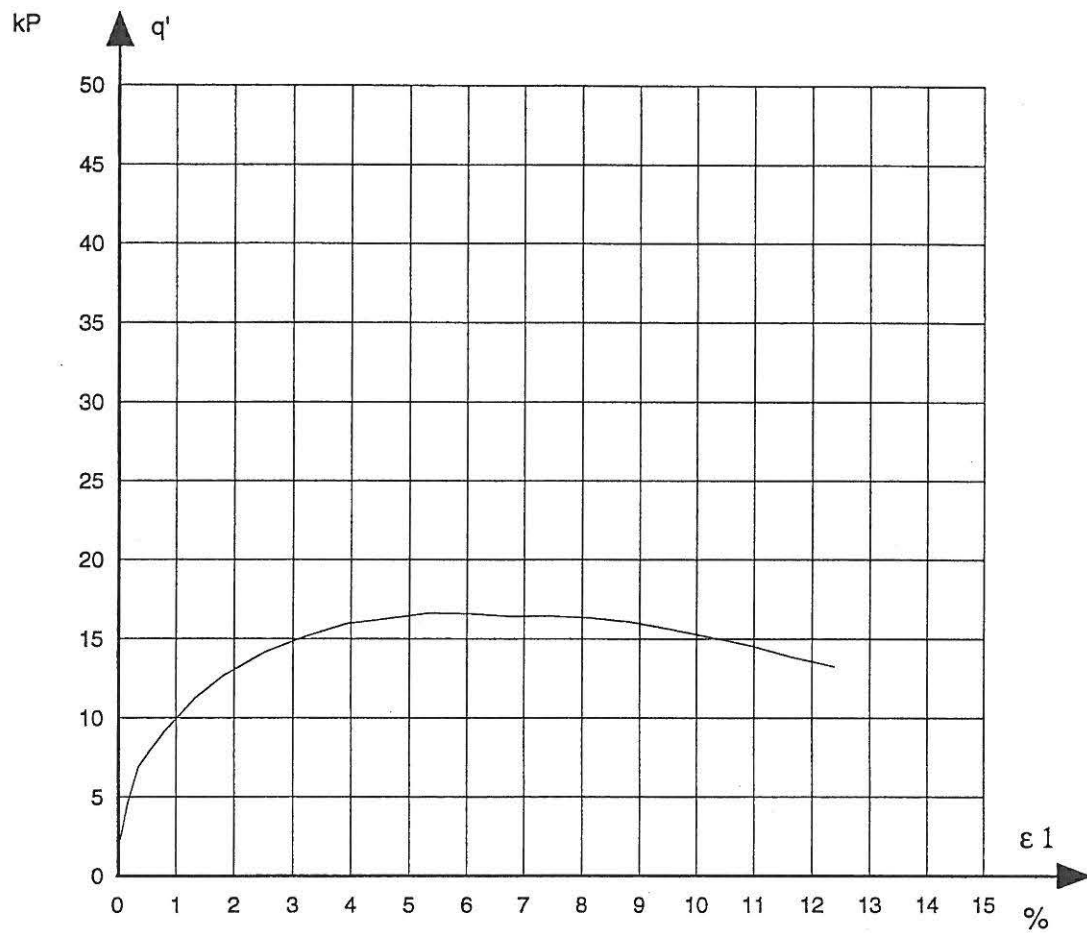
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	16.61	kPa	2.24	kPa
Mean normal stress	p'	8.04	kPa	3.25	kPa
Confining pressures	σ_3	2.50	kPa	2.50	kPa
Vertical strain	ϵ_1	5.34	%	0.00	%
Volumetric strain	ϵ_v	-4.93	%	0.00	%



q'	p'	ϵ_1	ϵ_v
2.24	3.25	0.00	0.00
2.24	3.25	0.05	-0.02
4.56	4.02	0.17	-0.05
6.87	4.79	0.35	-0.14
9.13	5.54	0.79	-0.45
11.31	6.27	1.36	-0.92
12.67	6.72	1.82	-1.35
14.17	7.22	2.53	-2.06
15.15	7.55	3.22	-2.78
15.94	7.81	3.92	-3.48
16.27	7.92	4.63	-4.20
16.61	8.04	5.34	-4.93
16.59	8.03	6.04	-5.61
16.42	7.97	6.75	-6.28
16.44	7.98	7.44	-6.91
16.30	7.93	8.15	-7.52
16.05	7.85	8.86	-8.10
15.61	7.70	9.55	-8.66
15.08	7.53	10.25	-9.15
14.56	7.35	10.95	-9.55
13.84	7.11	11.65	-9.91
13.25	6.92	12.39	-10.23

Job: Blokhus sand	Encl. No 41
Exc: HSP	Check: LB

Remark:

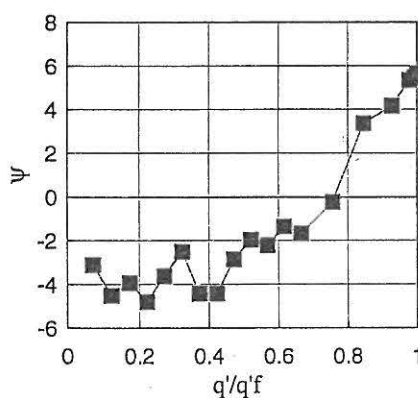
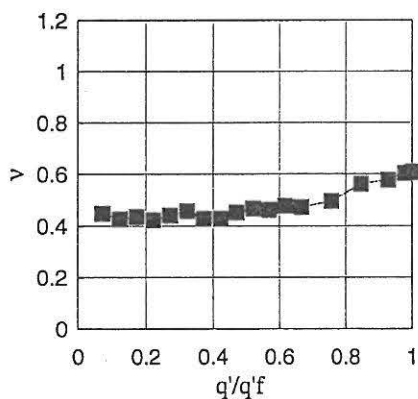


Job:	Encl. No
Blokhus sand	42
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	25	
Calibration file	Date	Void ratio	2.65	
		Saturation	0.710	0.726
Loadtransducer	16.08.73	Dimension H mm	0.93	
15001		D mm	72	
			70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	50-800 kPa
		ϵ_1	0.22 %
		ϵ_v	0.81 %
2. Drained compression.			
Deformation rate:		10 % ph	

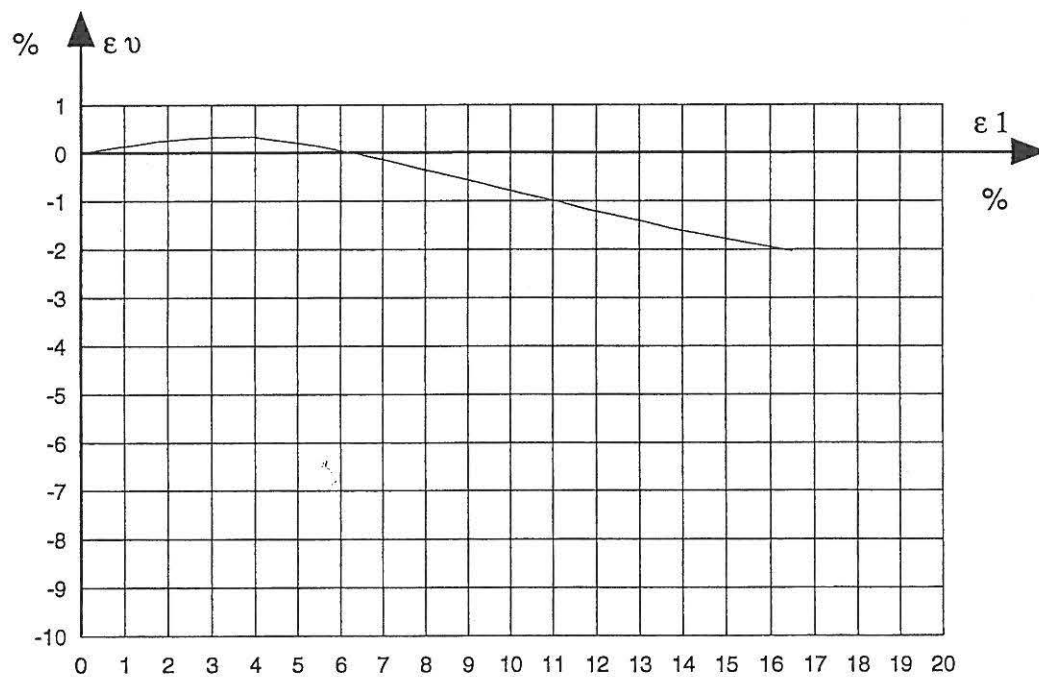
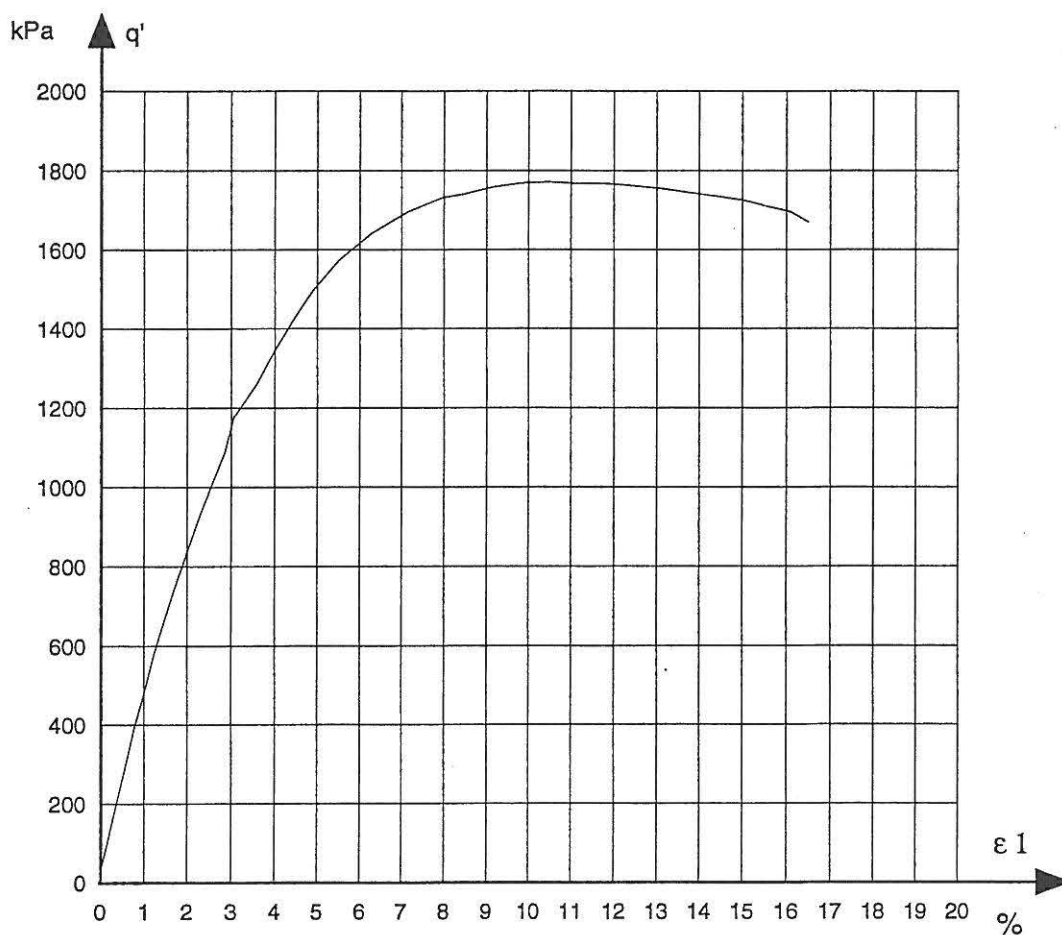
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1770.35	kPa	1339.09	kPa
Mean normal stress	p'	1390.12	kPa	1246.36	kPa
Confining pressures	σ_3	800.00	kPa	800.00	kPa
Vertical strain	ϵ_1	10.47	%	3.97	%
Volumetric strain	ϵ_v	-0.88	%	0.33	%



q'	p'	ϵ_1	ϵ_v
35.40	811.80	0.00	0.00
35.39	811.80	0.04	0.02
125.47	841.82	0.21	0.04
215.24	871.75	0.41	0.06
304.69	901.56	0.61	0.09
393.89	931.30	0.80	0.12
482.66	960.89	1.01	0.14
571.04	990.35	1.22	0.16
658.93	1019.64	1.48	0.20
746.42	1048.81	1.73	0.23
833.31	1077.77	1.99	0.26
919.65	1106.55	2.27	0.28
1005.38	1135.13	2.56	0.30
1090.29	1163.43	2.87	0.31
1176.08	1192.03	3.06	0.32
1339.09	1246.36	3.97	0.33
1496.74	1298.91	4.90	0.22
1641.00	1347.00	6.28	0.00
1730.49	1376.83	7.95	-0.34
1757.57	1385.86	9.15	-0.60
1770.35	1390.12	10.47	-0.88
1760.53	1386.84	12.44	-1.30
1734.12	1378.04	14.40	-1.68
1668.38	1356.13	16.52	-2.04

Job:	Encl. No
Blokhus sand	43
Exc:	Check:
HSP	LB

Remark:

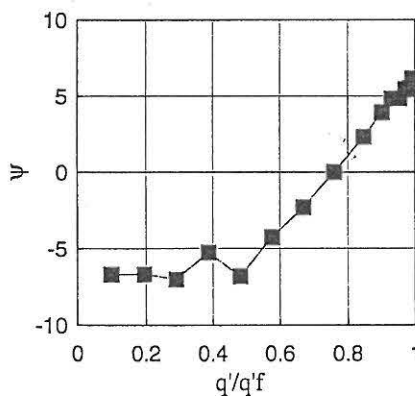
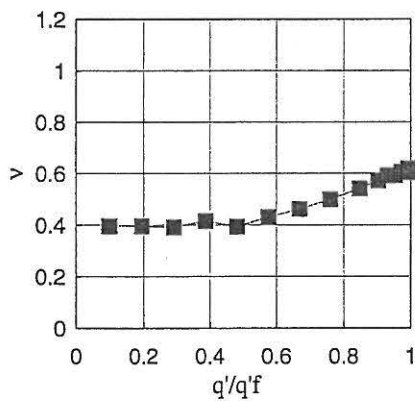


Job:	Encl. No
Blokhus sand	44
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	25.5	At failure 0.718
		Grain density	2.65	
Calibration file	Date	Void ratio	0.701	
Loadtransducer 15001	17.08.73	Saturation	0.97	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	50-800 kPa
		ϵ_1	0.25 %
		ϵ_v	0.97 %
	2. Drained compression.	Deformation rate: 10 % ph	

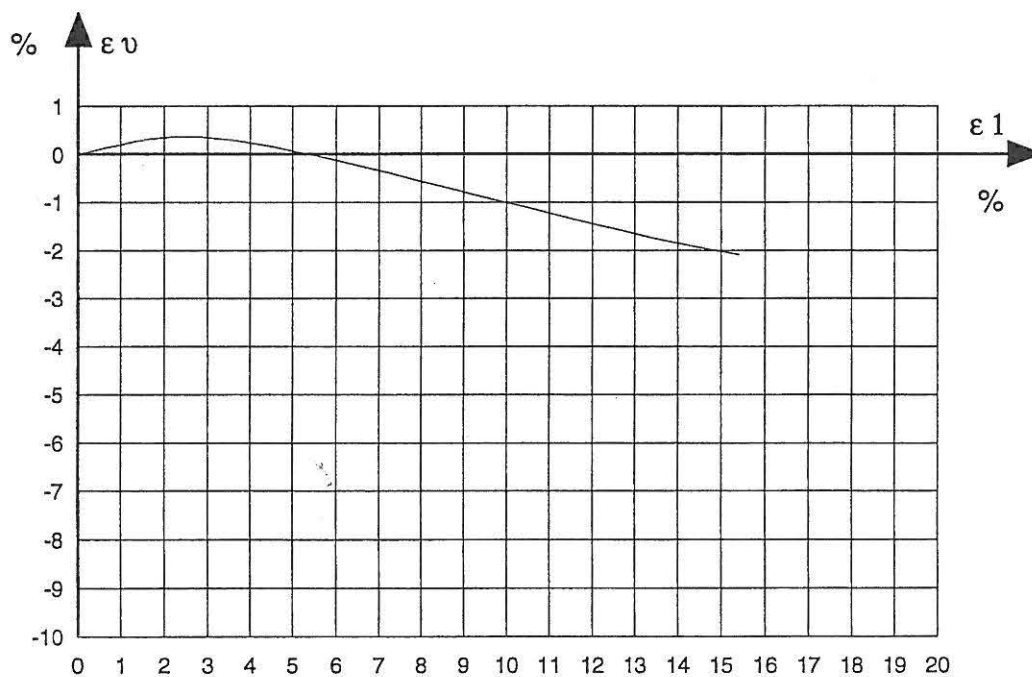
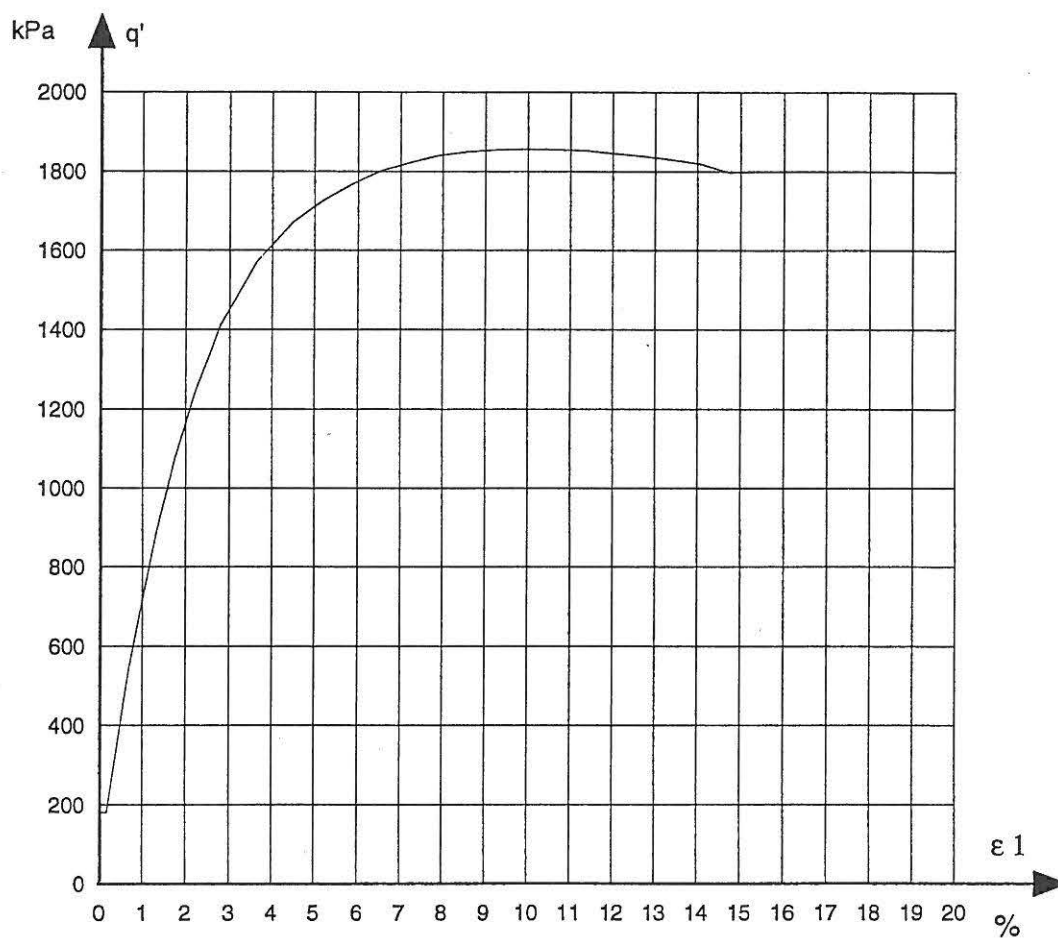
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1855.69	kPa	1409.67	kPa
Mean normal stress	p'	1418.56	kPa	1269.89	kPa
Confining pressures	σ_3	800.00	kPa	800.00	kPa
Vertical strain	ϵ_1	9.96	%	2.78	%
Volumetric strain	ϵ_v	-0.99	%	0.36	%



q'	p'	ϵ_1	ϵ_v
180.30	860.10	0.00	0.00
180.34	860.11	0.17	0.04
359.95	919.98	0.43	0.09
538.86	979.62	0.68	0.14
716.54	1038.85	1.00	0.20
893.24	1097.75	1.34	0.27
1068.21	1156.07	1.74	0.32
1240.75	1213.58	2.20	0.36
1409.67	1269.89	2.78	0.36
1570.83	1323.61	3.63	0.29
1671.64	1357.21	4.49	0.16
1725.02	1375.01	5.18	0.04
1767.78	1389.26	5.86	-0.09
1801.50	1400.50	6.55	-0.23
1822.54	1407.51	7.23	-0.38
1839.37	1413.12	7.91	-0.54
1849.24	1416.41	8.60	-0.69
1854.23	1418.08	9.28	-0.85
1855.69	1418.56	9.96	-0.99
1854.06	1418.02	10.64	-1.14
1852.42	1417.47	11.33	-1.30
1838.19	1412.73	12.70	-1.59
1818.54	1406.18	14.05	-1.86
1798.57	1399.52	15.42	-2.09

Job:	Encl. No
Blokhus sand	45
Exc:	Check:
HSP	LB

Remark:

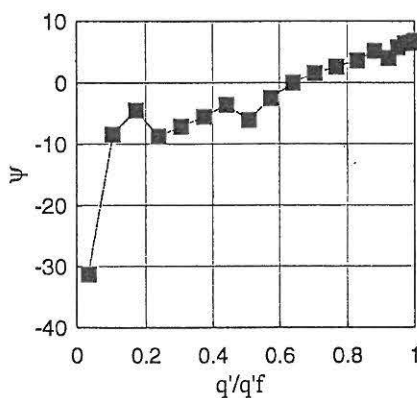
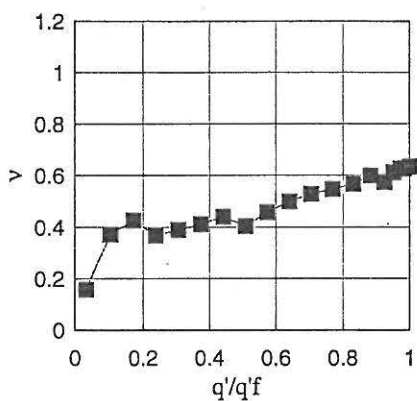


Job:	Encl. No
Blokhus sand	46
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	24.9	At failure 0.704
		Grain density	2.65	
Calibration file	Date	Void ratio	0.683	
Loadtransducer 5001	21.08.73	Saturation	0.97	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	50-400 kPa
		ϵ_1	0.17 %
		ϵ_v	0.54 %
2. Drained compression.			
Deformation rate:		9.7 % ph	

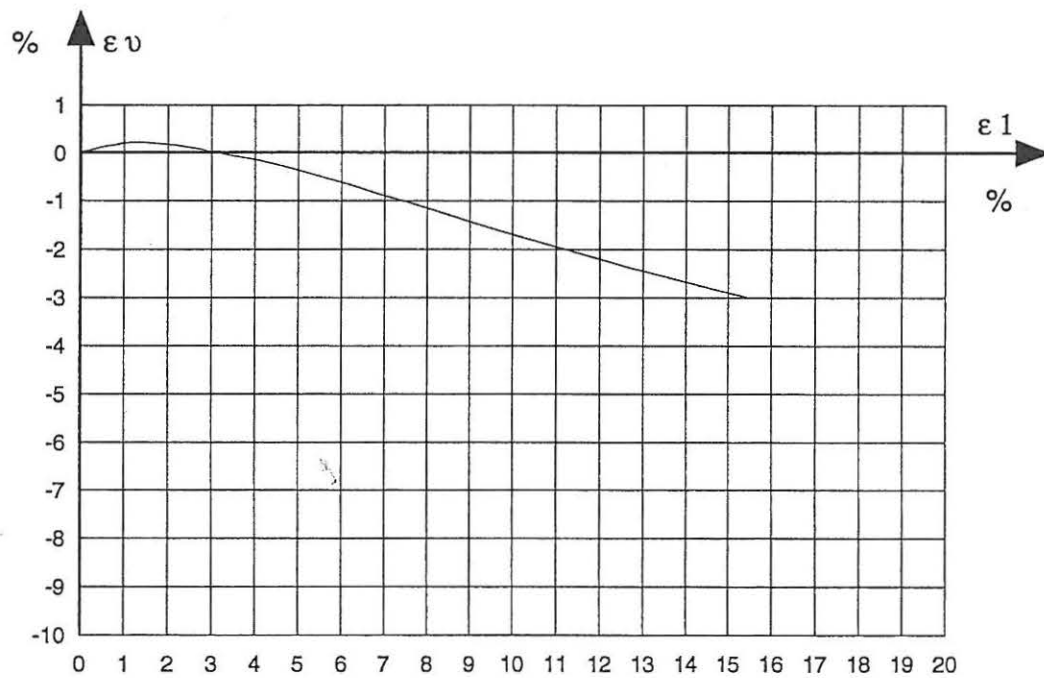
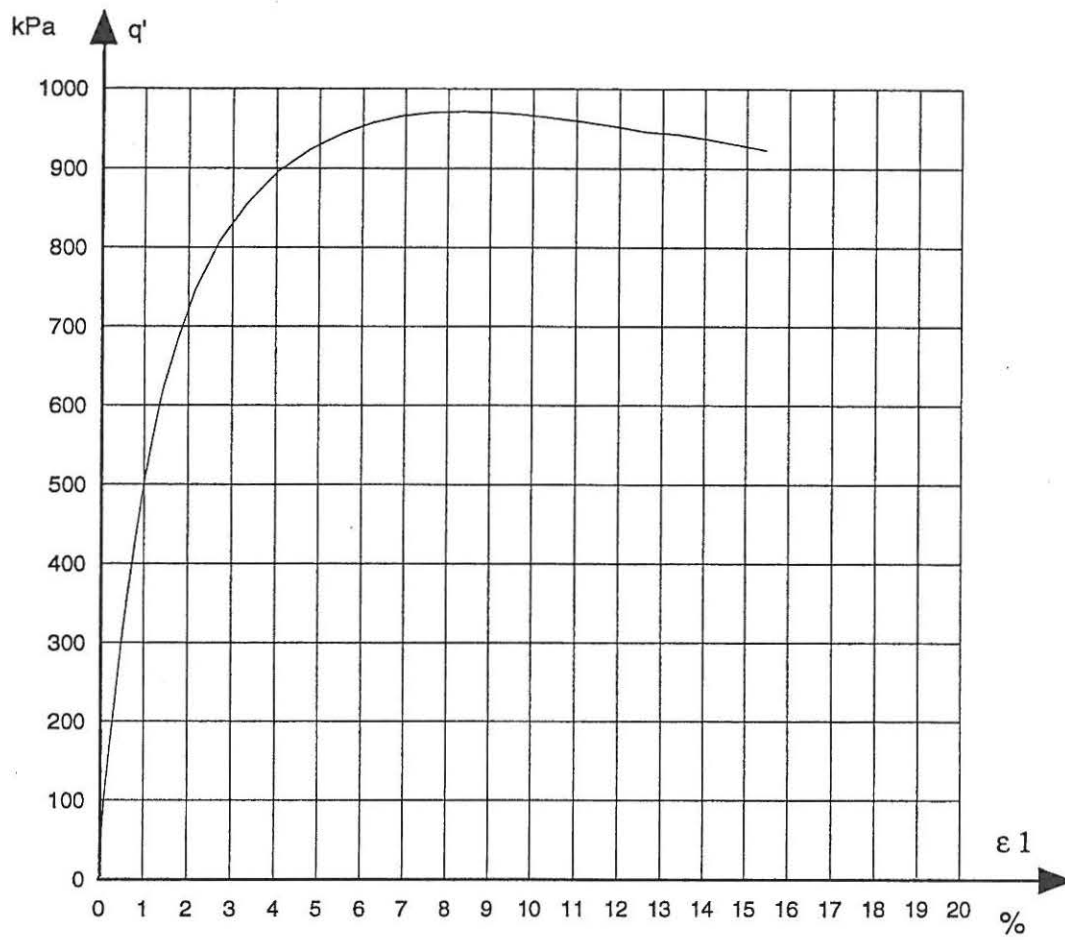
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	971.39	kPa	620.77	kPa
Mean normal stress	p'	723.80	kPa	606.92	kPa
Confining pressures	σ_3	400.00	kPa	400.00	kPa
Vertical strain	ϵ_1	8.35	%	1.44	%
Volumetric strain	ϵ_v	-1.25	%	0.22	%



q'	p'	ϵ_1	ϵ_v
33.10	411.03	0.00	0.00
33.08	411.03	0.03	0.02
99.17	433.06	0.10	0.04
165.11	455.04	0.22	0.05
230.92	476.97	0.36	0.09
296.60	498.87	0.49	0.12
362.08	520.69	0.63	0.14
427.33	542.44	0.79	0.16
492.32	564.11	0.97	0.20
556.86	585.62	1.19	0.22
620.77	606.92	1.44	0.22
683.75	627.92	1.76	0.20
745.70	648.57	2.14	0.16
805.46	668.49	2.69	0.09
855.19	685.06	3.32	-0.04
896.29	698.76	4.05	-0.14
923.21	707.74	4.78	-0.31
942.82	714.27	5.50	-0.49
956.62	718.87	6.21	-0.67
971.39	723.80	8.35	-1.25
967.37	722.46	9.78	-1.64
952.25	717.42	11.93	-2.18
936.66	712.22	14.04	-2.69
922.85	707.62	15.47	-3.00

Job: Blokhus sand	Encl. No 47
Exc: HSP	Check: LB

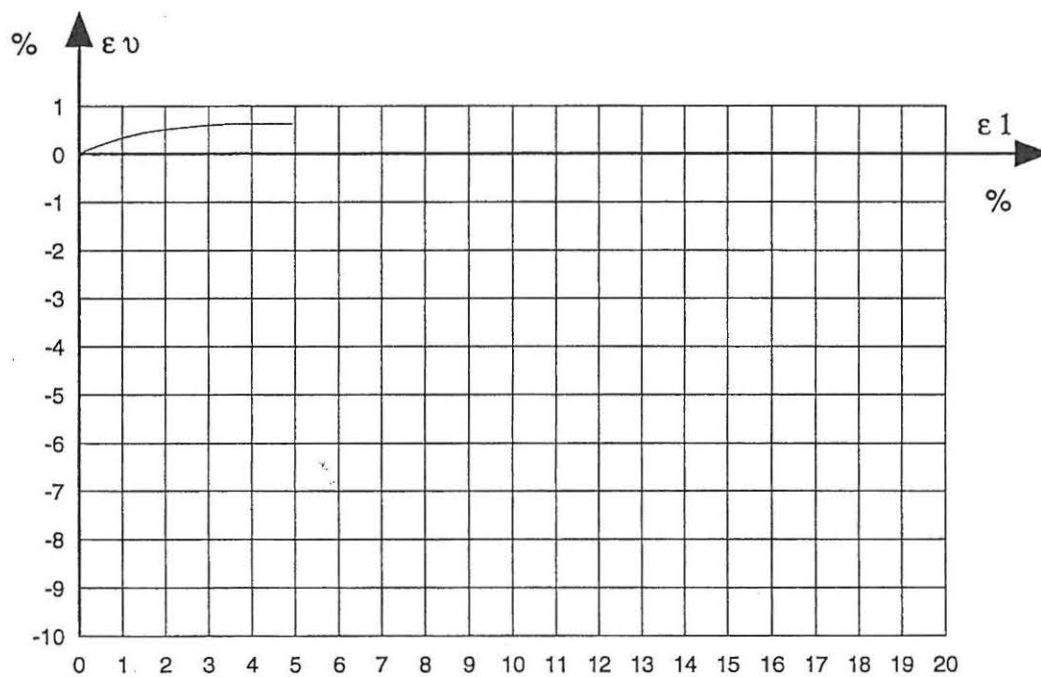
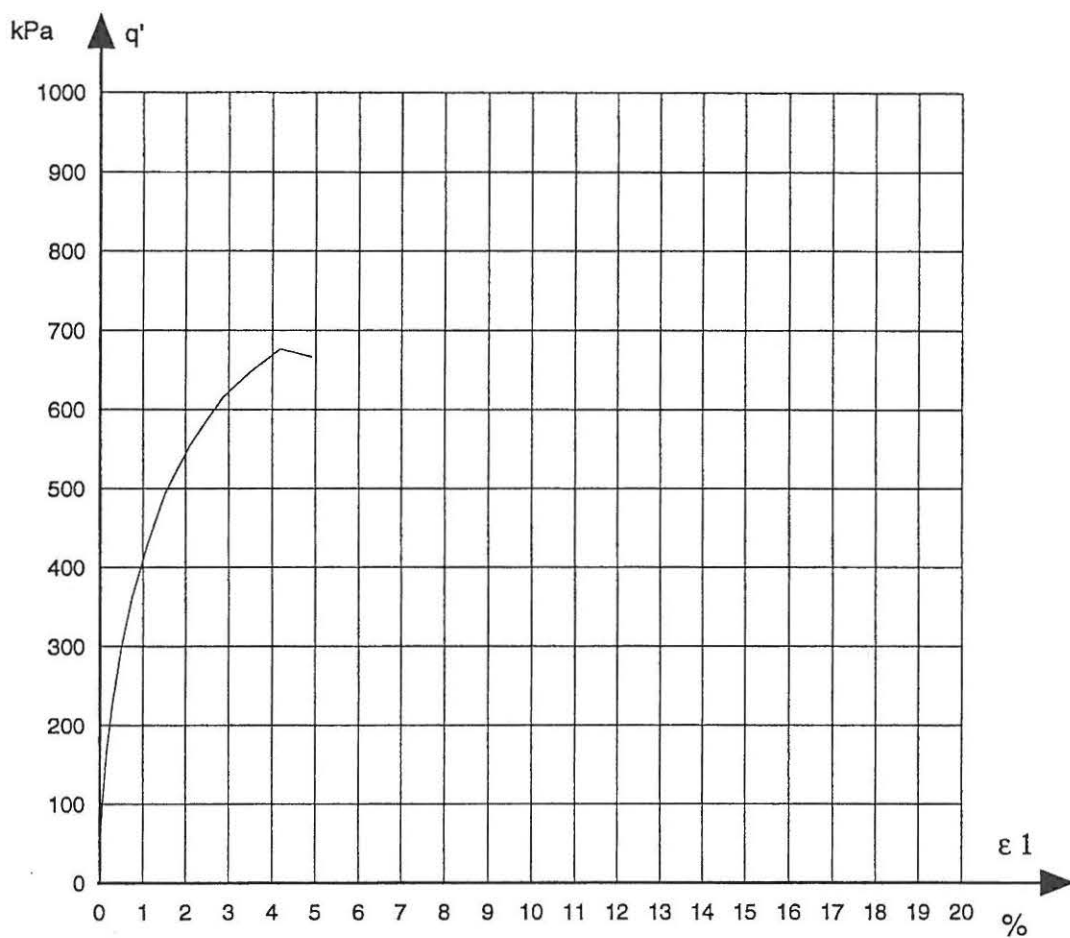
Remark:



Job:	Encl. No
Blokhus sand	48
Exc:	Check:
HSP	LB

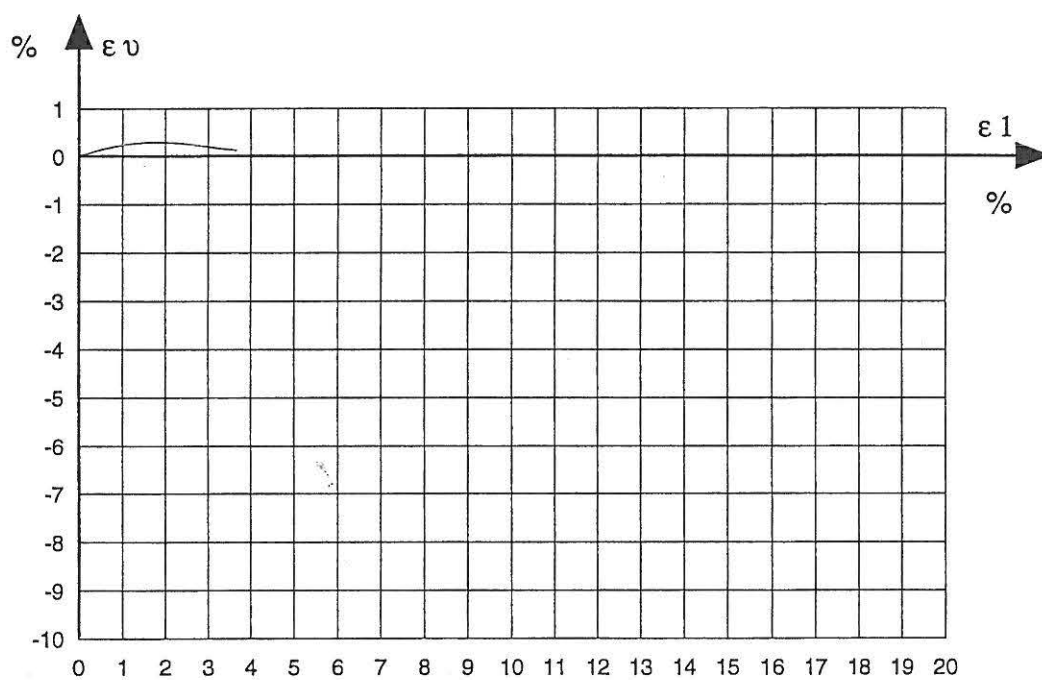
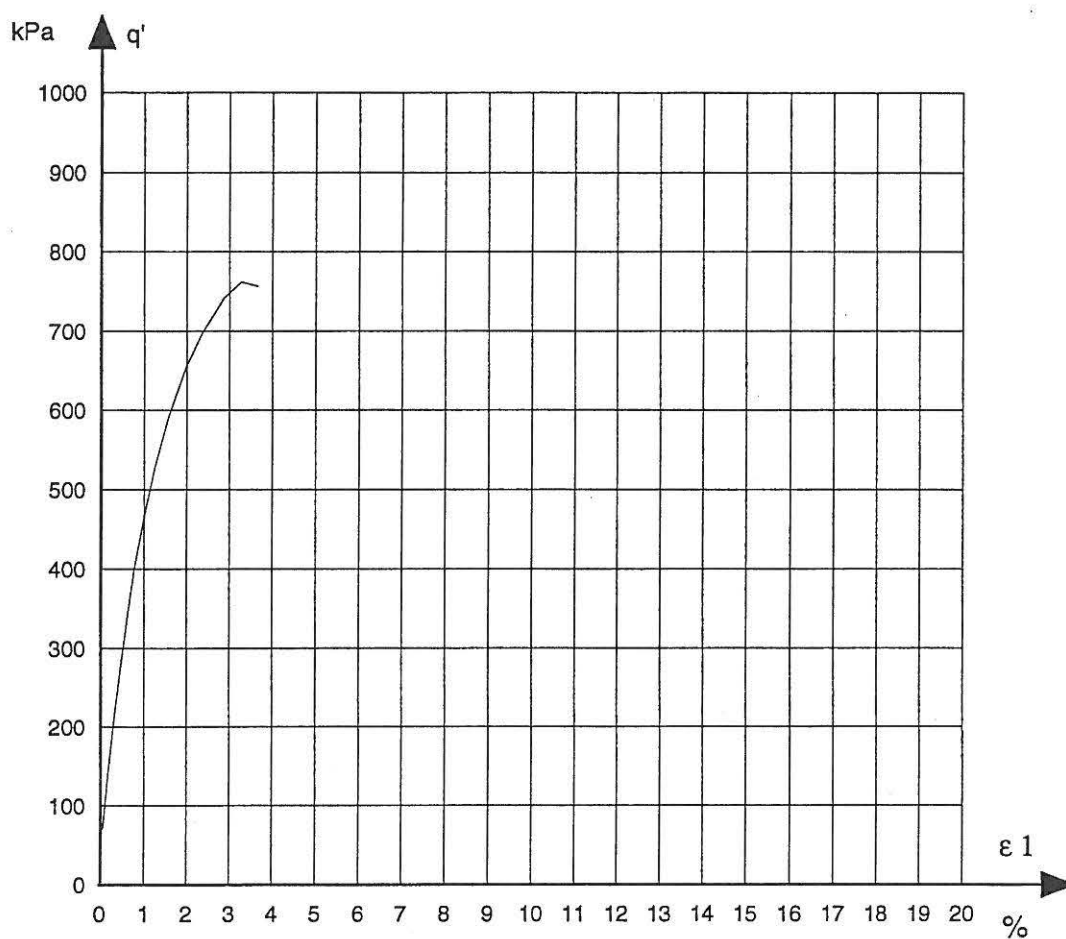
Remark:

The specimen slipped out before failure.



Job:	Encl. No
Blokhuis sand	50
Exc:	Check:
HSP	LB

The specimen slipped out before failure.

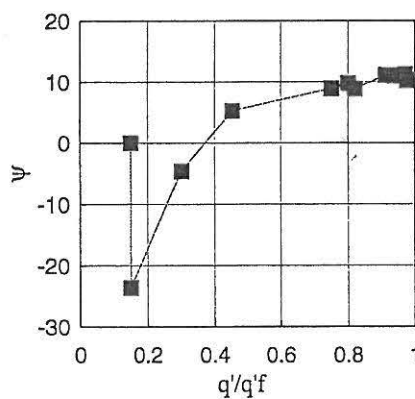
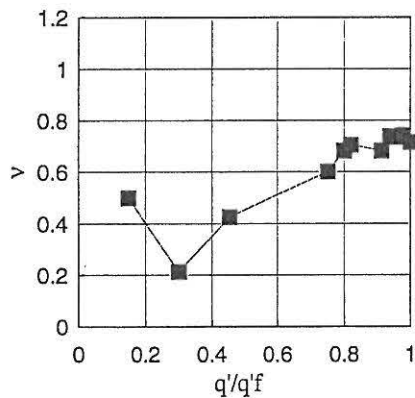


Job:	Encl. No
Blokhuis sand	52
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	23	
Calibration file	Date	Void ratio	2.65	
		Saturation	0.723	0.765
Loadtransducer	18.02.74	Dimension H mm	0.84	
2003		D mm	72	
			70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	20-20 kPa
		ϵ_1	0.00 %
		ϵ_v	0.00 %
	2. Drained compression.		
Deformation rate:		9.6 % ph	

		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	67.48	kPa	30.54	kPa
Mean normal stress	p'	42.49	kPa	30.18	kPa
Confining pressures	σ_3	20.00	kPa	20.00	kPa
Vertical strain	ϵ_1	6.23	%	0.21	%
Volumetric strain	ϵ_v	-2.42	%	0.05	%

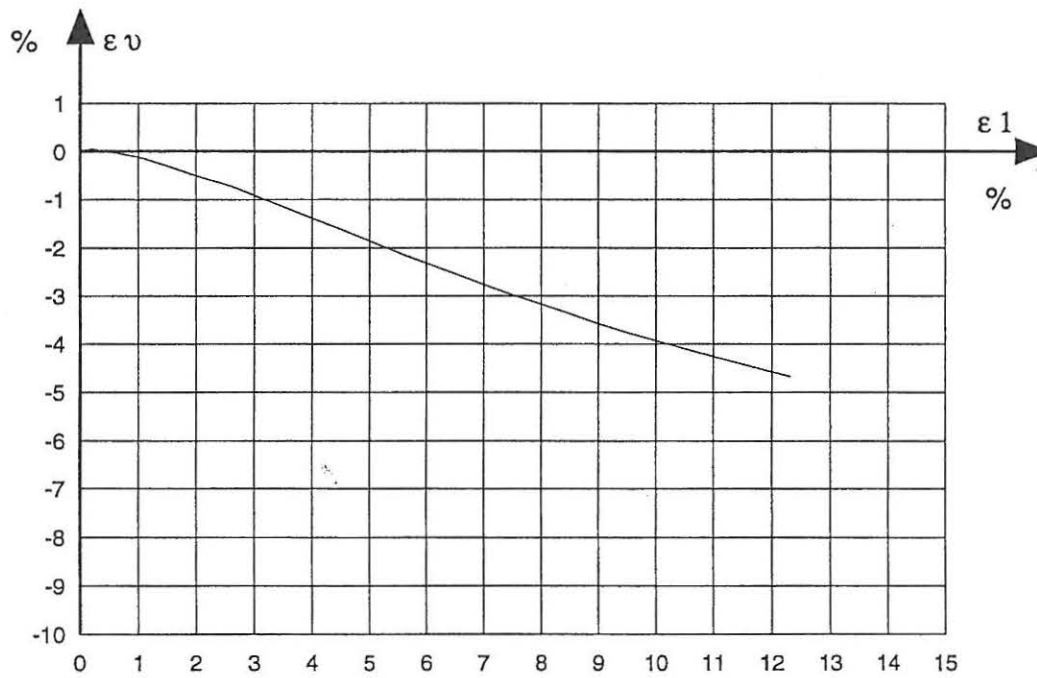
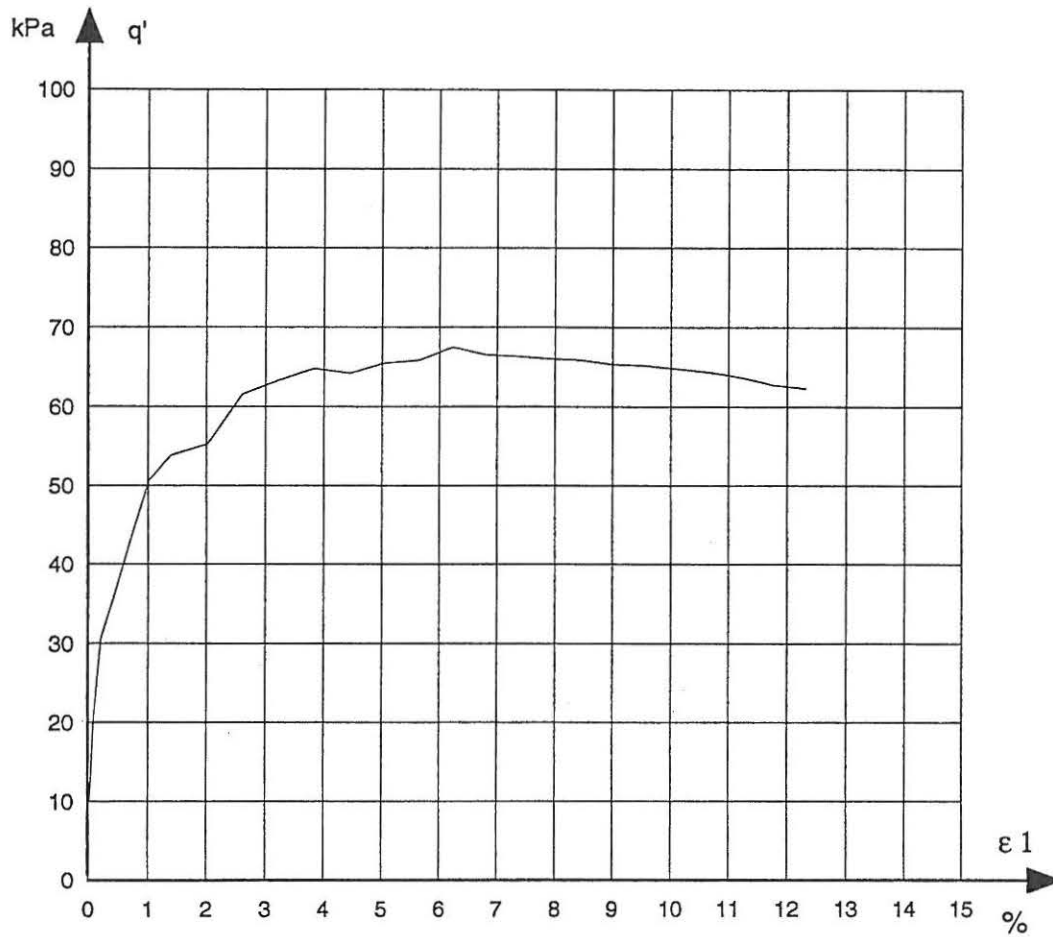


q'	p'	ϵ_1	ϵ_v
10.00	23.33	0.00	0.00
9.99	23.33	0.02	0.00
20.28	26.76	0.09	0.04
30.54	30.18	0.21	0.05
50.62	36.87	1.01	-0.11
53.89	37.96	1.40	-0.25
55.28	38.43	2.02	-0.51
61.55	40.52	2.61	-0.72
63.26	41.09	3.21	-1.01
64.79	41.60	3.83	-1.30
64.14	41.38	4.44	-1.59
65.47	41.82	5.05	-1.88
65.83	41.94	5.64	-2.17
67.48	42.49	6.23	-2.42
66.53	42.18	6.80	-2.67
66.29	42.10	7.35	-2.92
66.03	42.01	7.92	-3.14
65.86	41.95	8.46	-3.36
65.31	41.77	9.02	-3.59
65.20	41.73	9.57	-3.79
64.73	41.58	10.12	-3.97
64.31	41.44	10.67	-4.15
63.67	41.22	11.23	-4.33
62.30	40.77	12.32	-4.67

Job:	Encl. No
Blokhus sand	53
Exc:	Check:
HSP	LB

Remark:

No isotropic consolidation.

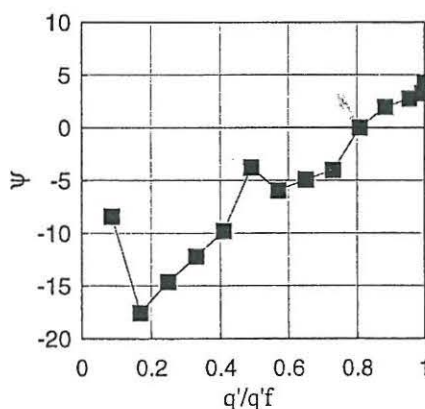
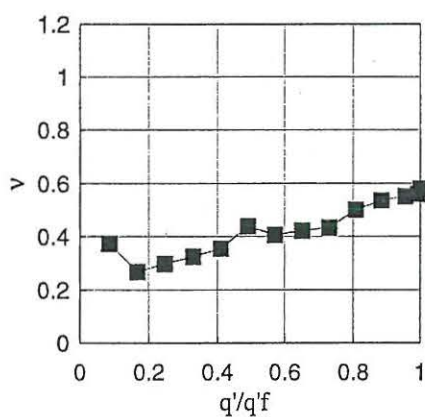


Job:	Encl. No
Blokhuis sand	54
Exc:	Check:
HSP	LB

Description of soil Blokhuis sand		Water content %	Before test 26.3	At failure 0.739
		Grain density	2.65	
Calibration file	Date	Void ratio	0.739	
		Saturation	0.94	
Loadtransducer 5001	19.02.74	Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	20-400 kPa
		ϵ_1	0.37 %
		ϵ_v	0.70 %
	2.Drained compression.		
	Deformation rate:		9.7 % ph

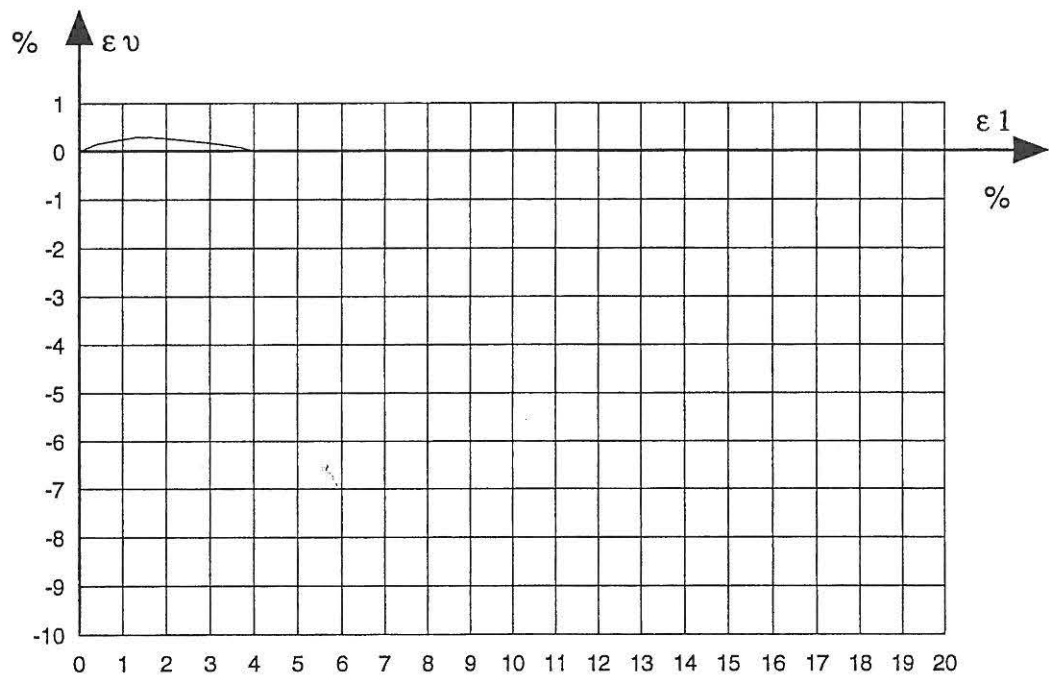
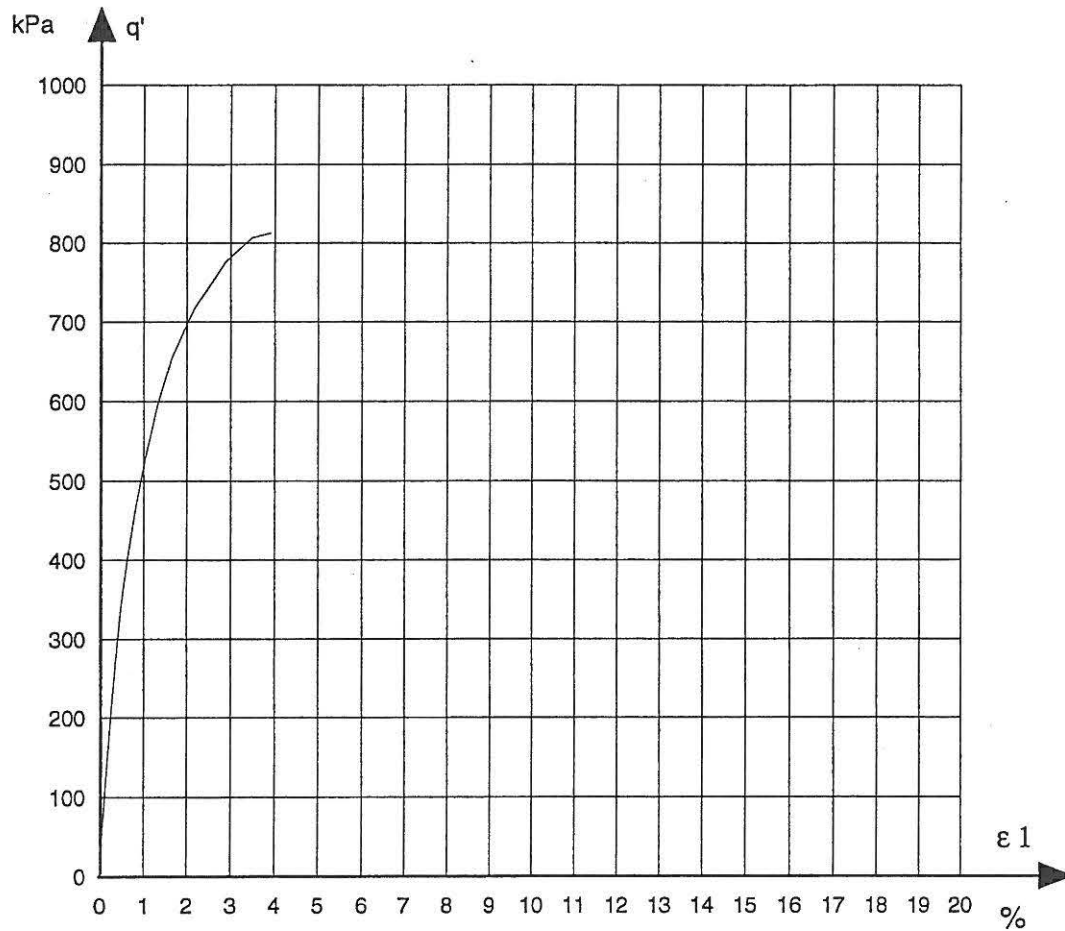
	Values at failure	Values for $\Delta\varepsilon_v = 0$
Deviator stress q'		655.76 kPa
Mean normal stress p'		618.59 kPa
Confining pressures σ_3		400.00 kPa
Vertical strain ε_1		1.66 %
Volumetric strain ε_v		0.29 %

[illegible]

Job: Blokhus sand	Encl. No 55
Exc:	Check:
HSP	LB

Remark:

The specimen slipped out before failure.

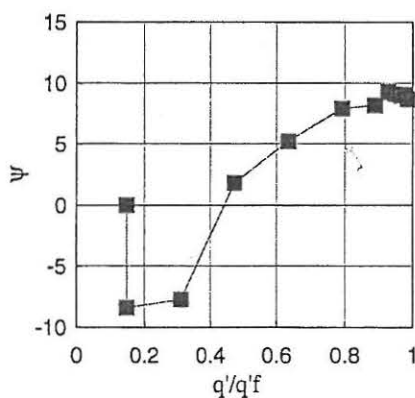
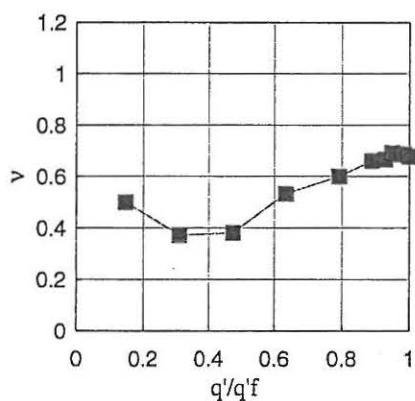


Job:	Encl. No
Blokhus sand	56
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure 0.771
		Grain density	26.7	
Calibration file	Date	Void ratio	2.65	
Loadtransducer 2003	19.02.74	Saturation	0.735	
		Dimension H mm	0.96	
		D mm	72	
			70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-20 kPa
		ϵ_1	0.02 %
		ϵ_v	0.04 %
	2. Drained compression.		
Deformation rate:			9.6 % ph

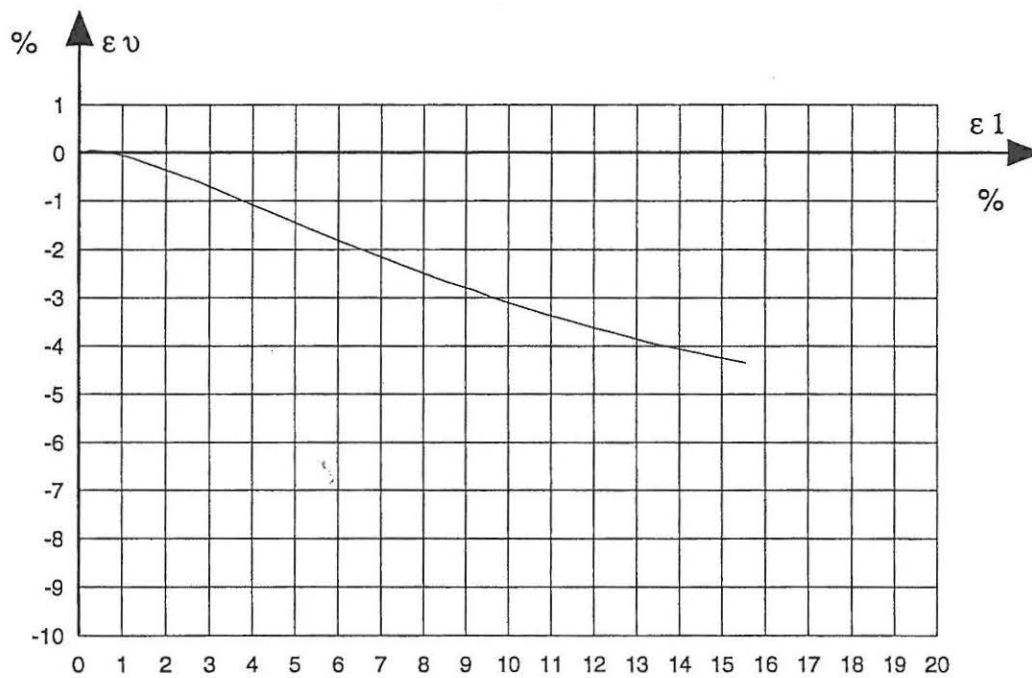
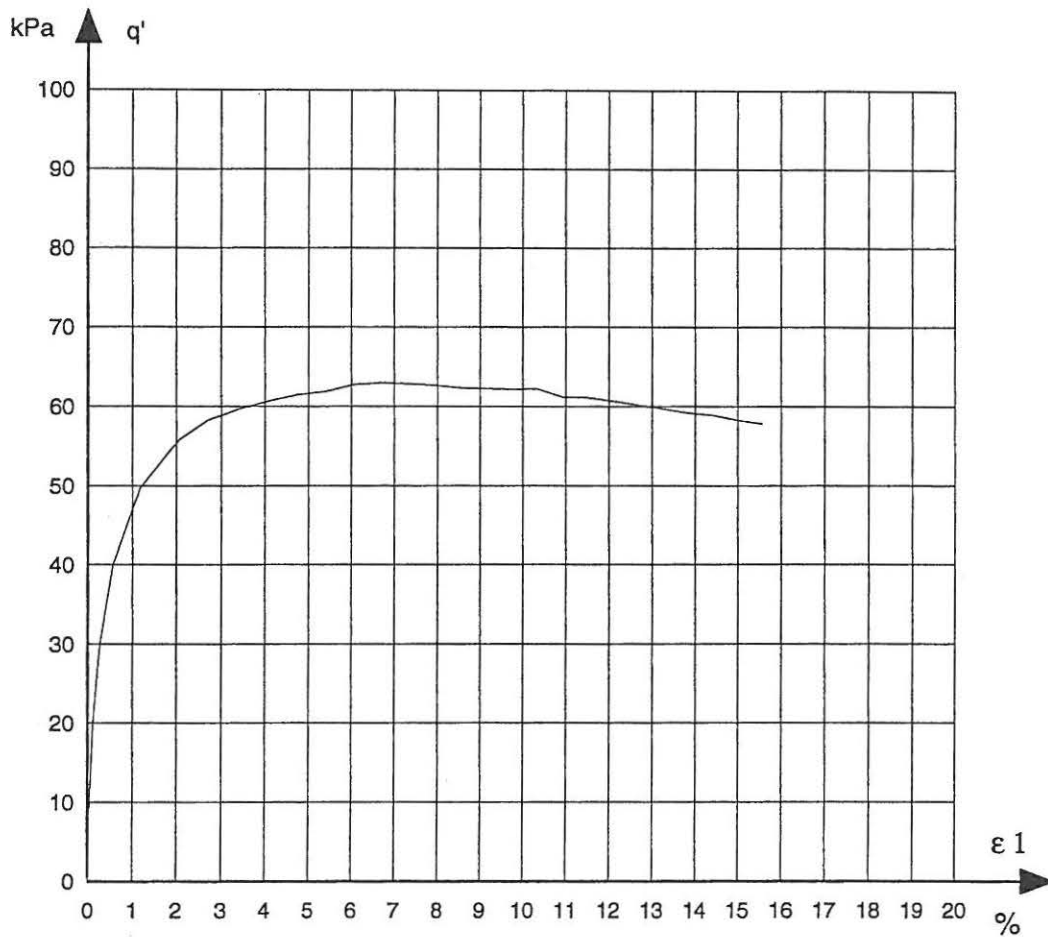
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	62.94	kPa	29.76	kPa
Mean normal stress	p'	40.98	kPa	29.92	kPa
Confining pressures	σ_3	20.00	kPa	20.00	kPa
Vertical strain	ϵ_1	6.71	%	0.26	%
Volumetric strain	ϵ_v	-2.06	%	0.05	%



q'	p'	ϵ_1	ϵ_v
9.20	23.07	0.00	0.00
9.22	23.07	0.04	0.00
19.50	26.50	0.11	0.02
29.76	29.92	0.26	0.05
39.92	33.31	0.54	0.04
49.78	36.59	1.17	-0.09
55.87	38.62	2.07	-0.38
58.32	39.44	2.72	-0.60
59.65	39.88	3.38	-0.85
60.65	40.22	4.05	-1.10
61.48	40.49	4.74	-1.35
61.90	40.63	5.41	-1.61
62.75	40.92	6.07	-1.84
62.94	40.98	6.71	-2.06
62.81	40.94	7.33	-2.27
62.65	40.88	7.93	-2.47
62.35	40.78	8.54	-2.67
62.25	40.75	9.14	-2.83
62.17	40.72	9.74	-3.03
61.21	40.40	10.92	-3.36
60.67	40.22	12.09	-3.65
59.72	39.91	13.25	-3.92
58.93	39.64	14.40	-4.13
57.84	39.28	15.56	-4.35

Job:	Encl. No
Blokhus sand	57
Exc:	Check:
HSP	LB

Remark:

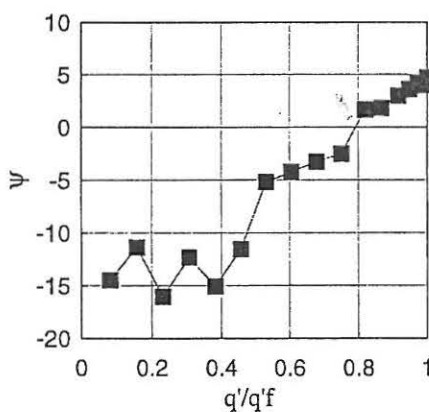
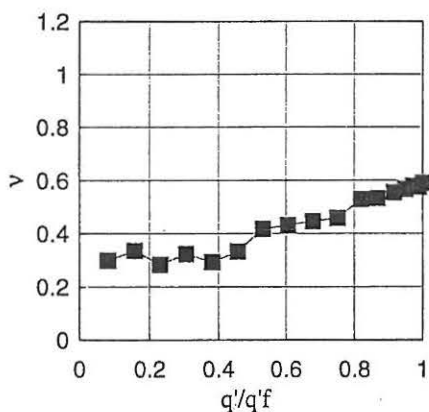


Job:	Encl. No
Blokhuss sand	58
Exc:	Check:
HSP	LB

Description of soil			Before test	At failure
Blokhus sand		Water content %	27.5	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.758	0.764
		Saturation	0.96	
Loadtransducer 5001	20.02.74	Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	20-400 kPa
		ϵ_1	0.23 %
		ϵ_v	0.72 %
	2. Drained compression.		
Deformation rate:			9.7 % ph

	Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress q'		655.77 kPa
Mean normal stress p'		618.59 kPa
Confining pressures σ_3		400.00 kPa
Vertical strain ϵ_1		1.82 %
Volumetric strain ϵ_v		0.36 %

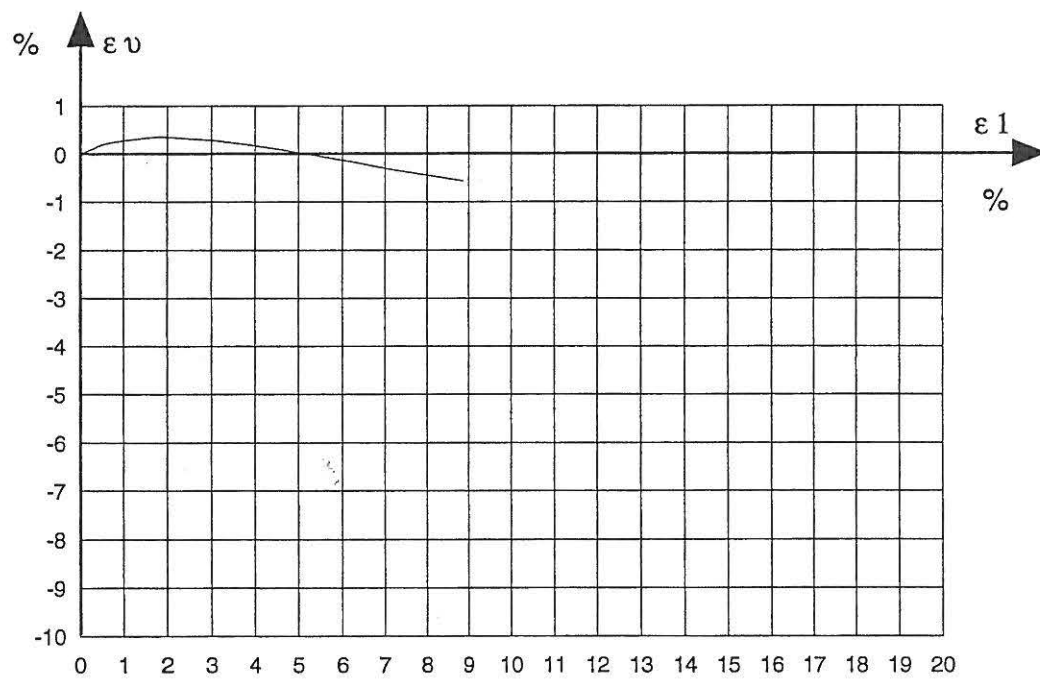
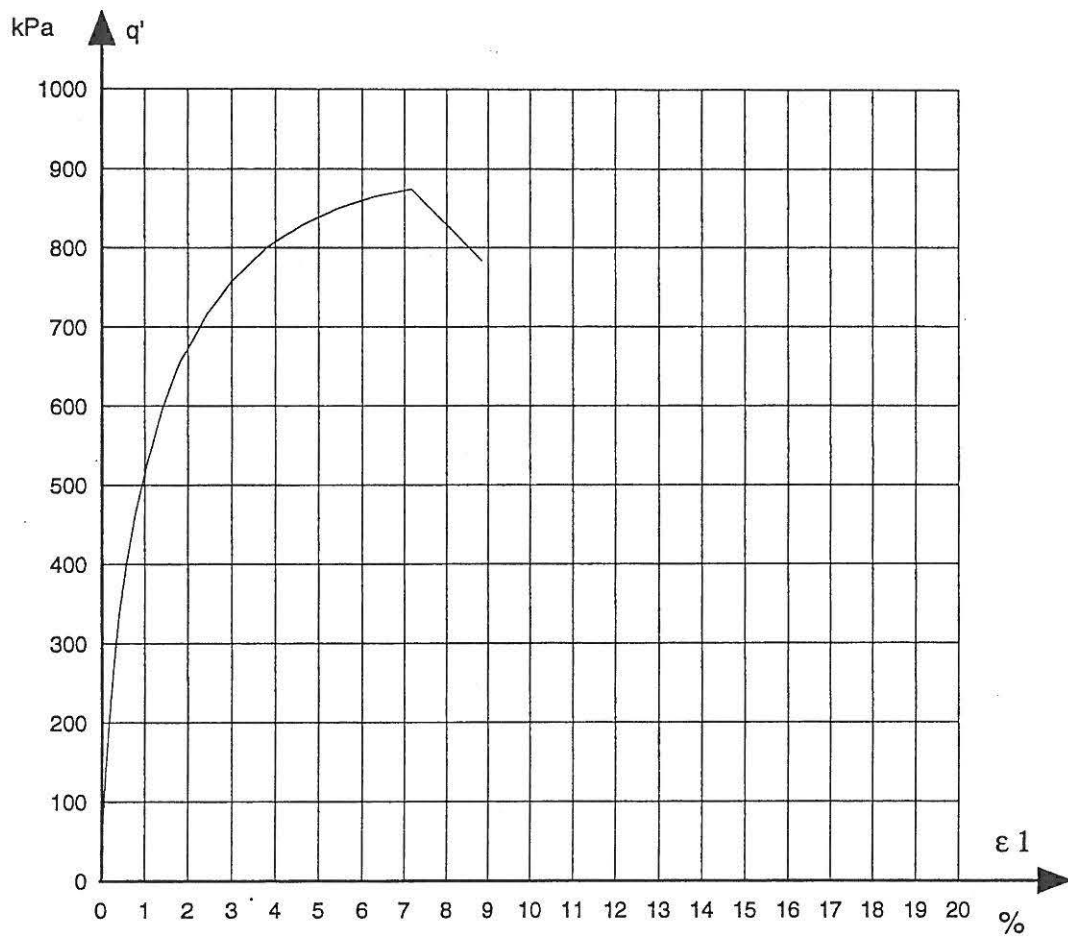


q'	p'	ϵ_1	ϵ_v
7.01	402.34	0.00	0.00
70.11	423.37	0.05	0.02
136.20	445.40	0.10	0.04
202.21	467.40	0.18	0.07
268.12	489.37	0.29	0.11
333.90	511.30	0.42	0.16
399.45	533.15	0.58	0.22
464.51	554.84	0.80	0.25
529.10	576.37	1.06	0.29
592.95	597.65	1.39	0.32
655.77	618.59	1.82	0.36
716.21	638.74	2.43	0.32
757.19	652.40	2.99	0.29
800.22	666.74	3.81	0.20
828.63	676.21	4.62	0.09
849.41	683.14	5.42	-0.04
864.32	688.11	6.26	-0.16
874.16	691.39	7.17	-0.32
783.08	661.03	8.86	-0.56

Job: Blokhus sand	Encl. No 59
Exc: HSP	Check: LB

Remark:

The specimen slipped out before failure.

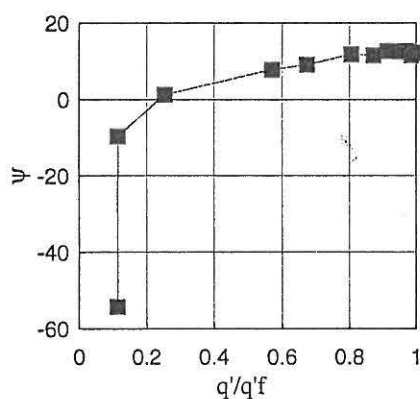
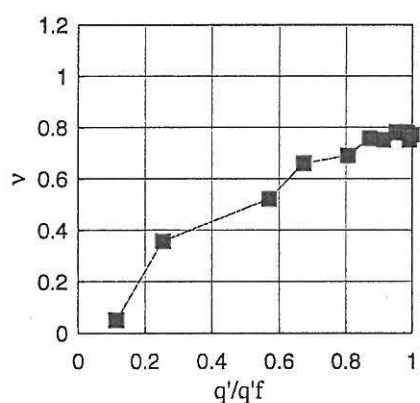


Job:	Encl. No
Blokhuis sand	60
Exc:	Check:
HSP	LB

Description of soil			Before test	At failure
Blokhus sand		Water content %	23.7	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.696	0.755
Loadtransducer 2003	21.02.74	Saturation	0.9	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-20 kPa
		ϵ_1	0.02 %
		ϵ_v	-0.02 %
	2. Drained compression.		
Deformation rate:			9.6 % ph

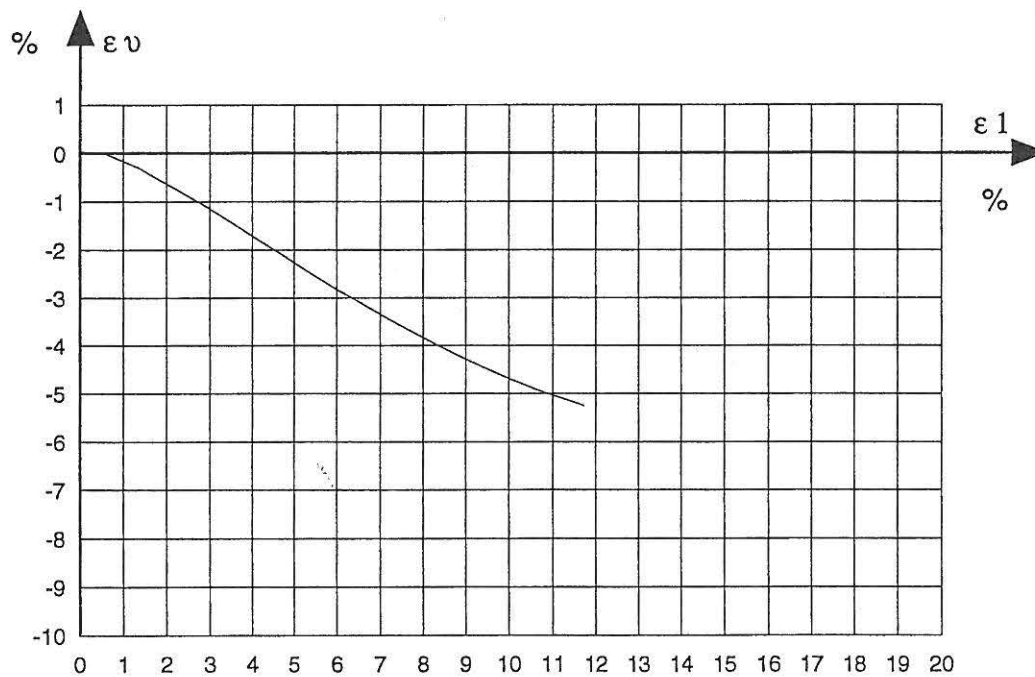
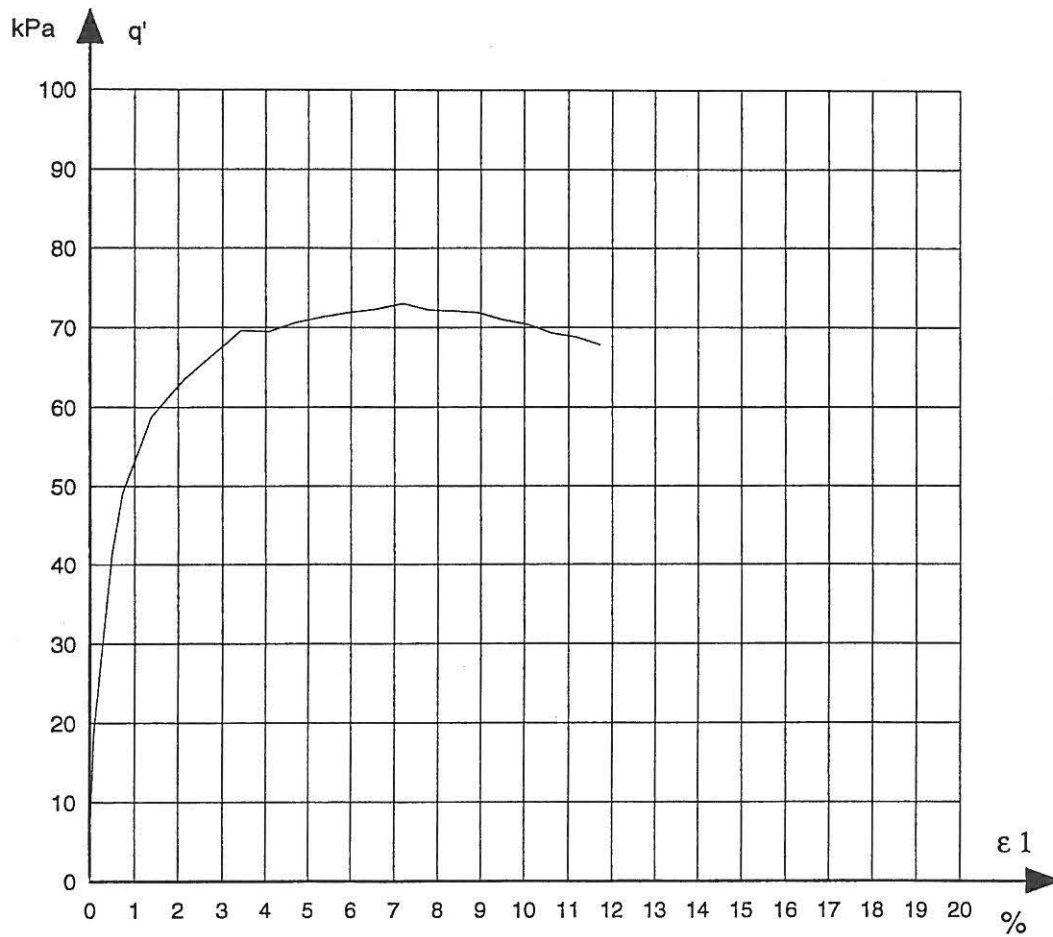
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	73.00	kPa	18.53	kPa
Mean normal stress	p'	44.33	kPa	26.18	kPa
Confining pressures	σ_3	20.00	kPa	20.00	kPa
Vertical strain	ϵ_1	7.19	%	0.08	%
Volumetric strain	ϵ_v	-3.45	%	0.04	%



q'	p'	ϵ_1	ϵ_v
8.20	22.73	0.00	0.00
8.24	22.75	0.02	0.02
18.53	26.18	0.08	0.04
41.52	33.84	0.48	0.02
49.06	36.35	0.71	-0.05
58.74	39.58	1.38	-0.31
63.56	41.19	2.15	-0.70
66.60	42.20	2.79	-1.03
69.61	43.20	3.43	-1.39
69.48	43.16	4.08	-1.75
70.64	43.55	4.72	-2.11
71.35	43.78	5.36	-2.47
71.92	43.97	5.98	-2.81
72.31	44.10	6.59	-3.12
73.00	44.33	7.19	-3.45
72.24	44.08	7.78	-3.74
72.10	44.03	8.36	-4.01
71.89	43.96	8.93	-4.26
70.97	43.66	9.50	-4.49
70.42	43.47	10.06	-4.71
69.33	43.11	10.61	-4.91
68.83	42.94	11.17	-5.09
67.87	42.62	11.71	-5.25

Job:	Encl. No
Blokhus sand	61
Exc:	Check:
HSP	LB

Remark:

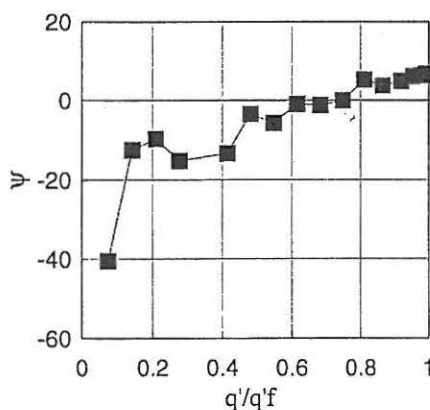
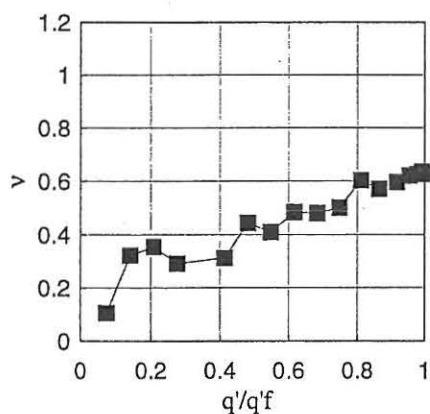


Job:	Encl. No
Blokhus sand	62
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	25.8	
Calibration file	Date	Void ratio	2.65	
		Saturation	0.719	0.731
Loadtransducer	21.02.74	Dimension H mm	0.95	
5001		D mm	72	
			70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	20-400 kPa
		ϵ_1	0.27 %
		ϵ_v	0.60 %
	2. Drained compression.		
	Deformation rate:		9.7 % ph

		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'		721.38 kPa
Mean normal stress	p'		640.46 kPa
Confining pressures	σ_3		400.00 kPa
Vertical strain	ϵ_1		1.68 %
Volumetric strain	ϵ_v		0.27 %

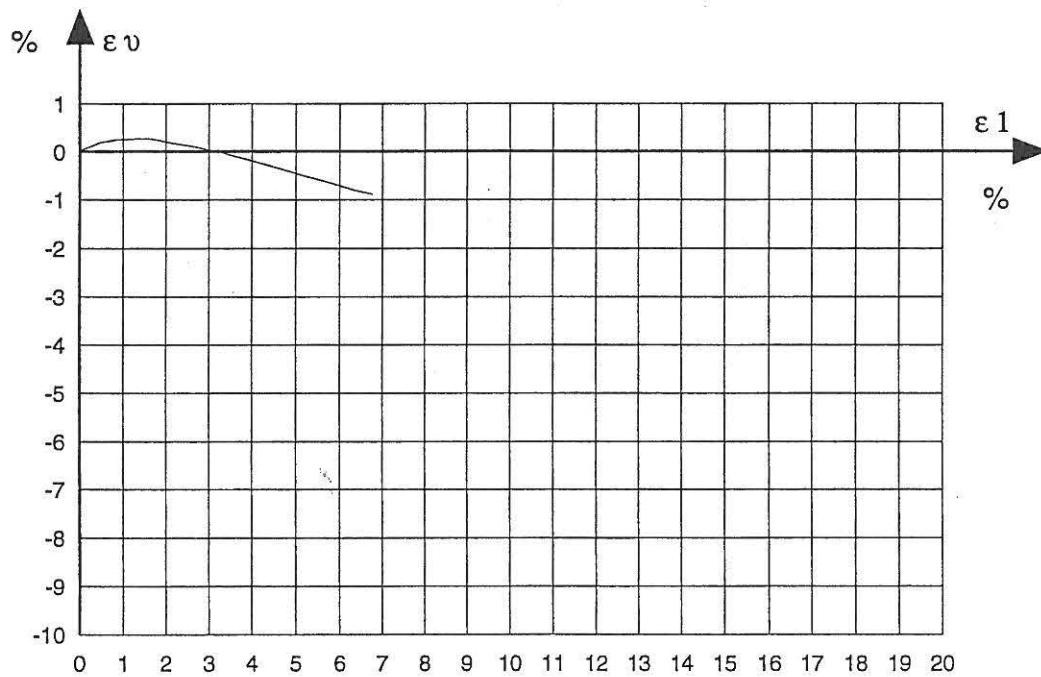
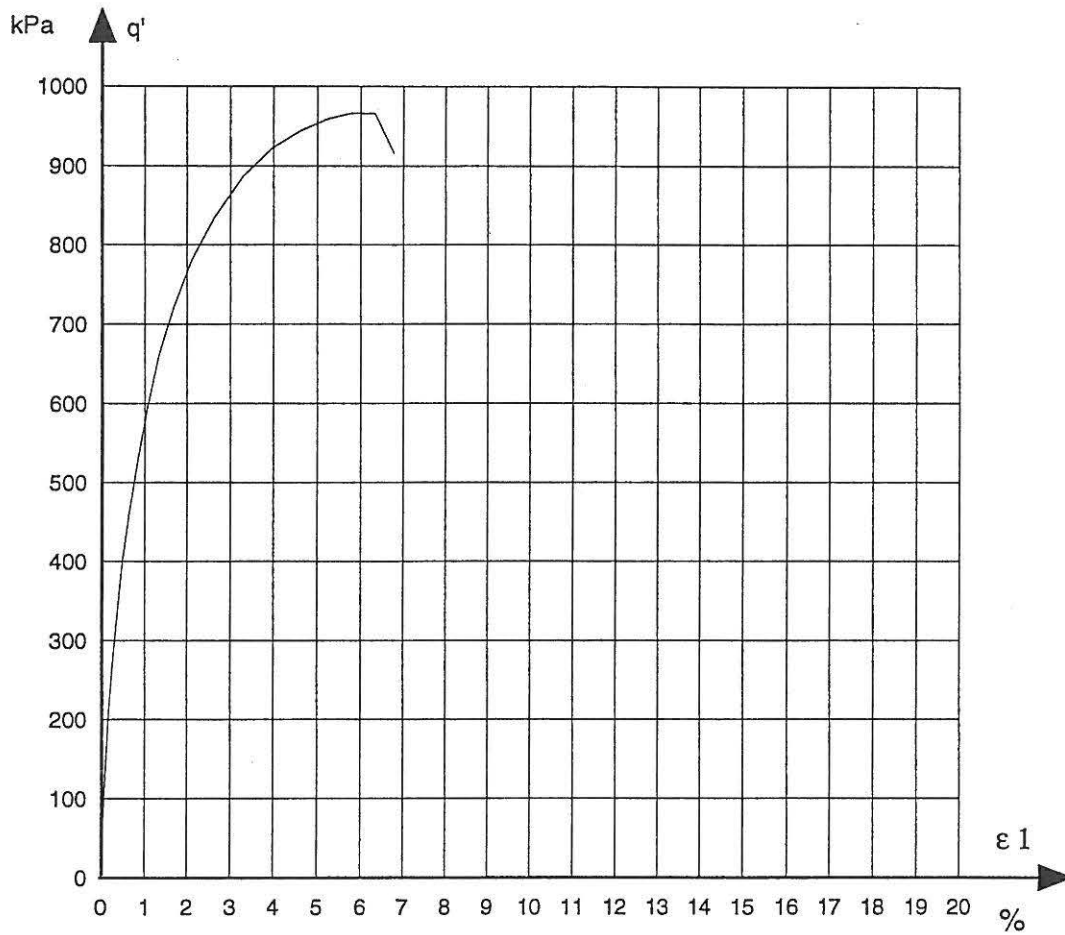


q'	p'	ϵ_1	ϵ_v
70.10	423.37	0.00	0.00
70.12	423.37	0.05	0.04
136.22	445.41	0.10	0.05
202.26	467.42	0.16	0.07
268.23	489.41	0.25	0.11
399.75	533.25	0.49	0.20
465.06	555.02	0.65	0.22
530.06	576.69	0.84	0.25
594.55	598.18	1.06	0.26
658.45	619.48	1.33	0.27
721.38	640.46	1.68	0.27
782.35	660.78	2.12	0.18
834.45	678.15	2.63	0.11
886.59	695.53	3.29	-0.02
921.47	707.16	3.97	-0.18
943.72	714.57	4.61	-0.34
958.31	719.44	5.22	-0.51
966.12	722.04	5.79	-0.65
965.53	721.84	6.33	-0.79
914.47	704.82	6.78	-0.88

Job:	Encl. No
Blokhus sand	63
Exc:	Check:
HSP	LB

Remark:

The specimen slipped out before failure.

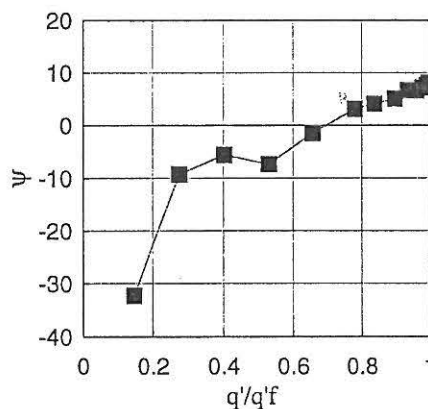
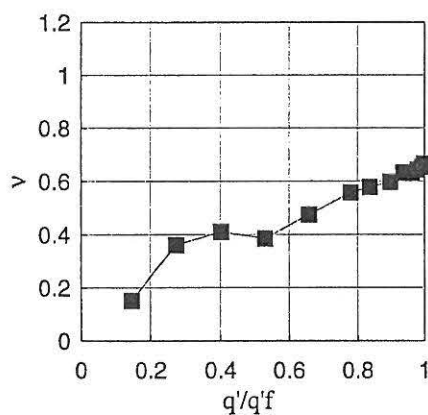


Job:	Blokhuis sand	Encl. No	64
Exc:	HSP	Check:	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	26.7	
Calibration file	Date	Void ratio	2.65	
Loadtransducer 5001	26.02.74	Saturation	0.723	
		Dimension H mm	0.98	
		D mm	72	
			70	0.747

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	20-200 kPa
		ϵ_1	0.13 %
		ϵ_v	0.56 %
	2. Drained compression.		
		Deformation rate:	9.7 % ph

	Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress q'		336.01 kPa
Mean normal stress p'		312.00 kPa
Confining pressures σ_3		200.00 kPa
Vertical strain ϵ_1		1.00 %
Volumetric strain ϵ_v		0.20 %

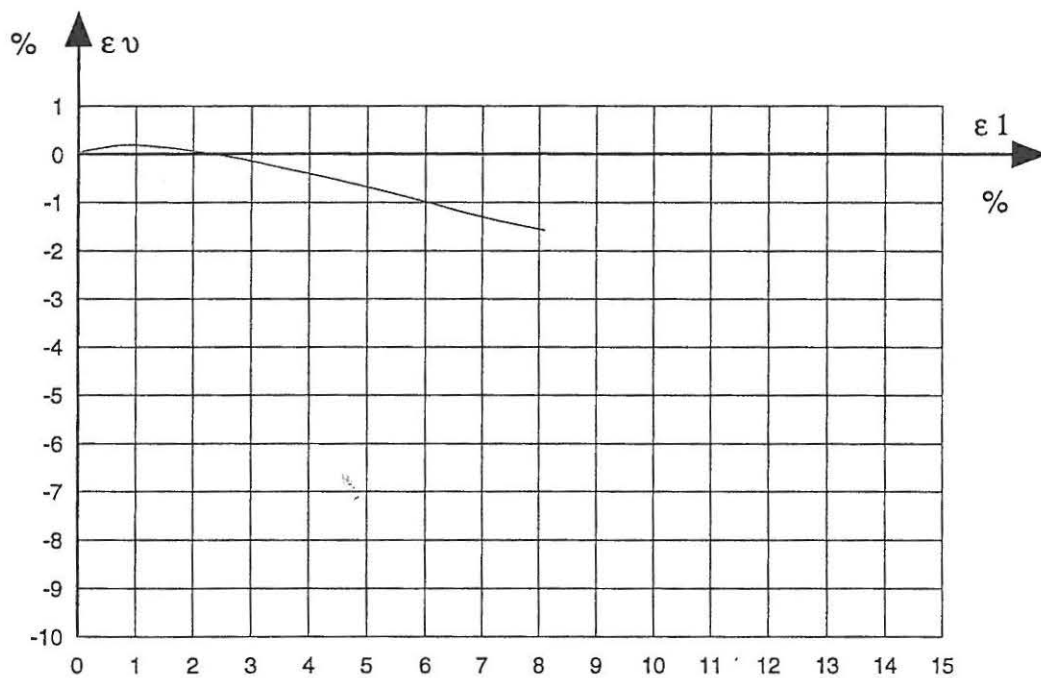
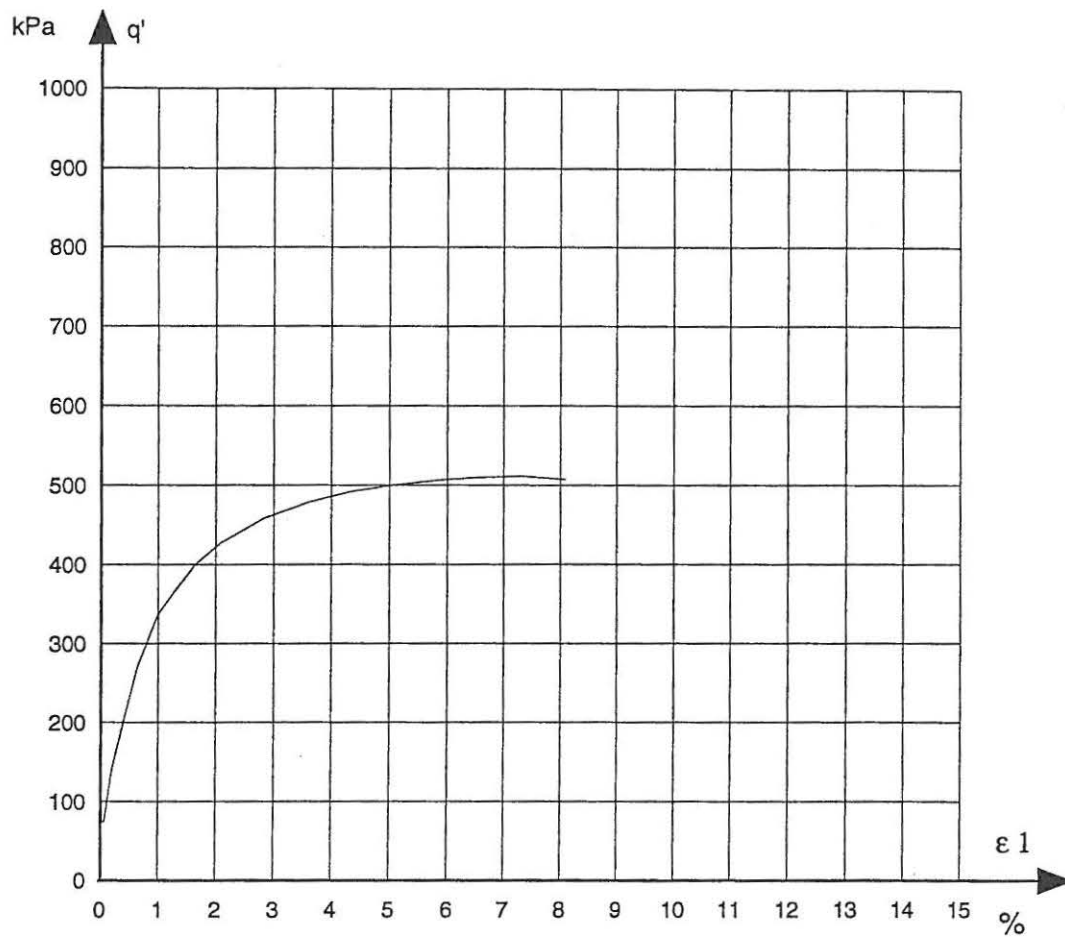


q'	p'	ϵ_1	ϵ_v
74.10	224.70	0.00	0.00
74.08	224.69	0.08	0.05
140.09	246.70	0.21	0.09
205.82	268.61	0.41	0.13
271.29	290.43	0.65	0.18
336.01	312.00	1.00	0.20
398.80	332.93	1.63	0.13
427.74	342.58	2.09	0.05
458.94	352.98	2.84	-0.09
478.53	359.51	3.59	-0.29
492.35	364.12	4.34	-0.49
500.48	366.83	5.10	-0.70
506.90	368.97	5.92	-0.96
510.12	370.04	6.57	-1.17
511.64	370.55	7.31	-1.39
507.46	369.15	8.10	-1.57

Job:	Encl. No
Blokhus sand	65
Exc:	Check:
HSP	LB

Remark:

The specimen slipped out before failure.

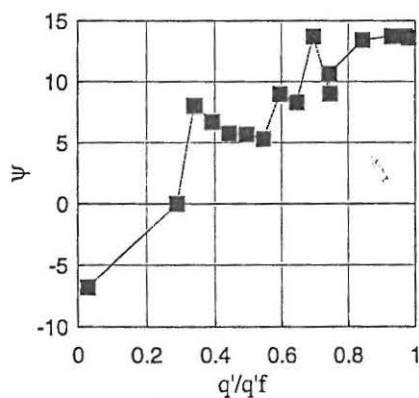
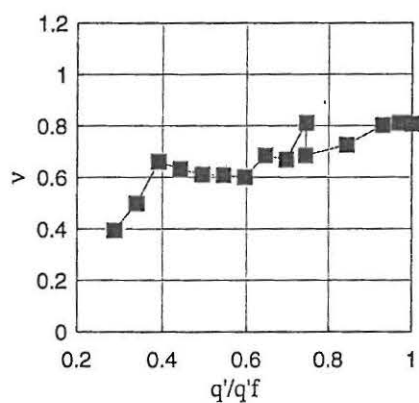


Job:	Encl. No
Blokhus sand	66
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.2	
Calibration file	Date	Void ratio	0.699	0.747
Loadtransducer	26.02.74	Saturation	0.92	
181		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-10 kPa
		ϵ_1	0.00 %
		ϵ_v	0.00 %
	2. Drained compression.	Deformation rate: 9.7 % ph	

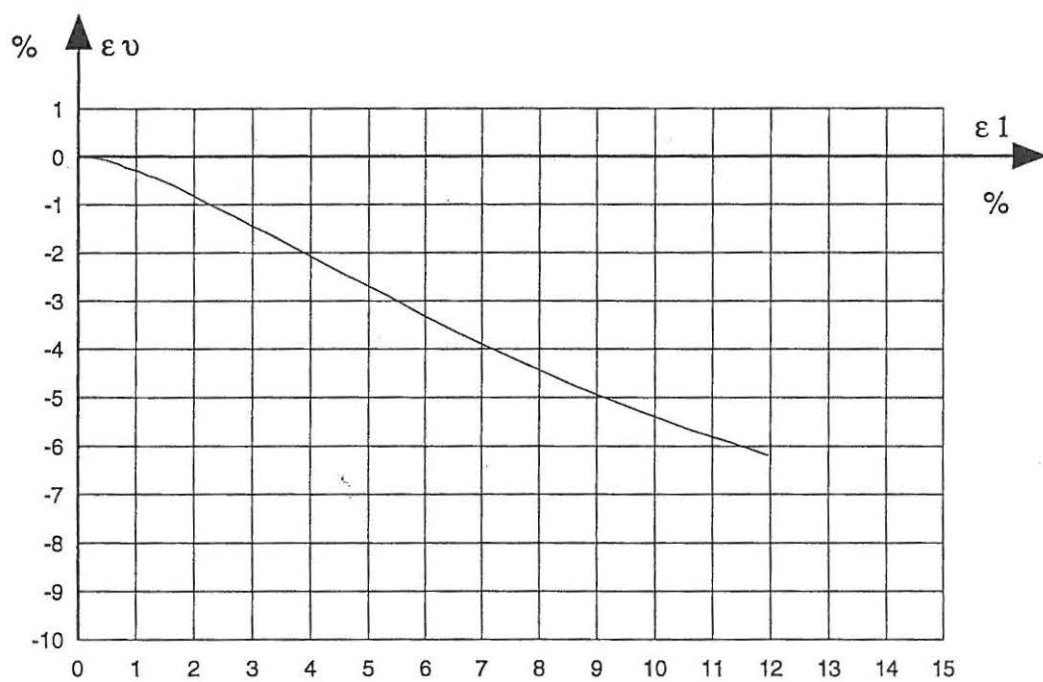
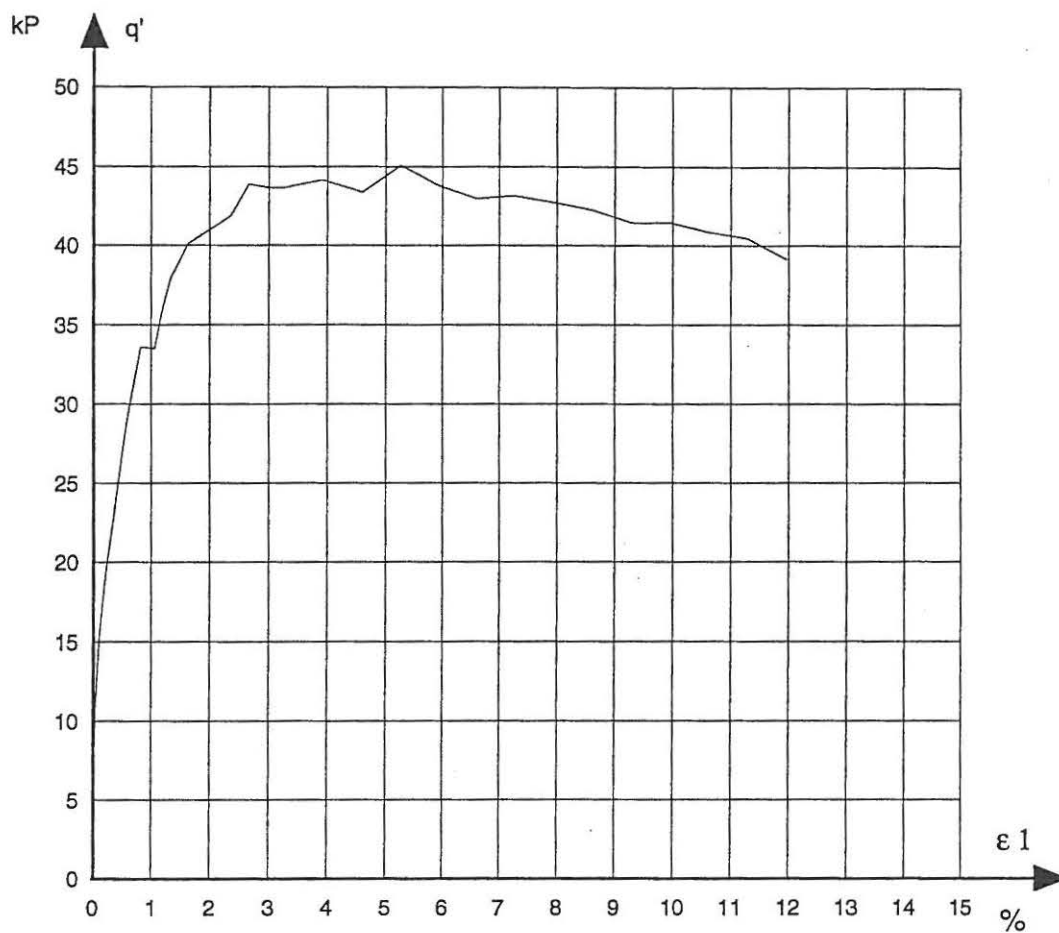
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	45.05 kPa	15.30 kPa
Mean normal stress	p'	24.37 kPa	14.45 kPa
Confining pressures	σ_3	9.35 kPa	9.35 kPa
Vertical strain	ϵ_1	5.26 %	0.13 %
Volumetric strain	ϵ_v	-2.85 %	0.02 %



q'	p'	ϵ_1	ϵ_v
1.31	9.79	0.00	0.00
12.97	13.67	0.09	0.02
15.30	14.45	0.13	0.02
17.61	15.22	0.18	0.00
19.93	15.99	0.25	-0.02
22.23	16.76	0.33	-0.04
24.53	17.53	0.41	-0.05
26.82	18.29	0.50	-0.07
29.11	19.05	0.60	-0.11
31.38	19.81	0.70	-0.14
33.63	20.56	0.82	-0.22
33.52	20.52	1.06	-0.31
37.96	22.00	1.34	-0.43
41.86	23.30	2.35	-1.05
43.65	23.90	2.99	-1.44
44.15	24.07	3.92	-2.02
45.05	24.37	5.26	-2.85
42.95	23.67	6.60	-3.68
42.72	23.59	7.95	-4.40
41.43	23.16	9.29	-5.07
41.45	23.17	9.95	-5.38
40.86	22.97	10.62	-5.67
40.45	22.83	11.29	-5.92
39.12	22.39	11.97	-6.19

Job:	Encl. No
Blokhus sand	67
Exc:	Check:
HSP	LB

Remark:
No isotropic consolidation.

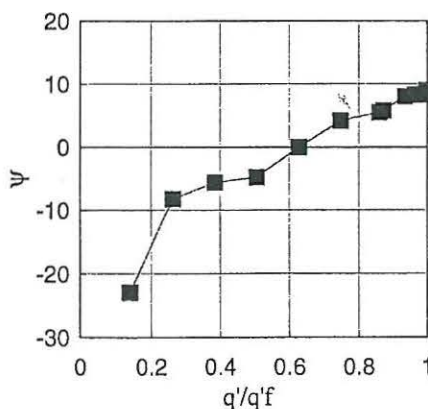
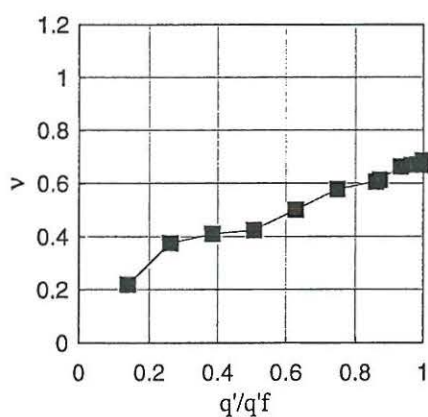


Job:	Encl. No
Blokhus sand	68
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.7	
Calibration file	Date	Void ratio	0.734	
Loadtransducer 5001	27.02.74	Saturation	0.89	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.	σ_3	20-200 kPa	
		ϵ_1	0.26 %	
		ϵ_v	0.24 %	
	2. Drained compression.			
Deformation rate:			9.7 % ph	

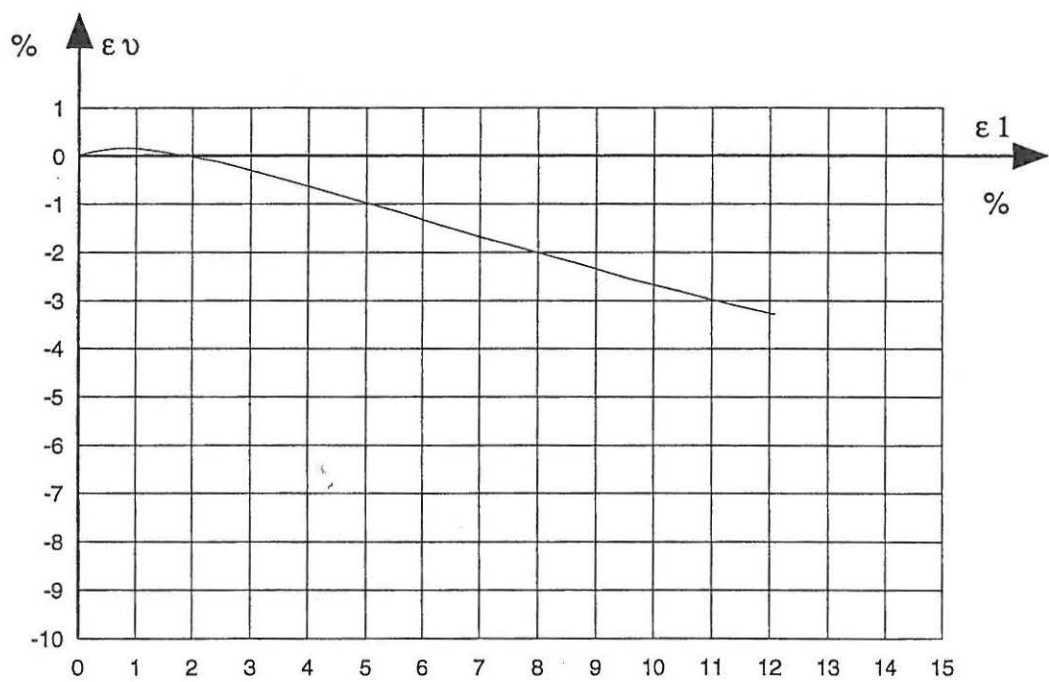
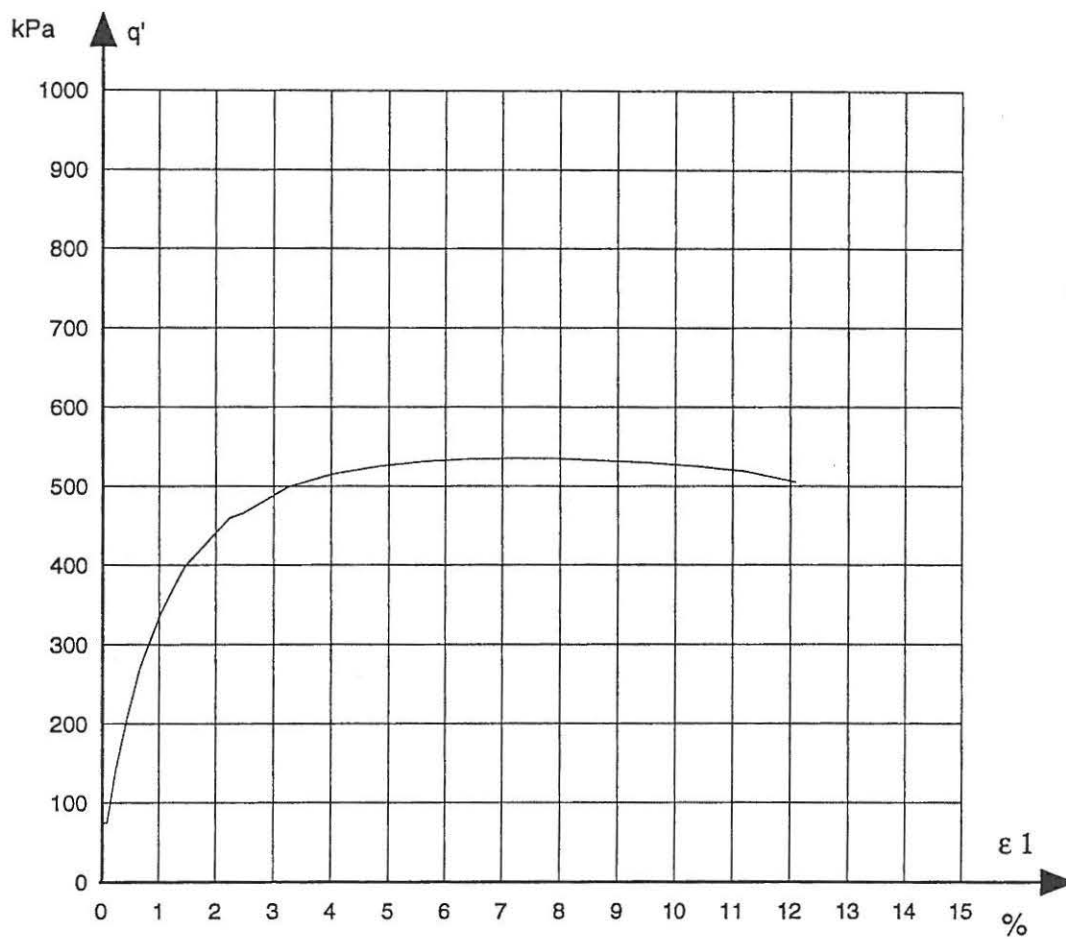
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	535.27	kPa	335.85	kPa
Mean normal stress	p'	378.42	kPa	311.95	kPa
Confining pressures	σ_3	200.00	kPa	200.00	kPa
Vertical strain	ϵ_1	7.88	%	1.01	%
Volumetric strain	ϵ_v	-1.97	%	0.16	%



q'	p'	ϵ_1	ϵ_v
74.10	224.70	0.00	0.00
74.06	224.69	0.10	0.05
140.04	246.68	0.24	0.09
205.75	268.58	0.45	0.13
271.15	290.38	0.68	0.16
335.85	311.95	1.01	0.16
399.28	333.09	1.47	0.09
460.16	353.39	2.24	-0.07
466.38	355.46	2.48	-0.13
499.76	366.59	3.26	-0.38
515.60	371.87	4.06	-0.65
525.45	375.15	4.85	-0.92
531.11	377.04	5.60	-1.17
533.91	377.97	6.33	-1.44
535.25	378.42	7.10	-1.71
535.27	378.42	7.88	-1.97
533.00	377.67	8.70	-2.24
529.65	376.55	9.57	-2.54
525.22	375.07	10.42	-2.80
518.57	372.86	11.24	-3.05
505.05	368.35	12.10	-3.28

Job:	Encl. No
Blokhus sand	69
Exc:	Check:
HSP	LB

Remark:
The specimen slipped out after failure.

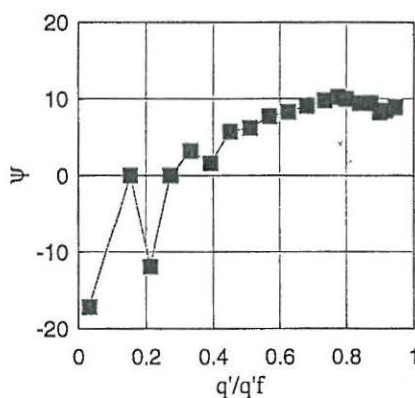
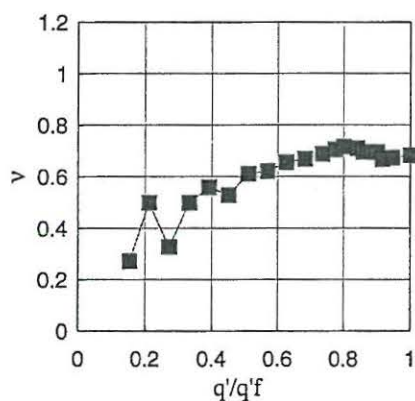


Job:	Encl. No
Blokhus sand	70
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	26.1	
Calibration file	Date	Void ratio	0.724	0.781
Loadtransducer 181	27.02.74	Saturation	0.95	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-10 kPa
		ϵ_1	0.00 %
		ϵ_v	0.00 %
	2. Drained compression.		
	Deformation rate:		9.7 % ph

		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	38.80	kPa	12.88	kPa
Mean normal stress	p'	22.28	kPa	13.64	kPa
Confining pressures	σ_3	9.35	kPa	9.35	kPa
Vertical strain	ϵ_1	9.45	%	0.20	%
Volumetric strain	ϵ_v	-3.30	%	0.04	%

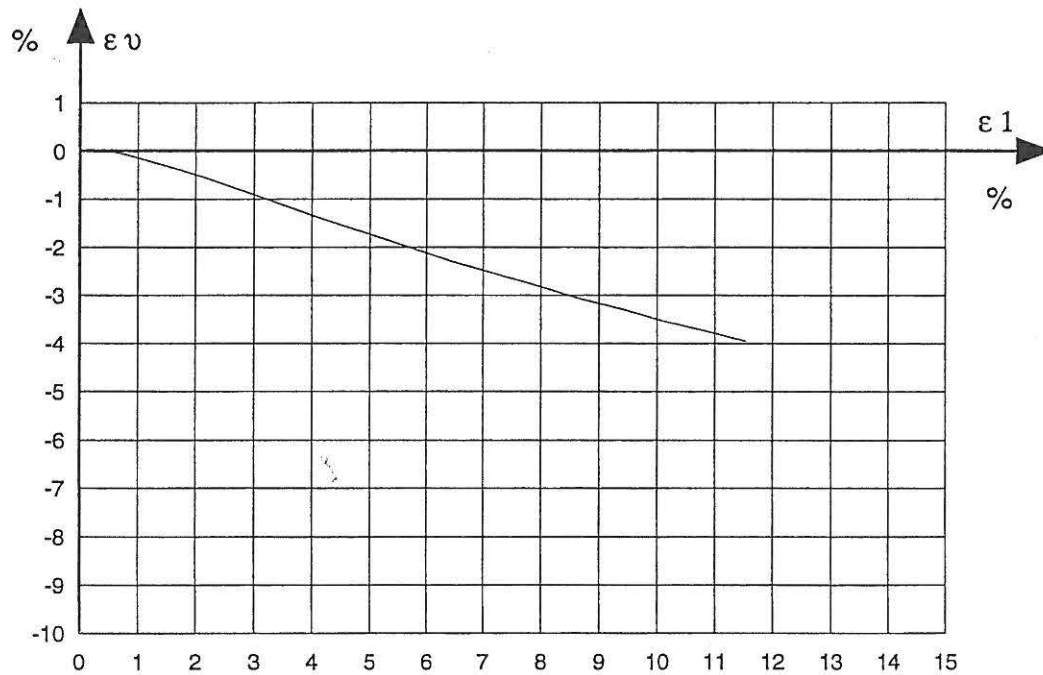
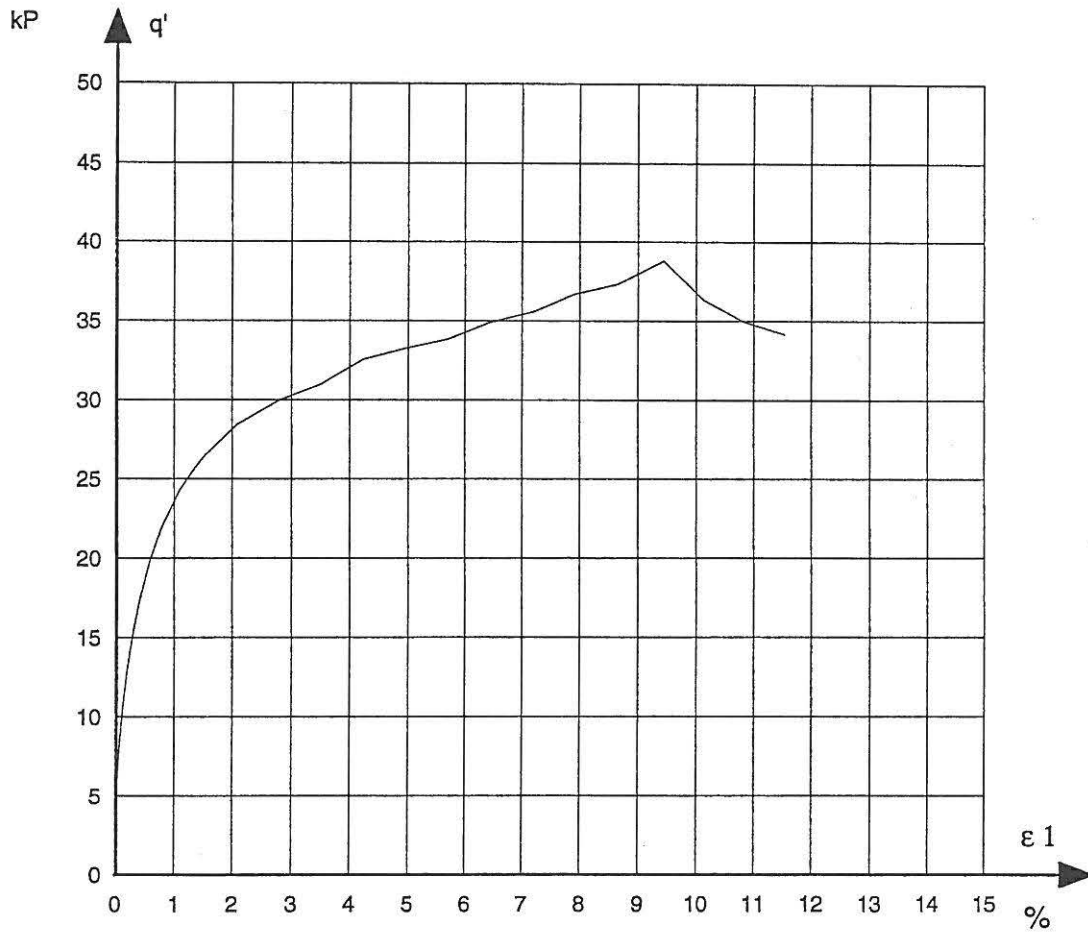


q'	p'	ϵ_1	ϵ_v
1.24	9.76	0.00	0.00
5.90	11.32	0.04	0.02
8.23	12.09	0.08	0.02
10.56	12.87	0.13	0.04
12.88	13.64	0.20	0.04
15.20	14.42	0.29	0.03
17.50	15.18	0.41	0.02
19.79	15.95	0.58	-0.02
22.04	16.70	0.80	-0.07
24.26	17.44	1.09	-0.16
26.42	18.16	1.51	-0.31
28.48	18.84	2.09	-0.52
29.99	19.35	2.79	-0.81
30.98	19.68	3.50	-1.12
32.57	20.21	4.23	-1.43
33.26	20.44	4.96	-1.71
33.83	20.63	5.70	-2.00
34.90	20.98	6.43	-2.29
35.55	21.20	7.19	-2.54
36.70	21.58	7.92	-2.80
38.80	21.79	8.65	-3.07
36.36	22.28	9.45	-3.30
34.97	21.47	10.12	-3.54
34.17	21.01	10.81	-3.74

Job:	Encl. No
Blokhus sand	71
Exc:	Check:
HSP	LB

Remark:

No isotropic consolidation.

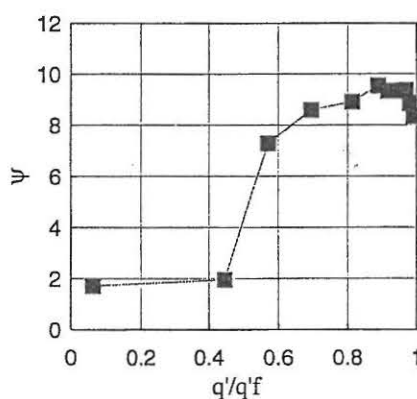
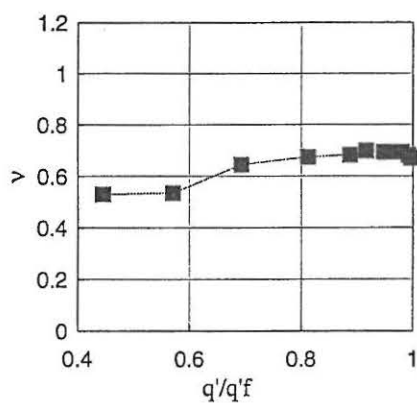


Job:	Encl. No
Blokhuis sand	72
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	26.7	
Calibration file	Date	Void ratio	0.741	
Loadtransducer 181	28.02.74	Saturation	0.96	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-4.35 kPa
		ϵ_1	-0.003 %
		ϵ_v	-0.054 %
	2. Drained compression.		
Deformation rate:			9.7 % ph

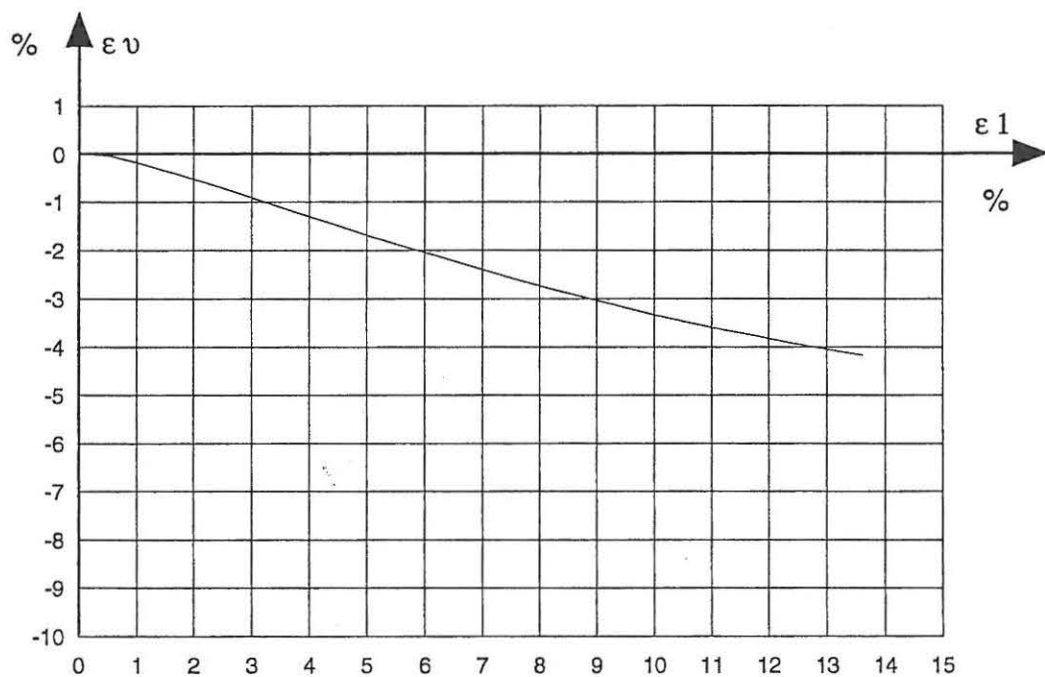
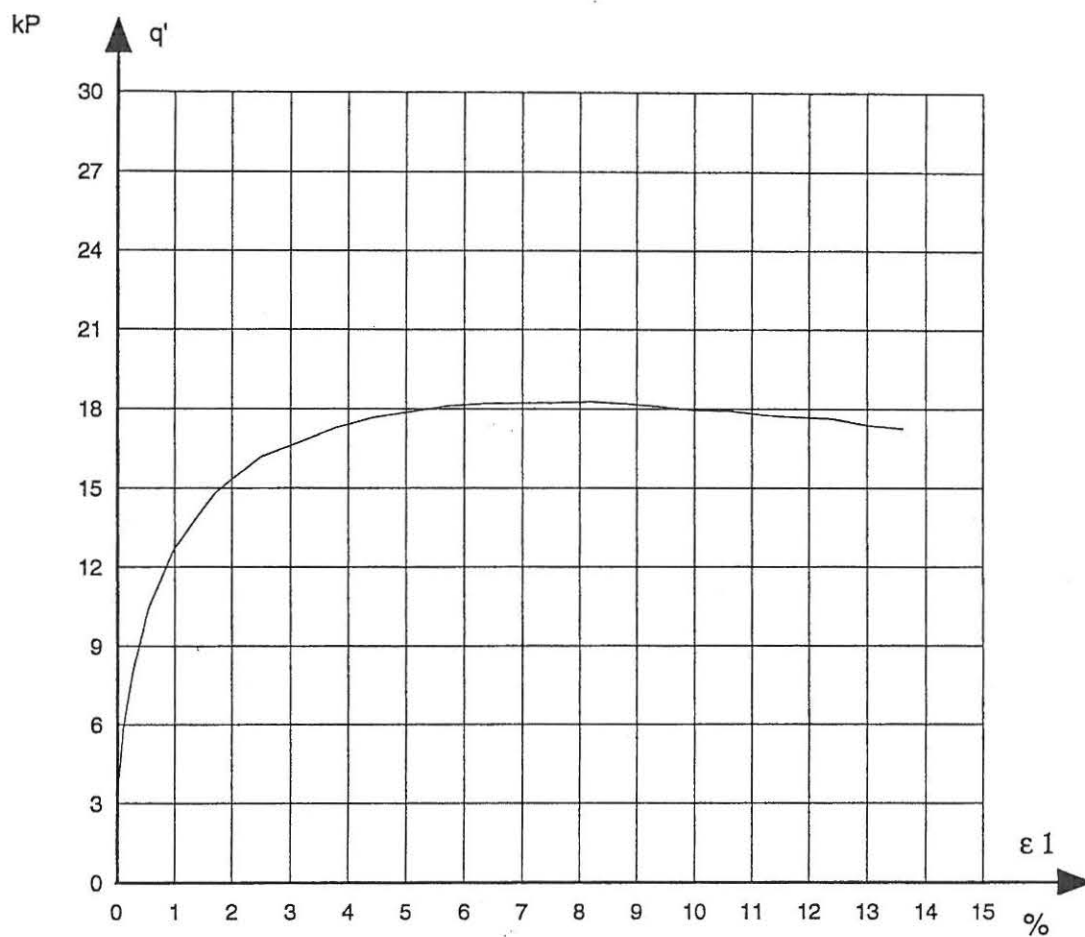
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	18.27	kPa	1.14	kPa
Mean normal stress	p'	10.44	kPa	4.73	kPa
Confining pressures	σ_3	4.35	kPa	4.35	kPa
Vertical strain	ϵ_1	8.19	%	0.00	%
Volumetric strain	ϵ_v	-2.80	%	0.00	%



q'	p'	ϵ_1	ϵ_v
1.14	4.73	0.00	0.00
8.12	7.06	0.29	-0.02
10.42	7.82	0.54	-0.04
12.66	8.57	0.98	-0.16
14.83	9.29	1.70	-0.42
16.19	9.75	2.49	-0.70
16.72	9.92	3.12	-0.96
17.29	10.11	3.77	-1.21
17.67	10.24	4.42	-1.46
17.89	10.31	5.07	-1.71
18.11	10.39	5.72	-1.95
18.20	10.42	6.35	-2.17
18.22	10.42	6.96	-2.38
18.24	10.43	7.58	-2.60
18.27	10.44	8.19	-2.80
18.20	10.42	8.80	-2.98
18.09	10.38	9.41	-3.16
17.94	10.33	10.02	-3.34
17.92	10.32	10.62	-3.50
17.77	10.27	11.22	-3.65
17.70	10.25	11.81	-3.77
17.64	10.23	12.41	-3.92
17.39	10.15	13.00	-4.04
17.27	10.11	13.62	-4.17

Job:	Encl. No
Blokhus sand	73
Exc:	Check:
HSP	LB

Remark:

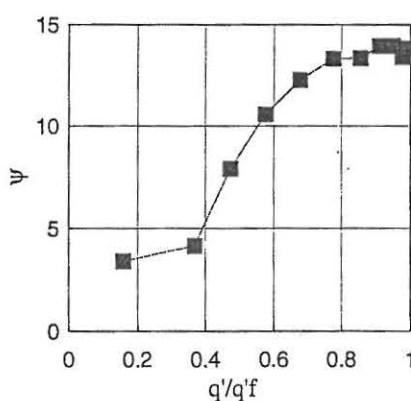
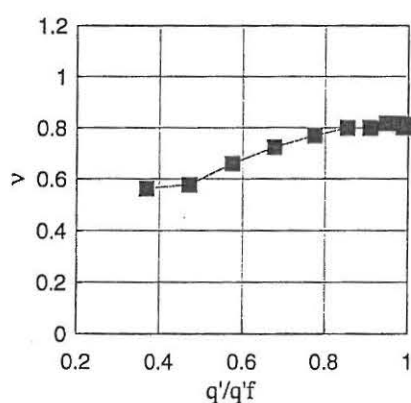


Job:	Encl. No
Blokhus sand	74
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.8	
Calibration file	Date	Void ratio	0.682	0.740
Loadtransducer	01.03.74	Saturation	0.96	
181		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-4.35 kPa
		ϵ_1	-0.003 %
		ϵ_v	-0.036 %
	2. Drained compression.		
	Deformation rate:		9.7 % ph

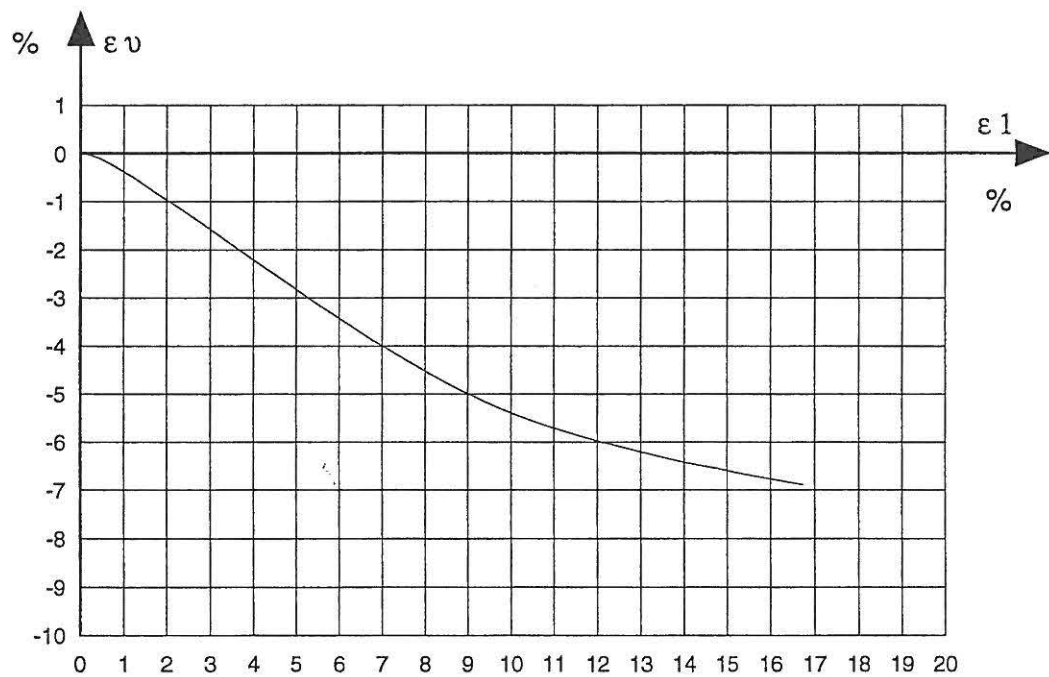
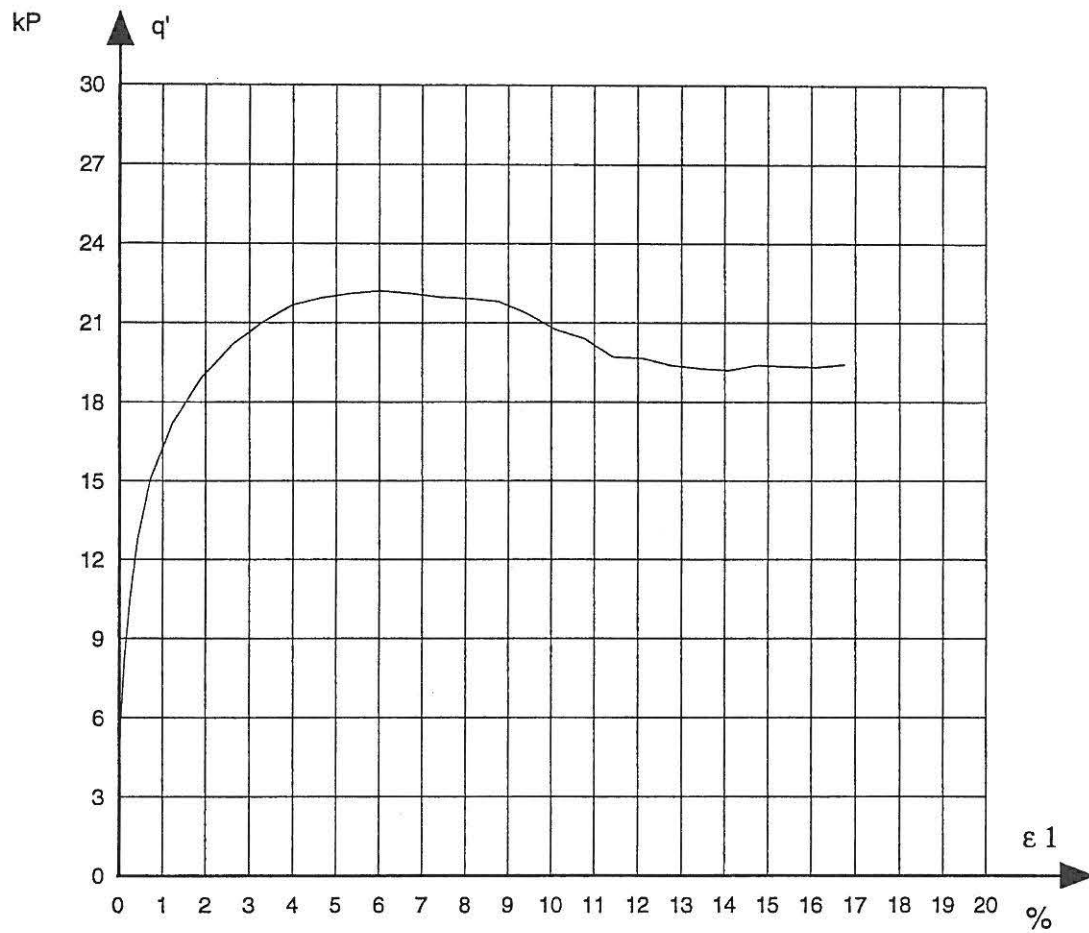
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	22.20	kPa	3.50	kPa
Mean normal stress	p'	11.75	kPa	5.52	kPa
Confining pressures	σ_3	4.35	kPa	4.35	kPa
Vertical strain	ϵ_1	6.04	%	0.00	%
Volumetric strain	ϵ_v	-3.45	%	0.00	%



q'	p'	ϵ_1	ϵ_v
3.50	5.52	0.00	0.00
8.15	7.07	0.14	-0.02
10.47	7.84	0.26	-0.04
12.77	8.61	0.43	-0.09
15.03	9.36	0.71	-0.22
17.20	10.08	1.23	-0.50
18.96	10.67	1.93	-0.92
20.18	11.08	2.62	-1.34
21.02	11.36	3.30	-1.77
21.66	11.57	3.98	-2.20
21.94	11.66	4.67	-2.62
22.11	11.72	5.36	-3.05
22.20	11.75	6.04	-3.45
22.10	11.72	6.72	-3.84
21.96	11.67	7.40	-4.20
21.91	11.65	8.08	-4.57
21.80	11.62	8.77	-4.89
21.36	11.47	9.44	-5.18
20.75	11.27	10.10	-5.43
19.72	11.16	10.78	-5.65
19.40	10.92	11.43	-5.83
19.20	10.91	12.09	-6.01
19.35	10.82	12.75	-6.15
19.42	10.78	13.41	-6.30

Job:	Encl. No
Blokhus sand	75
Exc:	Check:
HSP	LB

Remark:

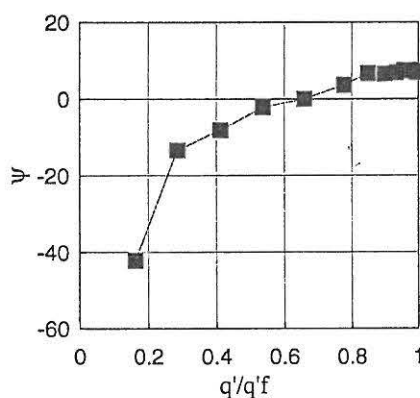
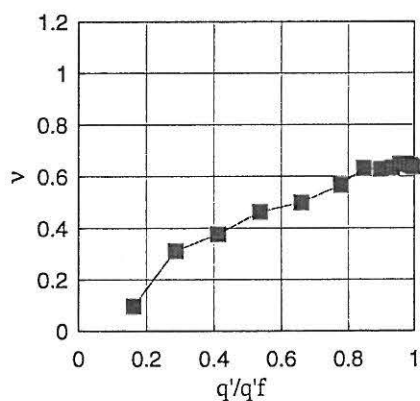


Job:	Encl. No
Blokhuis sand	76
Exc:	Check:
HSP	LB

Description of soil			Before test	At failure
Blokhus sand		Water content %	26.3	0.771
		Grain density	2.65	
Calibration file	Date	Void ratio	0.727	
		Saturation	0.96	
Loadtransducer	01.03.74	Dimension H mm	72	
5001		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	20-100 kPa
		ϵ_1	0.07 %
		ϵ_v	0.22 %
	2. Drained compression.		
Deformation rate:			9.7 % ph

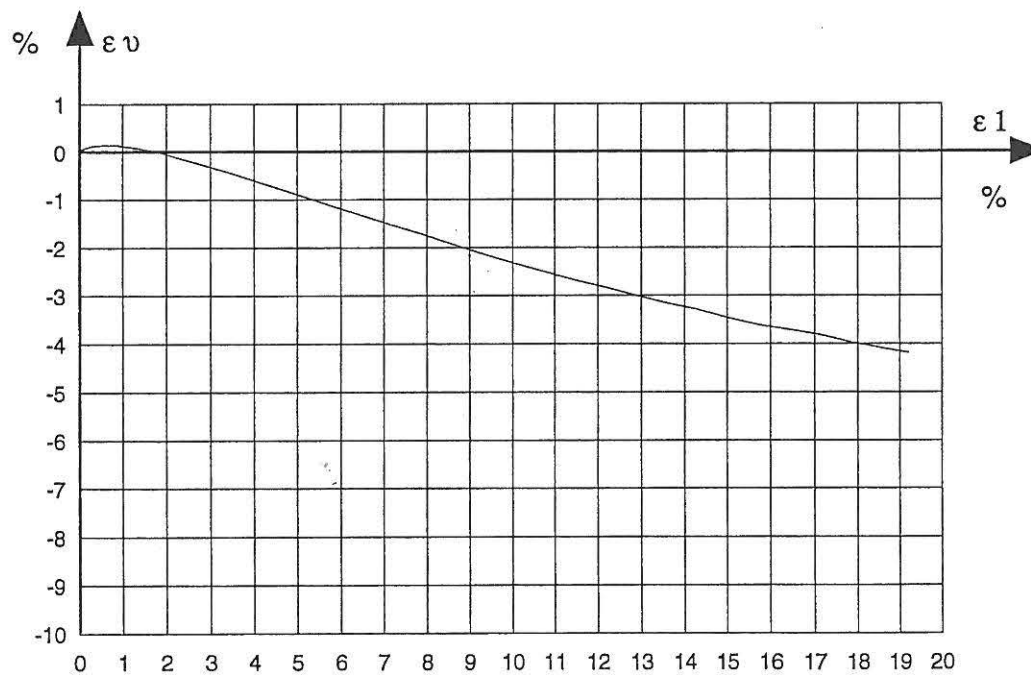
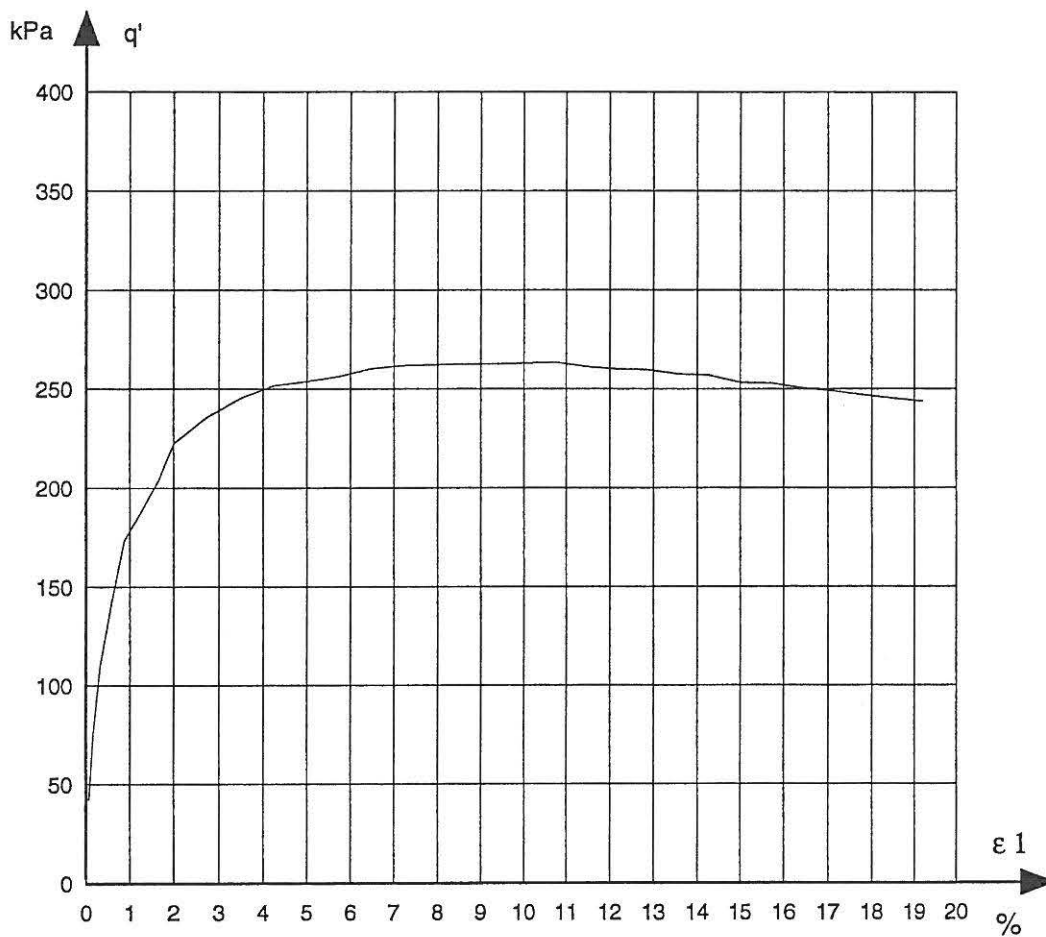
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	263.25	kPa	173.38	kPa
Mean normal stress	p'	187.75	kPa	157.79	kPa
Confining pressures	σ_3	100.00	kPa	100.00	kPa
Vertical strain	ϵ_1	10.78	%	0.87	%
Volumetric strain	ϵ_v	-2.51	%	0.14	%



q'	p'	ϵ_1	ϵ_v
42.30	114.10	0.00	0.00
42.33	114.11	0.07	0.05
75.36	125.12	0.16	0.09
108.30	136.10	0.31	0.13
140.99	147.00	0.55	0.14
173.38	157.79	0.87	0.14
204.33	168.11	1.67	0.04
222.88	174.29	2.01	-0.05
236.21	178.74	2.78	-0.25
245.53	181.84	3.52	-0.45
251.72	183.91	4.24	-0.67
253.91	184.64	5.04	-0.90
256.36	185.45	5.76	-1.12
260.20	186.73	6.46	-1.32
262.03	187.34	7.83	-1.70
262.68	187.56	9.28	-2.13
263.25	187.75	10.78	-2.51
261.18	187.06	11.48	-2.69
260.01	186.67	12.18	-2.83
259.62	186.54	12.85	-3.00
257.55	185.85	13.56	-3.16
253.26	184.42	15.00	-3.46
250.31	183.44	16.47	-3.72
243.76	181.25	19.21	-4.19

Job:	Encl. No
Blokhus sand	77
Exc:	Check:
HSP	LB

Remark:

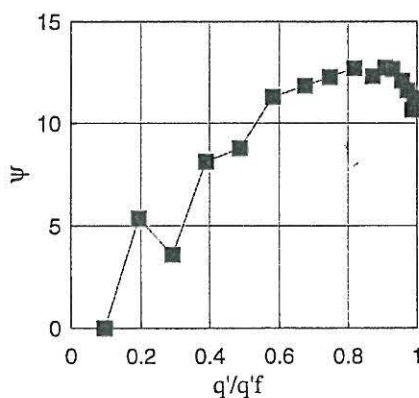
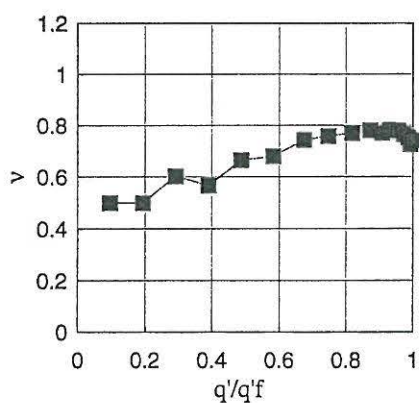


Job:	Encl. No
Blokhus sand	78
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.9	
Calibration file	Date	Void ratio	0.649	0.713
Loadtransducer 181	04.03.74	Saturation	1.02	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-1.85 kPa
		ϵ_1	0.001 %
		ϵ_v	-0.144 %
	2. Drained compression.		
	Deformation rate:		9.7 % ph

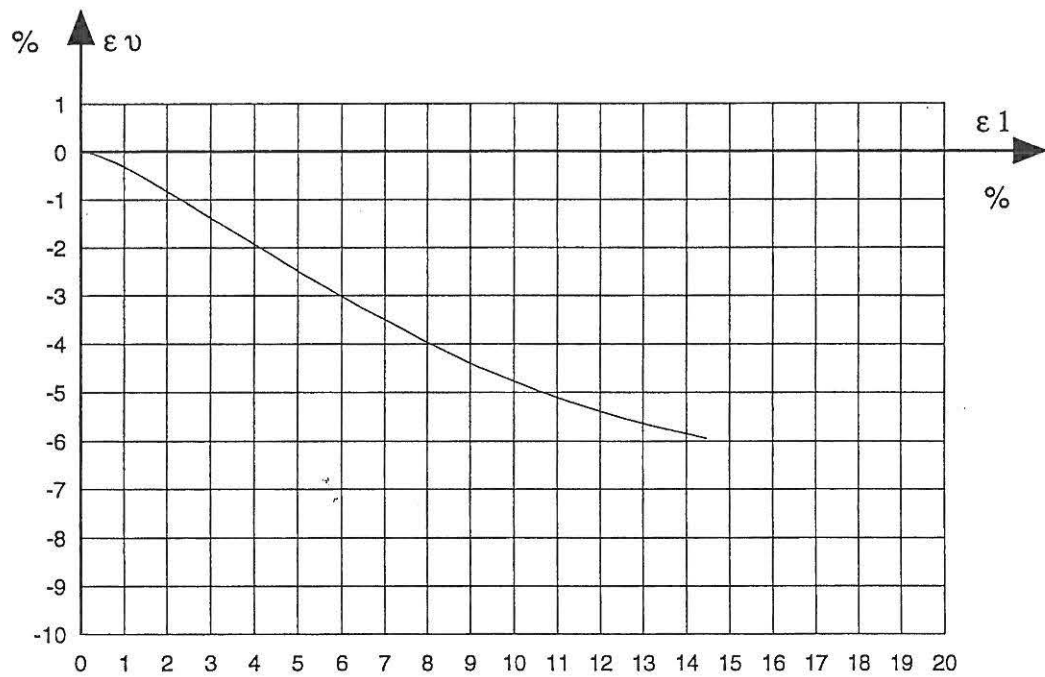
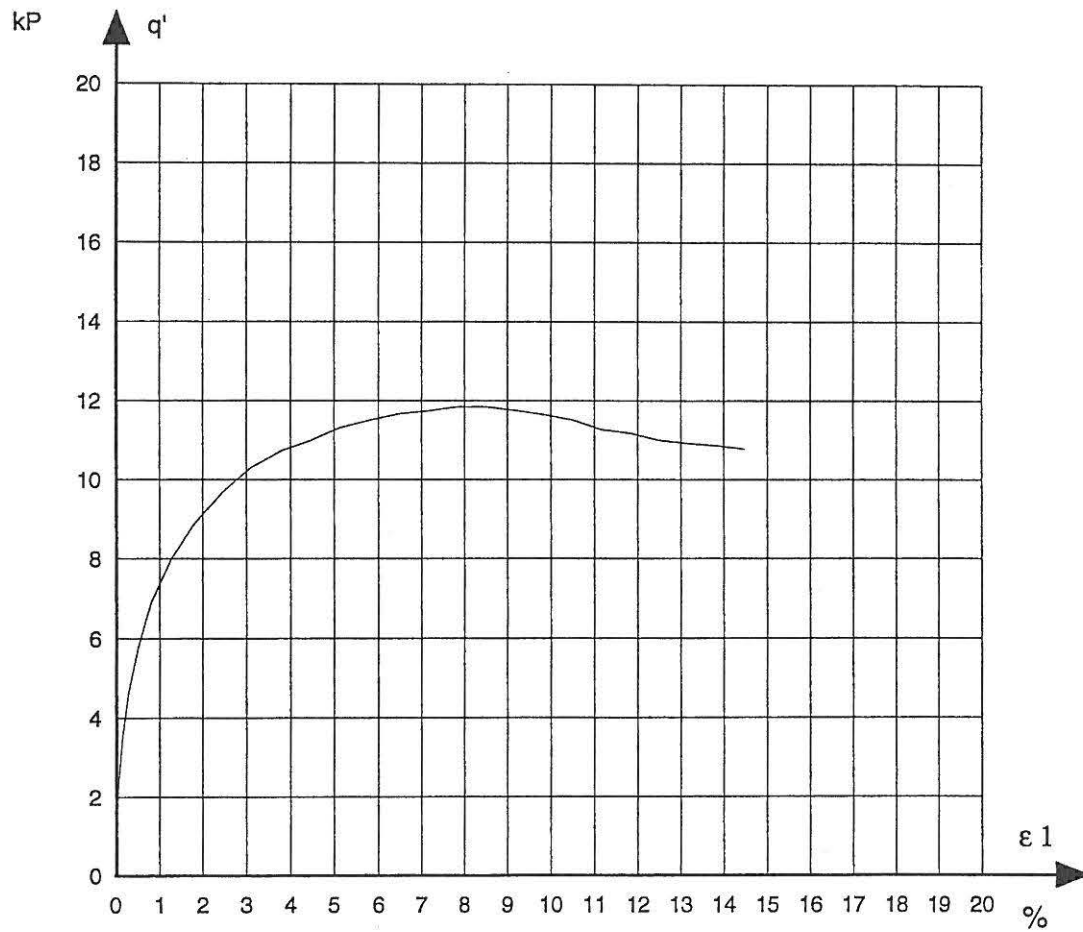
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	11.85	kPa	2.29	kPa
Mean normal stress	p'	5.80	kPa	2.61	kPa
Confining pressures	σ_3	1.85	kPa	1.85	kPa
Vertical strain	ϵ_1	7.84	%	0.07	%
Volumetric strain	ϵ_v	-3.90	%	0.00	%



q'	p'	ϵ_1	ϵ_v
1.13	2.23	0.00	0.00
1.13	2.23	0.02	0.00
2.29	2.61	0.07	0.00
3.46	3.00	0.16	-0.02
4.61	3.39	0.29	-0.04
5.76	3.77	0.51	-0.11
6.89	4.15	0.81	-0.22
7.99	4.51	1.26	-0.43
8.85	4.80	1.78	-0.70
9.68	5.08	2.45	-1.06
10.32	5.29	3.12	-1.44
10.74	5.43	3.79	-1.80
11.00	5.52	4.45	-2.18
11.33	5.63	5.13	-2.56
11.51	5.69	5.81	-2.92
11.68	5.74	6.49	-3.27
11.75	5.77	7.17	-3.57
11.85	5.80	7.84	-3.90
11.84	5.80	8.51	-4.19
11.76	5.77	9.19	-4.48
11.51	5.69	10.51	-4.94
11.19	5.58	11.82	-5.34
10.92	5.49	13.28	-5.70
10.78	5.44	14.48	-5.95

Job:	Encl. No
Blokhus sand	79
Exc:	Check:
HSP	LB

Remark:

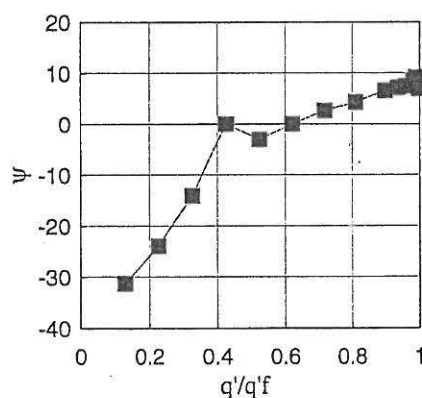
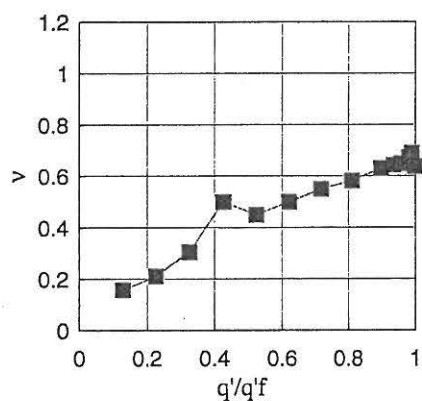


Job:	Encl. No
Blokhuis sand	80
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	25.5	Before test	At failure	
		Grain density	2.65			
Calibration file	Date	Void ratio	0.757			0.788
Loadtransducer 2003	05.03.74	Saturation	0.89			
		Dimension H mm	72			
		D mm	70			

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-100 kPa
		ϵ_1	0.15 %
		ϵ_v	0.36 %
	2. Drained compression.	Deformation rate: 9.7 % ph	

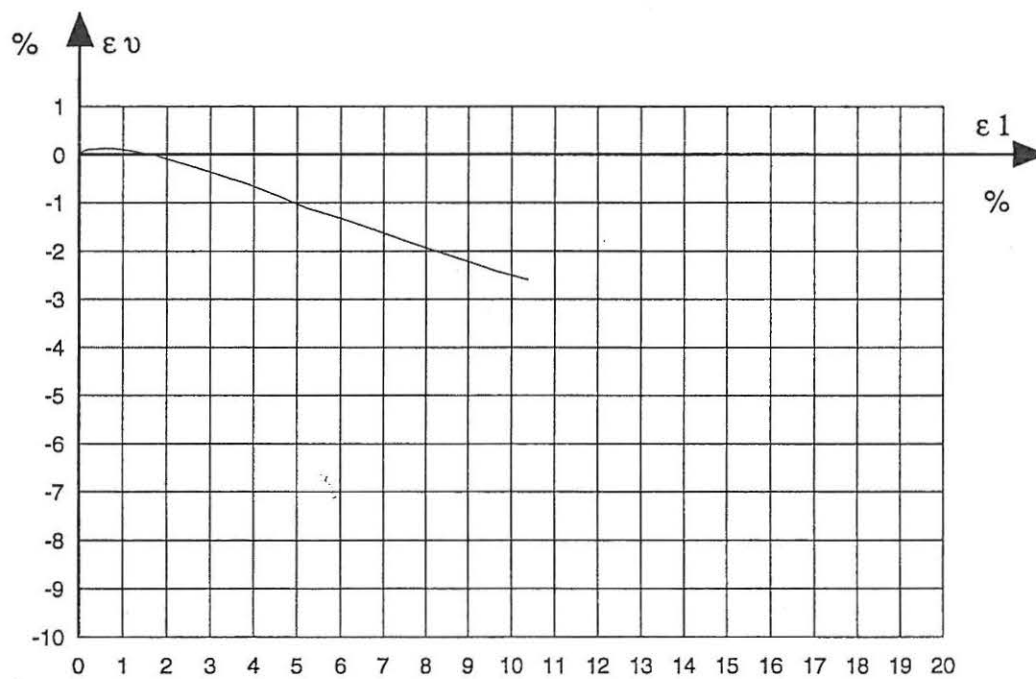
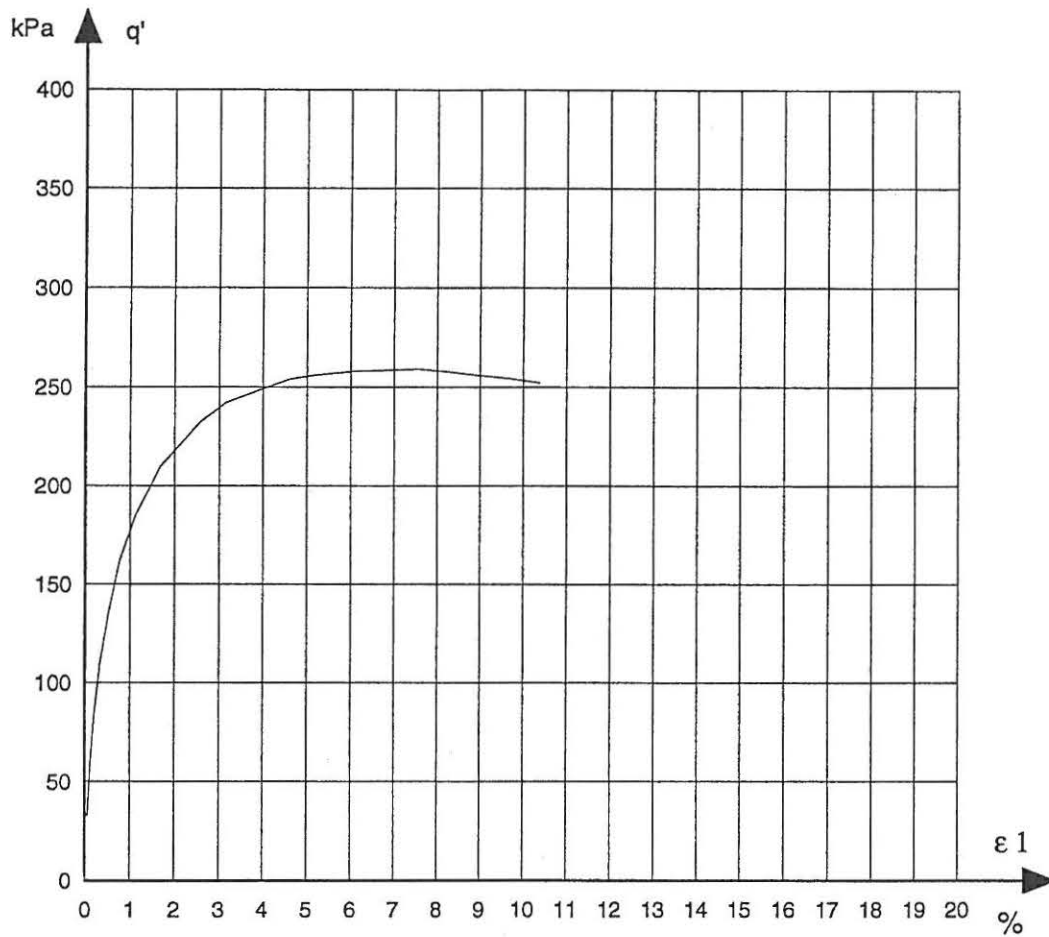
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	259.00	kPa	160.70	kPa
Mean normal stress	p'	186.33	kPa	153.57	kPa
Confining pressures	σ_3	100.00	kPa	100.00	kPa
Vertical strain	ϵ_1	7.57	%	0.75	%
Volumetric strain	ϵ_v	-1.80	%	0.13	%



q'	p'	ϵ_1	ϵ_v
33.00	111.00	0.00	0.00
32.96	110.99	0.05	0.04
58.69	119.56	0.12	0.07
84.38	128.13	0.21	0.11
109.97	136.66	0.33	0.11
135.45	145.15	0.51	0.13
160.70	153.57	0.75	0.13
185.53	161.84	1.12	0.09
209.65	169.88	1.67	0.00
232.27	177.42	2.58	-0.23
241.96	180.65	3.15	-0.40
248.00	182.67	3.87	-0.61
253.88	184.63	4.60	-0.87
256.00	185.33	5.21	-1.10
258.09	186.03	6.13	-1.35
258.46	186.15	6.88	-1.59
259.00	186.33	7.57	-1.80
257.60	185.87	8.24	-2.00
255.89	185.30	8.95	-2.20
254.41	184.80	9.66	-2.42
252.08	184.03	10.39	-2.60

Job:	Encl. No
Blokhus sand	81
Exc:	Check:
HSP	LB

Remark:

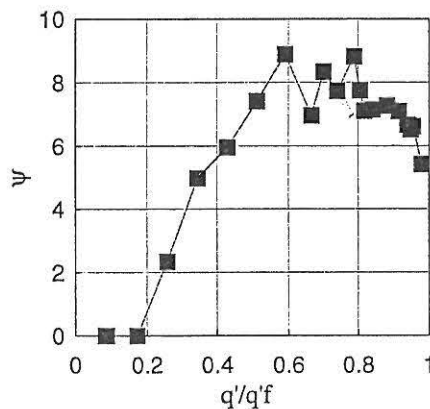
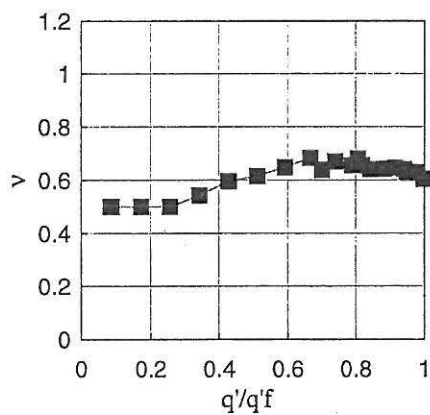


Job:	Blokhuis sand	Encl. No	82
Exc:	HSP	Check:	LB

Description of soil			Before test	At failure
Blokhus sand		Water content %	27.3	0.838
		Grain density	2.65	
Calibration file	Date	Void ratio	0.774	
		Saturation	0.94	
Loadtransducer	05.03.74	Dimension H mm	72	
181		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-2.5 kPa
		ϵ_1	-0.004 %
		ϵ_v	-0.144 %
2. Drained compression.			
Deformation rate:			9.7 % ph

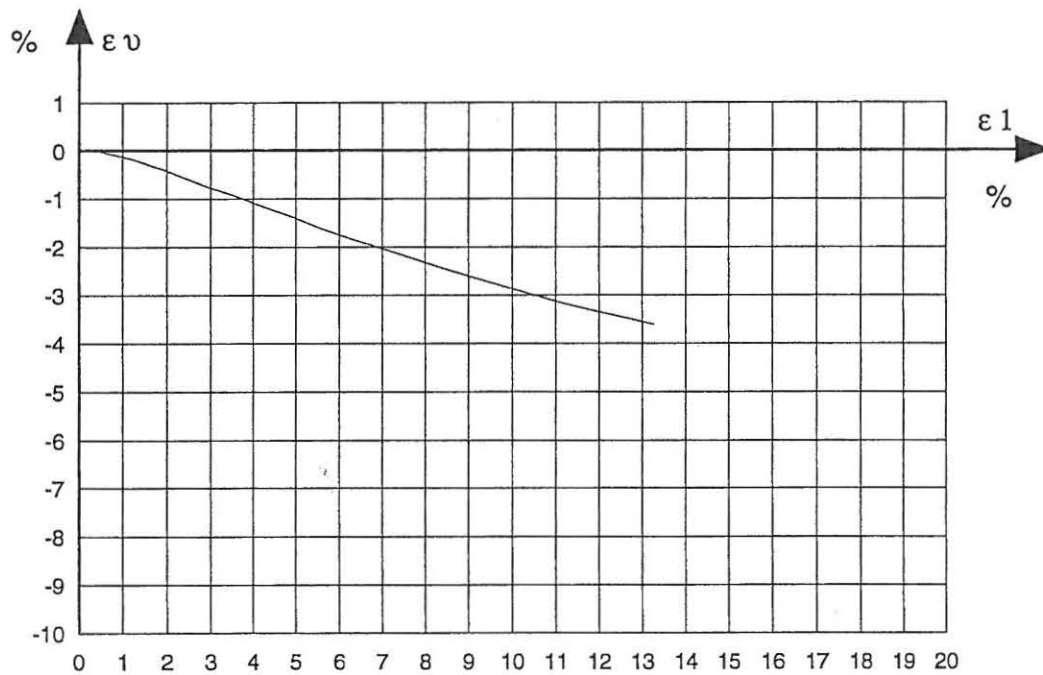
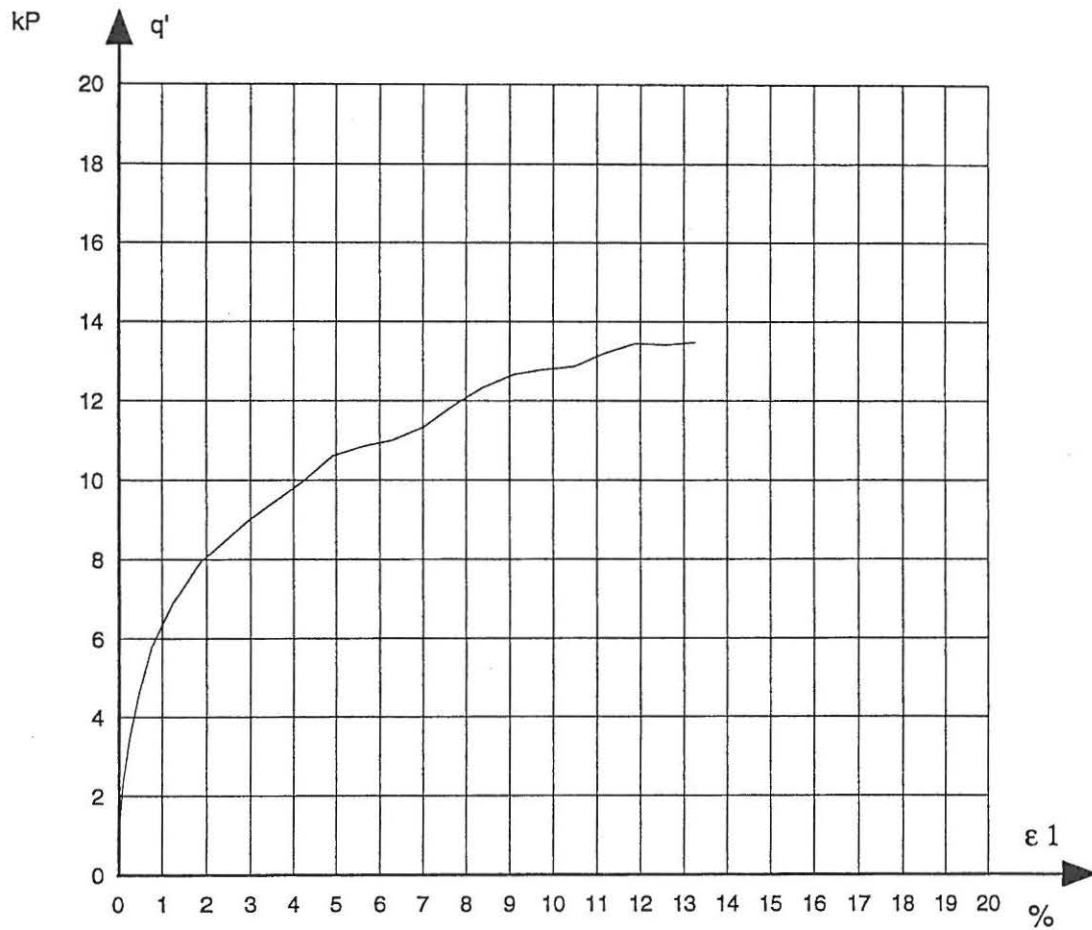
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'		3.48 kPa
Mean normal stress	p'		3.01 kPa
Confining pressures	σ_3		1.85 kPa
Vertical strain	ϵ_1		0.27 %
Volumetric strain	ϵ_v		0.00 %



q'	p'	ϵ_1	ϵ_v
1.15	2.23	0.00	0.00
1.15	2.23	0.03	0.00
2.32	2.62	0.12	0.00
3.48	3.01	0.27	0.00
4.63	3.39	0.48	-0.02
5.77	3.77	0.77	-0.07
6.89	4.15	1.23	-0.18
7.97	4.51	1.90	-0.38
8.98	4.84	2.94	-0.76
9.44	5.00	3.53	-0.92
9.96	5.17	4.22	-1.15
10.62	5.39	4.92	-1.37
10.85	5.47	5.62	-1.62
11.02	5.52	6.31	-1.84
11.34	5.63	7.01	-2.04
11.89	5.81	7.71	-2.24
12.33	5.96	8.40	-2.44
12.66	6.07	9.10	-2.63
12.78	6.11	9.79	-2.81
12.86	6.14	10.49	-3.00
13.20	6.25	11.18	-3.18
13.44	6.33	11.88	-3.32
13.41	6.32	12.58	-3.46
13.48	6.34	13.27	-3.61

Job:	Encl. No
Blokhus sand	83
Exc:	Check:
HSP	LB

Remark:
Specimen disturbed.

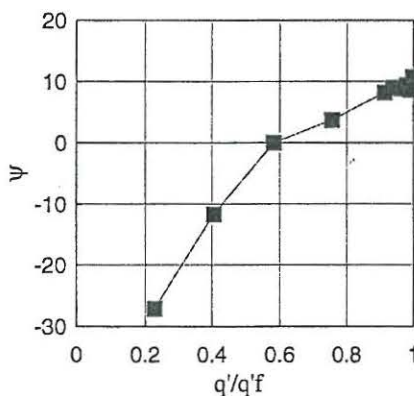
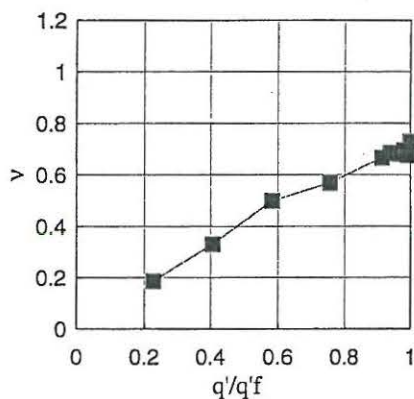


Job:	Encl. No
Blokhuis sand	84
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.8	
Calibration file	Date	Void ratio	0.740	0.779
Loadtransducer	06.03.74	Saturation	0.89	
2003		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-50 kPa
		ϵ_1	0.094 %
		ϵ_v	0.235 %
	2. Drained compression.		
	Deformation rate:		9.7 % ph

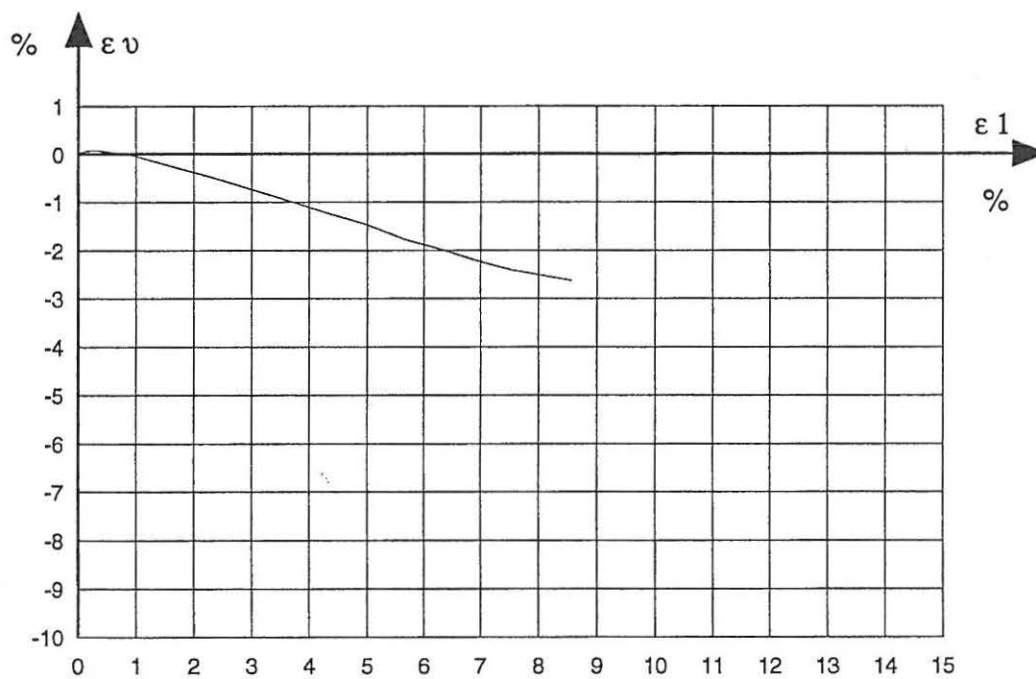
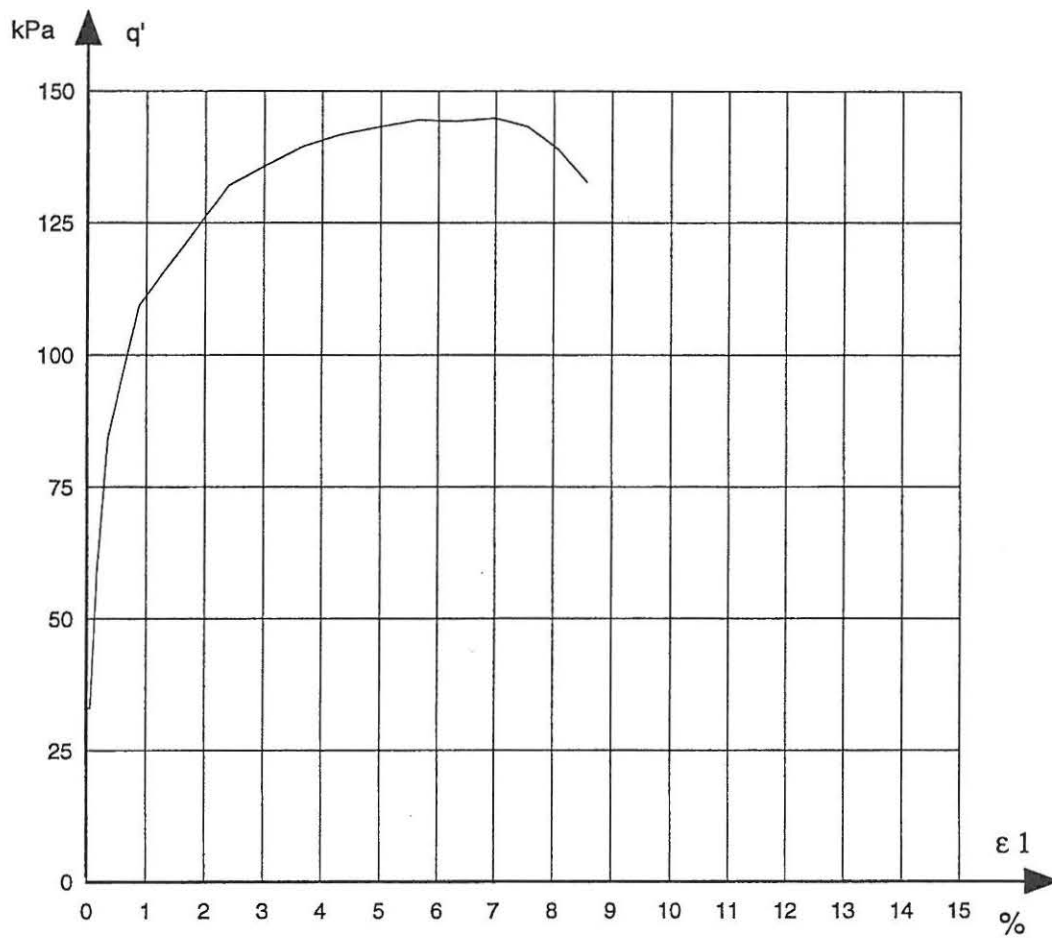
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	144.86	kPa	84.23	kPa
Mean normal stress	p'	98.29	kPa	78.08	kPa
Confining pressures	σ_3	50.00	kPa	50.00	kPa
Vertical strain	ϵ_1	6.99	%	0.35	%
Volumetric strain	ϵ_v	-2.24	%	0.07	%



q'	p'	ϵ_1	ϵ_v
33.00	61.00	0.00	0.00
32.95	60.98	0.06	0.04
58.66	69.55	0.16	0.07
84.23	78.08	0.35	0.07
109.25	86.42	0.87	0.00
132.05	94.02	2.39	-0.51
135.89	95.30	3.02	-0.74
139.54	96.51	3.67	-0.97
141.76	97.25	4.31	-1.23
143.30	97.77	4.98	-1.46
144.53	98.18	5.65	-1.77
144.30	98.10	6.31	-1.98
144.86	98.29	6.99	-2.24
143.22	97.74	7.56	-2.42
138.94	96.31	8.08	-2.53
132.58	94.19	8.58	-2.63

Job:	Encl. No
Blokhus sand	85
Exc:	Check:
HSP	LB

Remark:

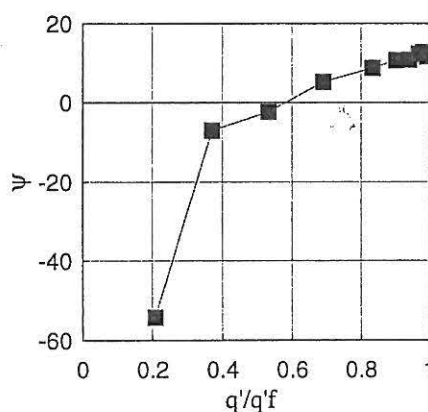
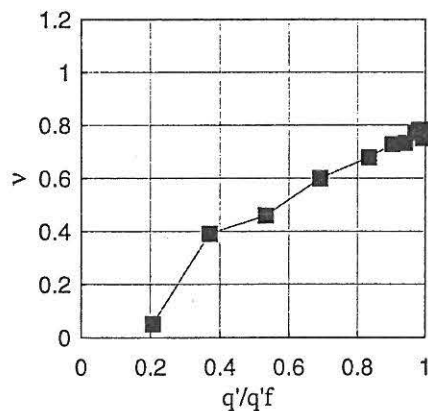


Job:	Encl. No
Blokhus sand	86
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.2	
Calibration file	Date	Void ratio	0.687	0.723
Loadtransducer 2003	07.03.74	Saturation	0.93	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-50 kPa
		ϵ_1	0.063 %
		ϵ_v	0.198 %
	2. Drained compression.		
Deformation rate:			9.7 % ph

		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	158.23	kPa	84.17	kPa
Mean normal stress	p'	102.74	kPa	78.06	kPa
Confining pressures	σ_3	50.00	kPa	50.00	kPa
Vertical strain	ϵ_1	5.22	%	0.44	%
Volumetric strain	ϵ_v	-2.11	%	0.09	%

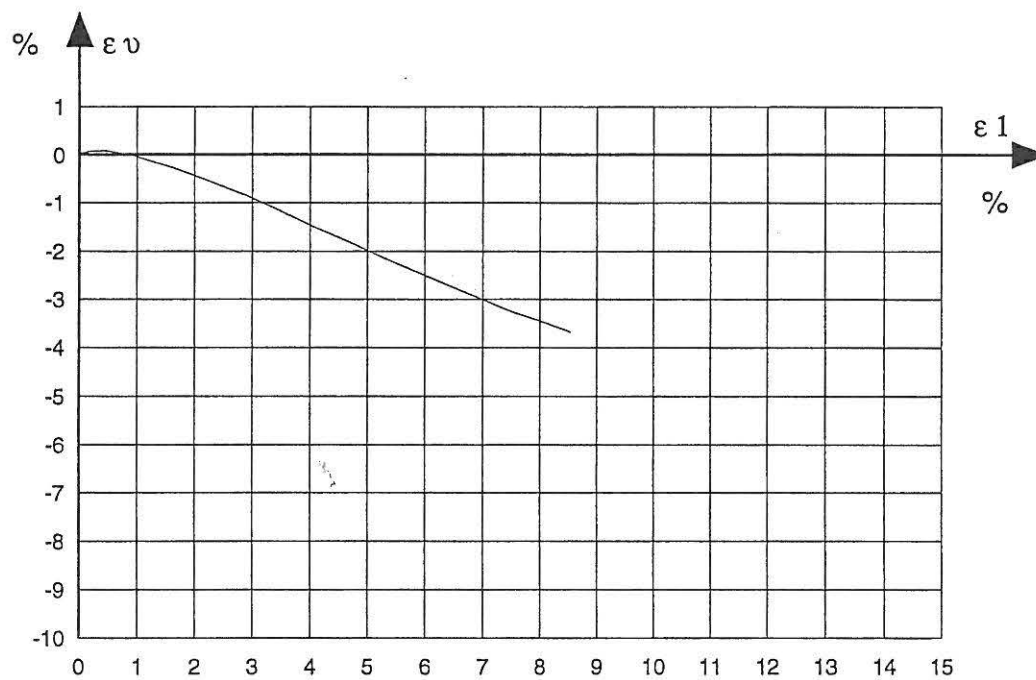
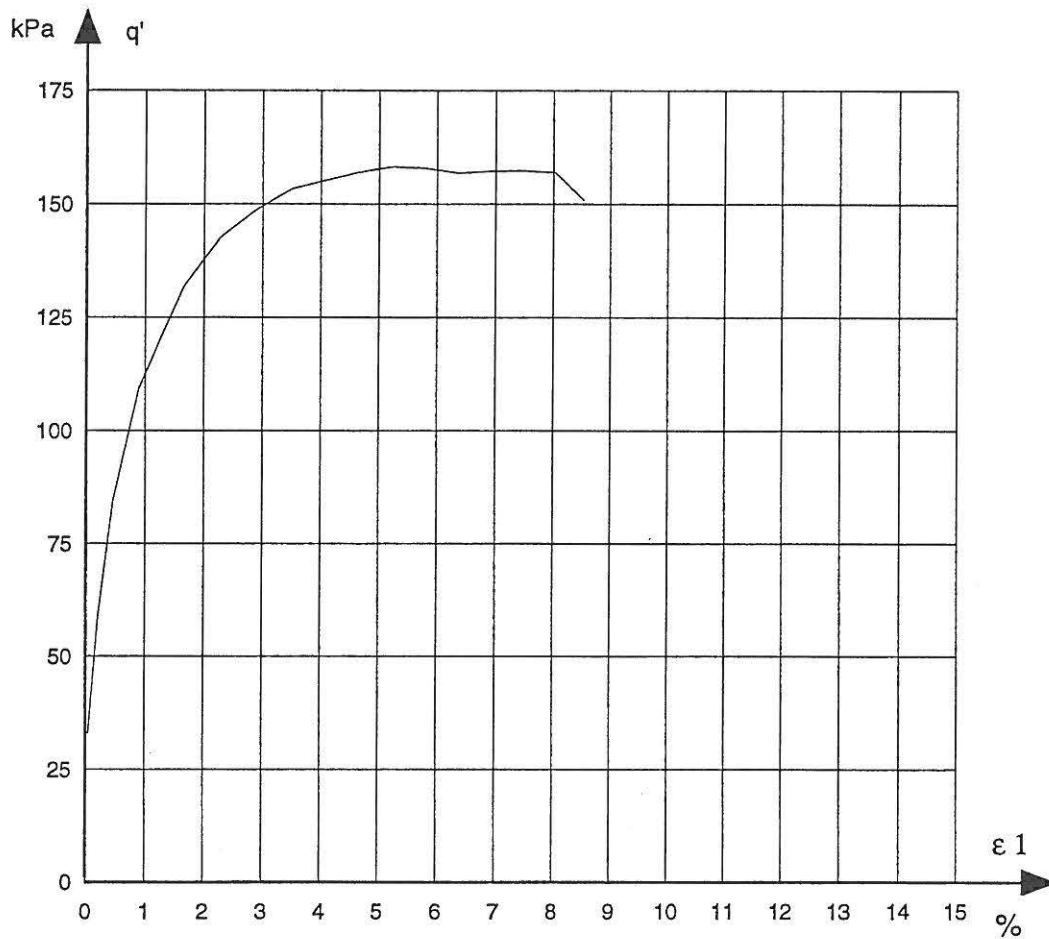


q'	p'	ϵ_1	ϵ_v
33.00	61.00	0.00	0.00
32.96	60.99	0.04	0.04
58.63	69.54	0.21	0.07
84.17	78.06	0.44	0.09
109.24	86.41	0.89	0.00
131.69	93.90	1.65	-0.27
142.63	97.54	2.28	-0.56
148.76	99.59	2.90	-0.85
153.27	101.09	3.50	-1.17
155.22	101.74	4.07	-1.50
157.04	102.35	4.65	-1.79
158.23	102.74	5.22	-2.11
157.88	102.63	5.79	-2.40
156.86	102.29	6.36	-2.69
157.33	102.44	6.93	-2.98
157.37	102.46	7.48	-3.25
157.02	102.34	8.03	-3.46
150.78	100.26	8.54	-3.68

Job: Blokhus sand	Encl. No 87
Exc: AMS	Check: LB

Remark:

Then specimen slipped out after failure.

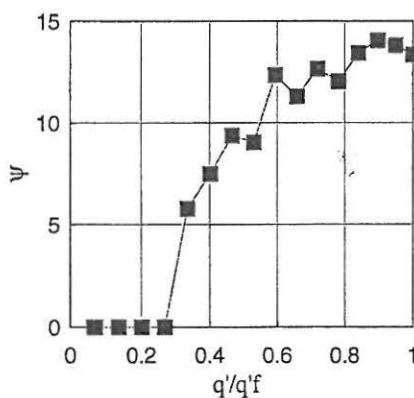
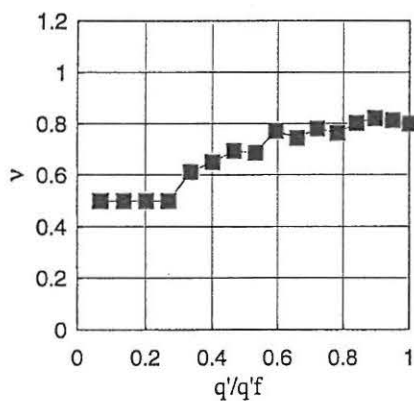


Job:	Encl. No
Blokhuis sand	88
Exc:	Check:
HSP	LB

Description of soil Blokhus sand		Water content %	Before test	At failure
		Grain density	24.2	
Calibration file	Date	Void ratio	0.681	0.721
Loadtransducer	06.03.74	Saturation	0.94	
181		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-1.85 kPa
		ϵ_1	-0.060 %
		ϵ_v	-0.090 %
	2. Drained compression.		
	Deformation rate:		9.7 % ph

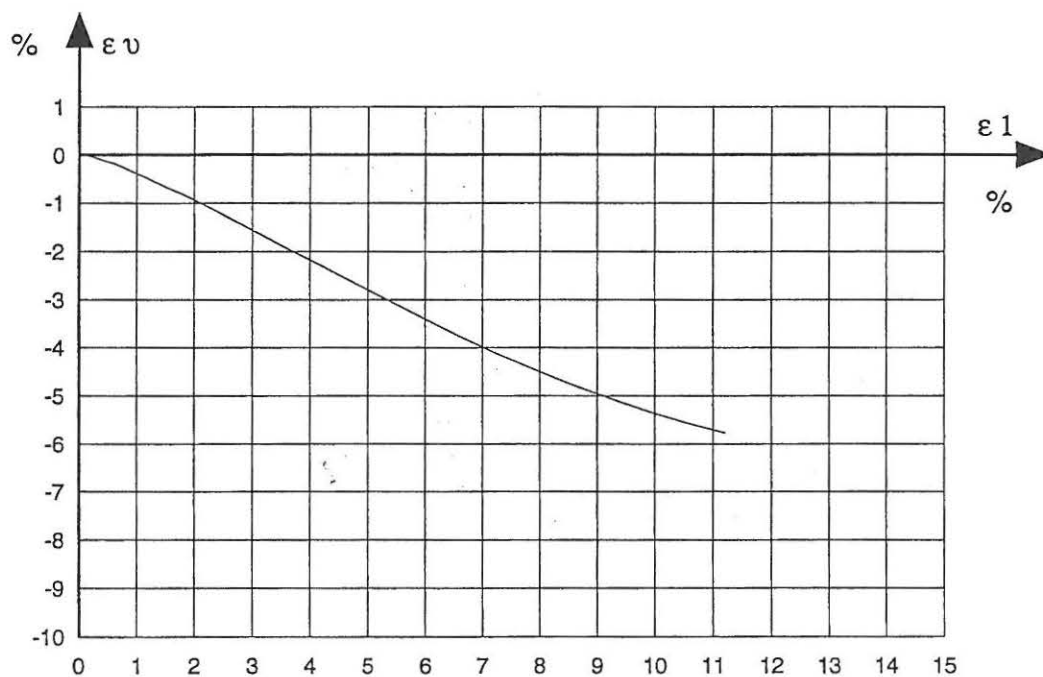
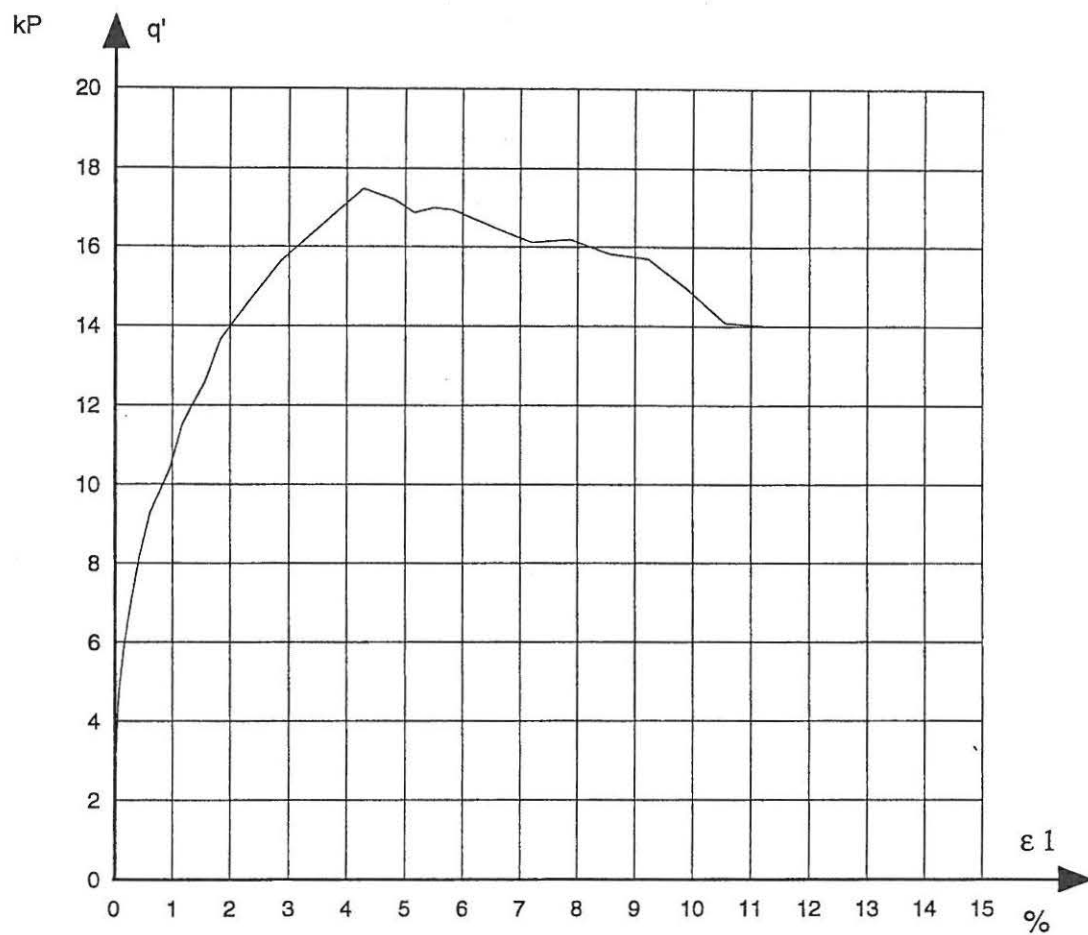
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	17.48	kPa	4.68	kPa
Mean normal stress	p'	7.68	kPa	3.41	kPa
Confining pressures	σ_3	1.85	kPa	1.85	kPa
Vertical strain	ϵ_1	4.28	%	0.08	%
Volumetric strain	ϵ_v	-2.35	%	0.00	%



q'	p'	ϵ_1	ϵ_v
1.19	2.25	0.00	0.00
1.19	2.25	0.00	0.00
2.35	2.63	0.02	0.00
3.52	3.02	0.05	0.00
4.68	3.41	0.08	0.00
5.84	3.80	0.17	-0.02
7.00	4.18	0.29	-0.05
8.14	4.56	0.42	-0.11
9.28	4.94	0.62	-0.18
10.38	5.31	0.95	-0.36
11.49	5.68	1.17	-0.47
12.57	6.04	1.56	-0.69
13.65	6.40	1.83	-0.83
14.66	6.74	2.34	-1.14
15.65	7.07	2.87	-1.48
16.57	7.37	3.59	-1.93
17.48	7.68	4.28	-2.35
17.20	7.58	4.82	-2.69
16.87	7.47	5.16	-2.89
17.00	7.52	5.50	-3.10
16.51	7.35	6.52	-3.72
16.19	7.25	7.87	-4.44
15.70	7.08	9.23	-5.05
14.02	6.52	11.22	-5.77

Job:	Encl. No
Blokhus sand	89
Exc:	Check:
HSP	LB

Remark:

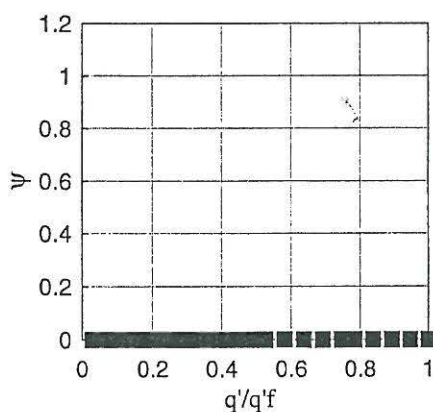
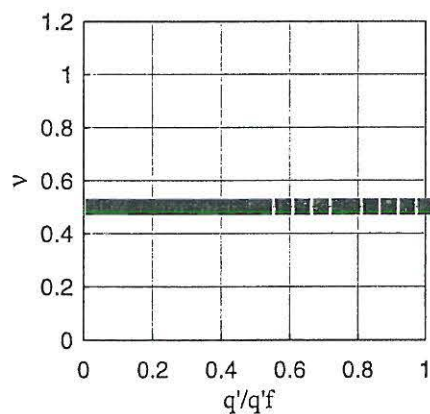


Job:	Encl. No
Blokhuis sand	90
Exc:	Check:
HSP	LB

Description of soil			Before test	At failure
Blokhus sand		Water content %	25.6	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.744	
Loadtransducer 500-1	11.03.74	Saturation	0.911	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	20-100 kPa
		ϵ_1	0.120 %
		ϵ_v	0.040 %
	2. Undrained compression.		
Deformation rate:			% ph

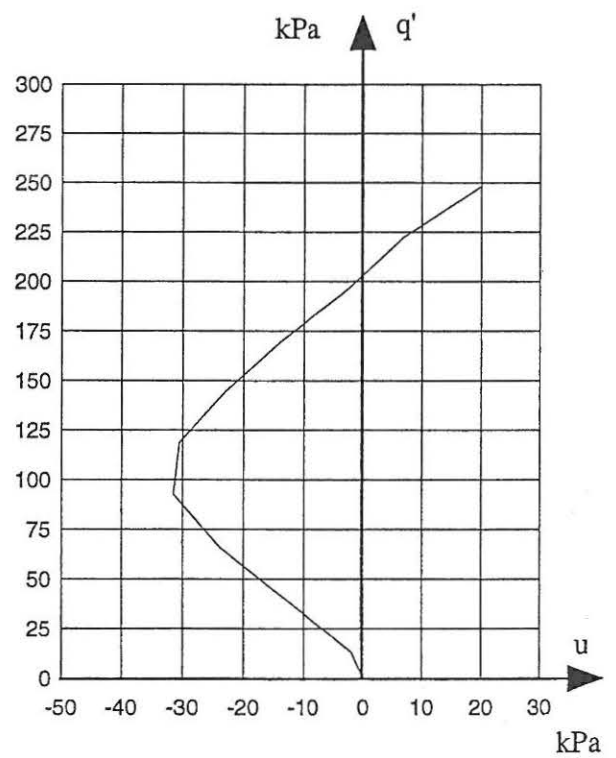
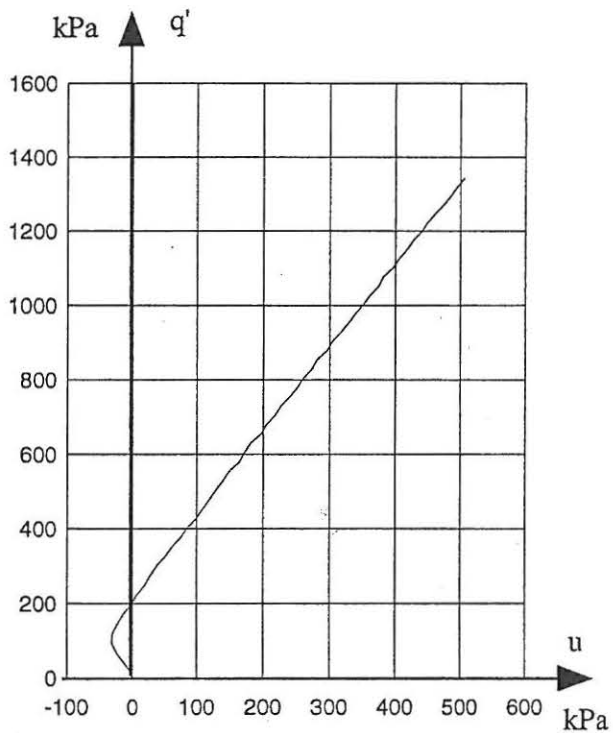
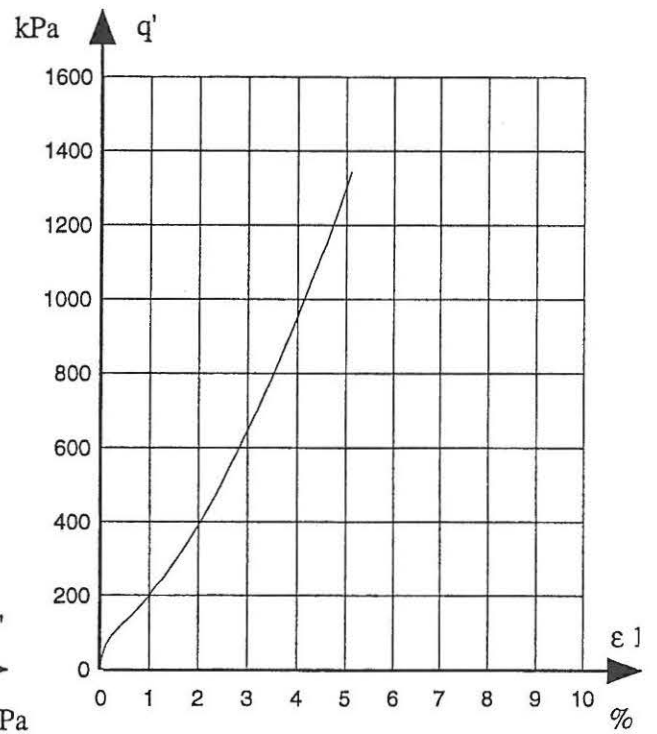
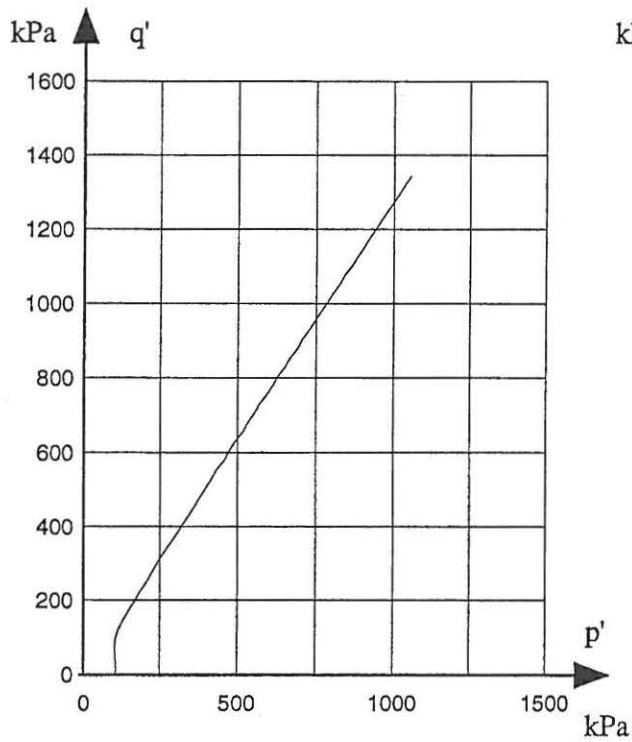
		Maximum values	Minimum values for σ_3
Deviator stress	q'	1,343.40 kPa	92.40 kPa
Mean normal stress	p'	1,055.80 kPa	100.30 kPa
Confining pressures	σ_3	608.00 kPa	69.50 kPa
Vertical strain	ϵ_1	5.11 %	0.23 %
Volumetric strain	ϵ_v	0.00 %	0.00 %



q'	p'	ϵ_1	ϵ_v
0.00	101.00	0.00	0.00
39.68	101.23	0.04	0.00
92.40	100.30	0.23	0.00
144.63	126.21	0.63	0.00
196.52	164.01	0.98	0.00
248.13	203.71	1.30	0.00
305.95	243.98	1.60	0.00
350.74	279.91	1.82	0.00
401.75	320.42	2.05	0.00
452.60	360.87	2.27	0.00
503.28	397.76	2.47	0.00
553.84	434.61	2.65	0.00
604.21	474.40	2.84	0.00
654.34	516.11	3.03	0.00
704.40	552.80	3.20	0.00
779.24	611.75	3.45	0.00
853.74	666.58	3.70	0.00
927.99	727.33	3.92	0.00
1001.93	785.98	4.15	0.00
1051.07	825.36	4.29	0.00
1124.51	880.84	4.51	0.00
1197.67	939.22	4.72	0.00
1270.69	998.56	4.91	0.00
1343.40	1055.80	5.11	0.00

Job:	Encl. No
Blokhus sand	91
Exc:	Check:
HSP, AMS	FRJ

Remark:
Deformation rate not measured.

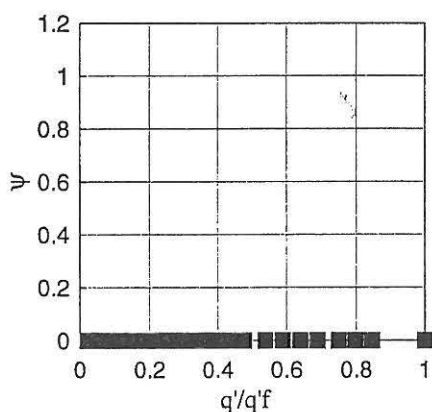
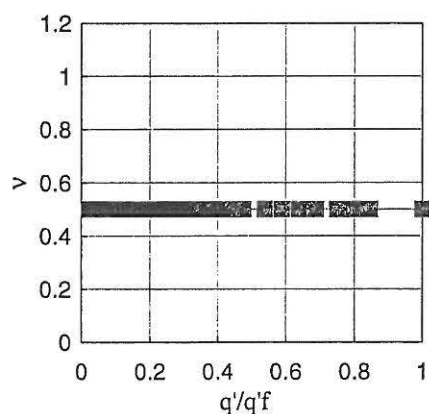


Job:	Encl. No
Blokhuis sand	92
Exc:	Check:
HSP, AMS	FRJ

Description of soil			Before test	At failure
Blokhus sand		Water content %	25.9	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.714	
Loadtransducer 1500-1	13.03.74	Saturation	0.959	
		Dimension H mm	72	
		D mm	70	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	10-50 kPa
		ϵ_1	0.109 %
		ϵ_v	-0.054 %
	2. Undrained compression.		
Deformation rate:			% ph

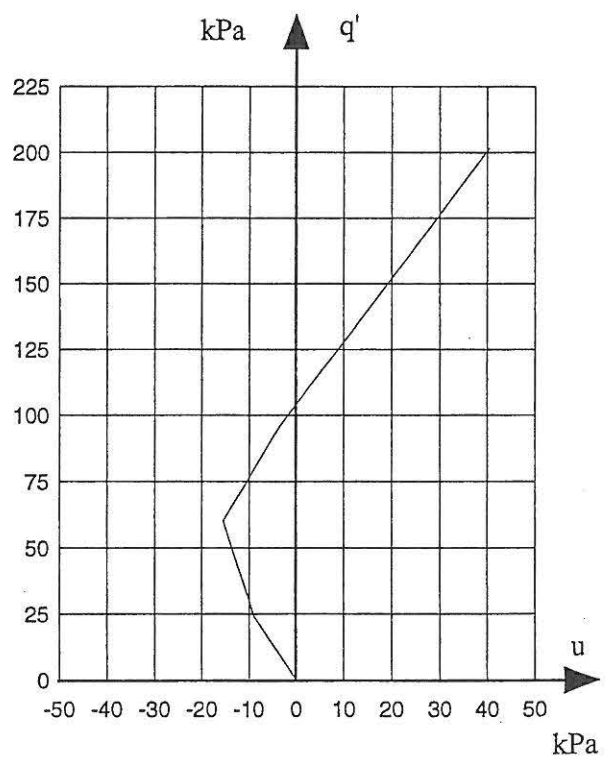
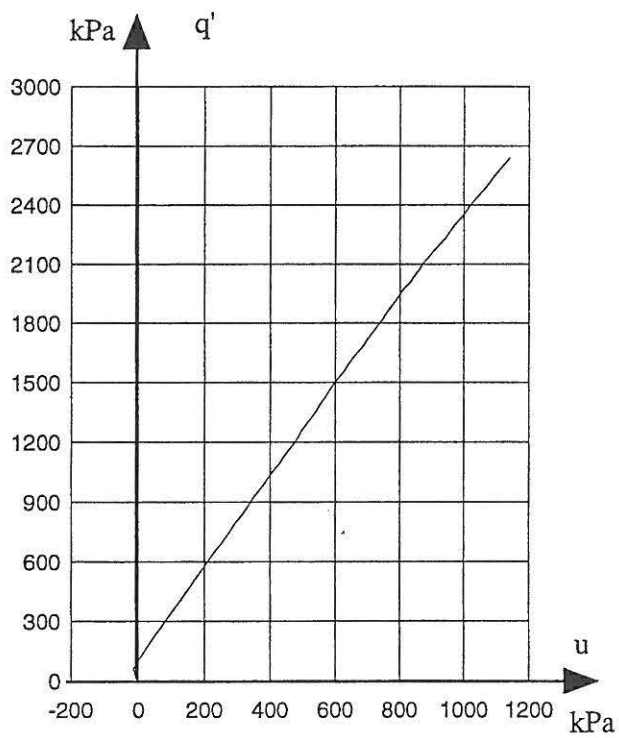
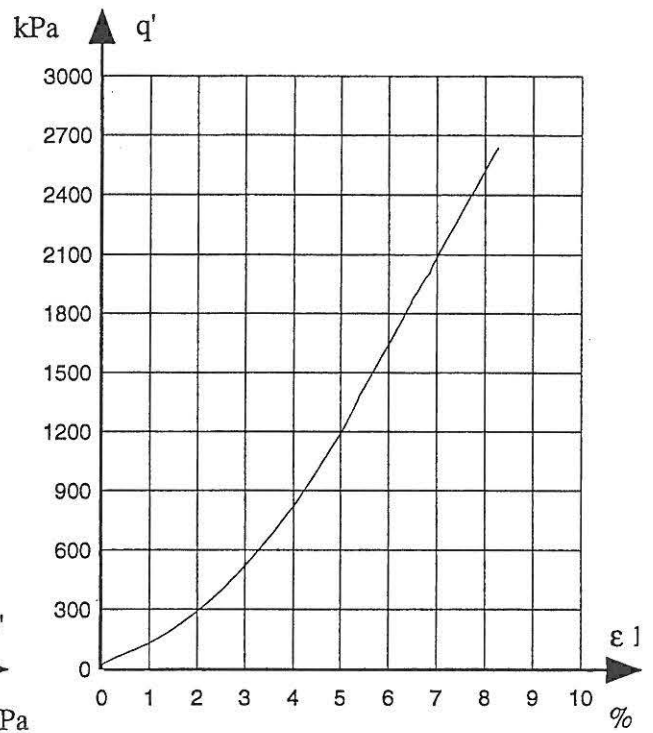
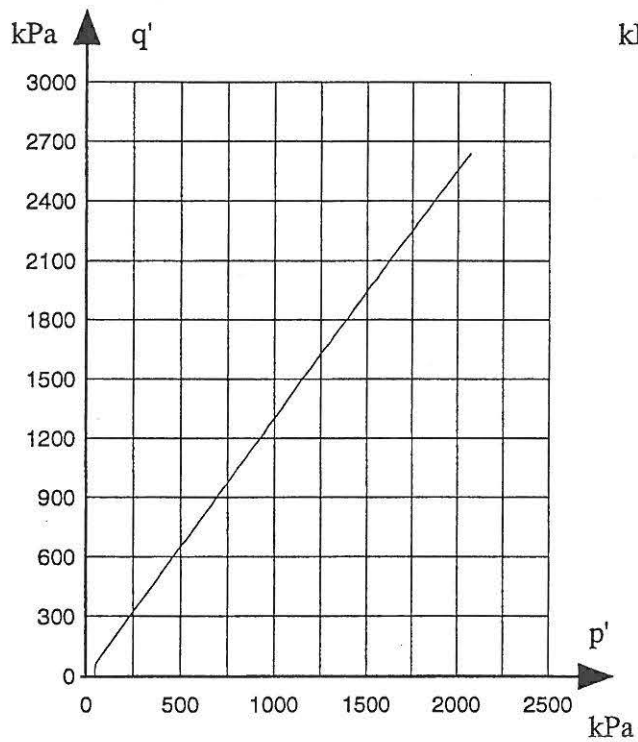
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2,639.54	kPa	60.13	kPa
Mean normal stress	p'	2,071.85	kPa	55.04	kPa
Confining pressures	σ_3	1,192.00	kPa	35.00	kPa
Vertical strain	ϵ_1	8.27	%	0.32	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.00	50.50	0.00	0.00
60.13	55.04	0.32	0.00
131.22	105.74	1.01	0.00
201.72	158.24	1.50	0.00
271.77	211.59	1.90	0.00
341.38	263.79	2.26	0.00
410.67	316.89	2.57	0.00
479.69	369.90	2.85	0.00
548.40	421.80	3.11	0.00
651.05	499.02	3.45	0.00
753.00	581.00	3.79	0.00
854.51	658.84	4.10	0.00
955.60	735.53	4.38	0.00
1056.26	812.09	4.65	0.00
1156.36	891.45	4.91	0.00
1256.32	964.77	5.15	0.00
1422.18	1092.06	5.51	0.00
1553.89	1193.96	5.80	0.00
1684.59	1298.53	6.10	0.00
1814.75	1400.92	6.38	0.00
1976.05	1522.68	6.75	0.00
2104.36	1625.45	7.03	0.00
2231.53	1736.84	7.33	0.00
2639.54	2071.85	8.27	0.00

Job:	Encl. No
Blokhus sand	93
Exc:	Check:
HSP, AMS	FRJ

Remark:
Deformaton rate not measured.



Job:	Encl. No
Blokhuis sand	94
Exc:	Check:
HSP, AMS	FRJ

